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Revision 0

Waste Feed Delivery Transfer System Analysis

Prepared for the U.S. Department of Energy
Assistant Secretary for Environmental Management

CH2MHILL

Hanford Group, Inc.

Richland, Washington

Contractor for the U.S. Department of Energy
Office of River Protection under Contract DE-AC06-99RL14047

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Waste Feed Delivery Transfer System Analysis

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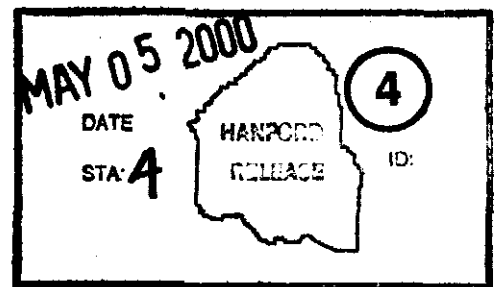
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Abstract: This document provides a documented basis for the required design pressure rating and pump pressure capacity of the Hanford Site waste-transfer system in support of the waste feed delivery to the privatization contractor for vitrification. The scope of the analysis includes the 200 East Area double-shell tank waste transfer pipeline system and the associated transfer system pumps for all Phase 1B and Phase 2 waste transfers from AN, AP, AW, AY, and AZ Tank Farms.

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Waste Feed Delivery Transfer System Analysis

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Date Published
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SUMMARY

This engineering analysis was prepared to provide a documented basis for the design pressure rating of the waste-transfer system pipeline and a documented basis for the design and operating pressure of the transfer-system pumps. The waste-transfer system will be used to transfer waste from the Hanford Site double-shell and single-shell tanks to the privatization contractor for vitrification. The scope of the analysis includes all Phase 1B and Phase 2 waste transfers from the AN, AP, AW, AY, and AZ Tank Farms. In performing the analysis, no assumptions were made that might restrict how the wastes are processed before transfer to the privatization contractor. The analysis focuses on the hydraulics of the waste-transfer system using available waste-characterization data. The analysis also compares the design pressure rating of the existing transfer system pipeline to the design pressure rating determined by this analysis.

The initial approach of the analysis was to identify waste properties for each tank and then calculate the pressure drop through the planned transfer routes using the tank-specific waste properties. The pressure drop information is used to determine the pipe and pump design pressure ratings. Calculation of the pressure drop requires knowledge of the waste properties in addition to the physical dimensions of the transfer system. Waste properties include the liquid density, liquid viscosity, particle density, particle size distribution, and volume percent solids. After reviewing available waste-property information, it was found that there are insufficient tank-specific data available to perform the pressure drop calculations. Additionally, the waste feed delivery plan for Phase 1B and Phase 2, which identifies individual waste transfers, has not been finalized. Identification of individual waste transfers is required to determine the physical route of the transfer pipeline and to calculate the pressure drop. In the absence of complete tank-specific waste-property data for each tank and a final plan for waste transfers, the pressure drop for each of the Phase 1B and Phase 2 waste transfers could not be calculated.

As an alternative, this analysis calculates the pressure drop through each transfer route using three sets of typical slurry waste properties. Slurry waste properties, rather than supernate waste properties, were chosen to evaluate each transfer route to provide maximum flexibility for planning of the Phase 1B and Phase 2 transfers. If all of the transfer lines are capable of carrying slurry waste, the Phase 1B and Phase 2 transfers can use any of the transfer pipelines to transfer waste to the privatization contractor. The first set of waste properties was chosen to provide a conservative set of values that would encompass all of the tank waste transfers; it included 30 volume percent solids. The second and third sets of waste properties, while still somewhat conservative, were chosen to be more representative of actual slurry waste properties. These sets of properties included 15 and 10 volume percent solids, respectively, and are similar to the design bases used for projects that are currently making modifications to the waste-transfer system. In addition to the evaluation of the three sets of slurry waste properties, the pressure drop through each of the transfer routes was calculated for a total of 52 different combinations of waste properties. These combinations included supernate and slurry waste transfers. The waste combinations provide an indication of the sensitivity of the pressure drop in the pipeline to variations in the waste properties. The waste combinations also provide a tool to quickly evaluate the pressure drop through a particular transfer route with different waste properties. Waste-transfer routes were identified for all Phase 1B transfers using Hanford Tank Waste Operations Simulator (HTWOS) Cases 3S4 and 3S6D. The physical characteristics of each of

these waste-transfer routes were identified and used as the basis for the pressure-drop calculations. Transfer system physical characteristics include pipe size, equivalent lengths of the carbon steel and stainless steel portions of the transfer pipeline, changes in elevation over the transport route, and pipe interior surface roughness. The transfer routes that were evaluated are comprised of the existing transfer-system infrastructure and the modifications that are planned by Projects W-211, W-314 and W-521.

Calculation of the pressure drop through the transfer-system piping requires a determination of the critical velocity of the waste. The critical velocity correlation in "The Critical Velocity in Pipeline Flow of Slurries" (Oroskar and Turian 1980) was used to determine the minimum waste transport velocity required to maintain solids in suspension for slurry transfers. The velocity at the transition from laminar to turbulent flow was used as the critical velocity for supernate transfers. The pressure drop in the system is calculated at 1.3 times the predicted critical velocity to assure an adequate margin. The pressure-drop correlations in "Solid-Liquid Flow Slurry Pipeline Transportation" (Wasp et. al. 1977), with modifications from *Applied Fluid Dynamics Handbook* (Blevins 1984), were used to determine the drop in pressure resulting from friction losses in the transfer pipeline. The calculated pressure drop was then compared to the design pressure rating of the pipeline. Transfer routes that exceeded the pipe design pressure rating at the critical velocity were identified.

The results of this analysis show that supernate transfers will not exceed the design pressure rating of any of the transfer routes that were evaluated. There is sufficient design margin in the waste-transfer system to transfer supernate. The results of the analysis for slurry transfers show that portions of the waste-transfer system may not be adequate. Using the enveloping first set of slurry waste-property data (Appendix D, Case 3b, Group 2 Properties), a majority of the transfer routes would be subject to settling of solids and possibly plugging of the pipeline. The second and third sets of data (Appendix D, Case 4, Groups 1 and 2 Properties), which more closely approximate the actual waste streams, resulted in a slightly smaller percentage of pipelines being susceptible to plugging. The analysis indicates that there is insufficient design margin in the waste-transfer system to support the use of any of the three selected sets of slurry waste properties. An accurate evaluation of the hydraulics of the transfer system requires a more complete determination of the actual properties of the slurry during transfer. The results of the analysis also indicate that the pressure drop in the system is sensitive to changes in waste properties. A reduction in particle size, particle density, or volume percent solids will each reduce the pressure drop in the system. Reductions in more than one of these waste properties has the potential to significantly reduce the pressure drop in the system.

Based on the results of this analysis, alternatives are identified and recommendations are made to improve the ability to accurately evaluate the waste-transfer system. The recommendations are summarized below.

- Consider addition of a lift station or staging transfers through AP tank farm
- Investigate the viability of increasing the design pressure rating of the existing waste-transfer pipeline that is rated at 1.9 MPa (275 lbf/in²)
- Determine the maximum design pressure rating of the new waste-transfer pipeline being installed by Project W-314 that could be achieved without design modifications

- Establish a test program to determine the maximum design pressure rating of the Plutonium-Uranium Extraction (PUREX) facility jumper connectors
- Review particle-size data to identify methods of reducing the uncertainty associated with particle size and density
- Inspect the inside of existing transfer lines to establish a baseline condition for the pipe to assess the level of corrosion and roughness
- Ensure that future tank samples are analyzed for properties relevant to the determination of critical velocity and pressure drop and validate modeling with Hanford waste or simulant
- Investigate the application of process controls to control or limit the range of waste properties that adversely affect pressure drop in the transfer system.

The results of this analysis should only be used for conceptual design and planning until more accurate waste characterization information is available.

All eight recommendations do not need to be performed. Favorable results from the recommendations related to waste characterization could preclude the necessity for a hardware related fix. Alternatively, some combination of hardware and better waste property characterization could be sufficient to ensure that the transfer system is adequate for the waste feed delivery mission.

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LIST OF ABBREVIATIONS AND UNITS

DST	double-shell tank
EOL	end of life
HLW	high-level waste
HTWOS	Hanford Tank Waste Operation Simulator
LAW	low-activity waste
PUREX	Plutonium-Uranium Extraction (facility)
°C	degrees Celsius
cP	centipoise = 0.001 kg/m-s
°F	degrees Fahrenheit
ft	feet = 0.3048 meters (m)
ID	internal diameter
in.	inch = 2.54 centimeters (cm)
kg	kilogram
kg/L	kilogram per liter
L	liter
lbf/in ²	pounds per square inch = 0.006895 megapascals (MPa)
mil	0.001 inch (in.)
MPa	megapascal
s	second (unit of time)
µm	micrometer or micron

ENGLISH SYMBOLS USED IN EQUATIONS

C_D	coefficient of drag
C_L	liquid phase volume fraction
C_s	particulate solid phase volume fraction
d_p	particulate solid phase median size diameter
D	internal pipe diameter
f	pipe friction factor (Darcy) for slurry
f_d	= $f - f_v$, pipe friction factor for heterogeneous particulate solid phase of slurry
f_v	pipe friction factor for homogeneous "vehicle" liquid phase of slurry
g	gravitational acceleration
h	fluid head
h_L	head loss
L	length of pipe
n	hindered settling velocity exponent
p	pressure
R	flow regime designation
Re_p	particle Reynolds number
Re_{pipe}	pipe flow Reynolds number
v	slurry velocity
v_{cr}	generic term for critical velocity

V_s	settling velocity
v^*	friction velocity
V_∞	characteristic velocity of a settling particulate
X_{caustic}	fraction of dissolved solids composed of sodium hydroxide
X_{salt}	fraction of dissolved solids composed of sodium salts and other salts

GREEK SYMBOLS USED IN EQUATIONS

γ	ratio of hindered settling velocity to critical velocity
θ	angle of pipeline relative to horizontal (positive for upward slope)
κ_{cv}	percent root mean square deviation from critical velocity experimental data
ρ_L	carrier liquid phase density
ρ_s	particulate solid phase density
ρ_v	homogeneous "vehicle" density
ρ_{water}	density of water
χ	fraction of the turbulent fluid eddies that possess a velocity larger than the particulate settling velocity
Φ_v	total volume percent solids attributed to homogeneous "vehicle" phase of slurry
Φ_{dj}	$\Phi_{sj} - \Phi_{vj}$, solid concentration of j^{th} particle size remaining in heterogeneous phase of slurry
Φ_{sj}	solid concentration at pipe centerline for j^{th} particle size
Φ_{vj}	solid concentration for j^{th} particle size attributed to homogeneous "vehicle" phase of slurry
β	ratio of mass transport coefficient to momentum transport coefficient (see Equation 9)
κ	von Karman boundary-layer constant (see Equation 9)
ε	roughness of pipe
μ_L	carrier liquid phase viscosity
μ_v	homogeneous "vehicle" viscosity
μ_{water}	viscosity of water
Δh	head loss per unit length of pipe

1.0 INTRODUCTION

CH2M HILL Hanford Group, Inc. is responsible for delivering waste from tanks on the Hanford Site to the privatization contractor for vitrification, BNFL Inc. As part of the planning process for waste feed delivery, the capability of the waste-transfer system is being evaluated. The waste-transfer system comprises the transfer pumps, transfer pipelines, transfer system valves, and associated structures and instrumentation used to transport waste between storage tanks and from storage tanks to the privatization contractor. Waste will be pumped out of the tanks using new transfer pumps and will be delivered to the privatization contractor through a combination of new and existing transfer pipelines. Double-Shell Tank (DST) Subsystem specifications are being prepared as a basis for the conceptual design of the new and replacement equipment required to provide a safe and reliable system for transferring waste. These specifications will include design criteria for parameters such as pipe size, pipe material, range of flow rates, transfer pump operating conditions, and waste characteristics. The purpose of this analysis is to provide a documented basis for DST subsystem specifications for piping and pumps.

Wastes have been successfully transferred between tanks at the Hanford Site for more than 50 years. Although a number of pipelines have been plugged during that period, the cause of the plugging generally has been attributed to the formation of salts as the waste cooled in the pipeline. Previous slurry transfers in the tank farms typically were maintained above a specified flow velocity to ensure that any solids that might be present in the waste would not settle out during the transfer. For approximately the last 20 years, the minimum flow velocity was specified in terms of a minimum Reynolds number of 20,000. In an 80 mm (3-in.) nominal pipeline, this equates to a flow velocity of approximately 1.8 m/s (6 ft/s). To meet the mission of delivering waste feed to the privatization contractor, the existing transfer system had to be extended to the location of the new vitrification facility. This extension of the transfer system adds approximately 1,220 m (4,000 ft) of equivalent pipe length to connect the existing transfer system to the privatization contractor. Before the preparation of this analysis, several preliminary pressure-drop calculations were performed to evaluate the adequacy of the new waste-transfer routes to the vitrification facility. The results of the calculations indicated that the existing transfer pipelines might not be adequate to meet the needs of the waste feed delivery program.

The initial approach of the analysis was to identify waste properties for each tank and then calculate the pressure drop through the planned transfer routes using the tank-specific waste properties. The pressure drop information is used to determine the pipe and pump design pressure ratings. Calculation of the pressure drop requires knowledge of the waste properties in addition to the physical dimensions of the transfer system. Waste properties include the liquid density, liquid viscosity, particle density, particle size distribution, and volume percent solids. After reviewing available waste-property information, it was found that there are insufficient tank-specific data available to perform the pressure drop calculations. Additionally, the waste feed delivery plan for Phase 1B and Phase 2, which identifies individual waste transfers, has not been finalized. Identification of individual waste transfers is required to determine the physical route of the transfer pipeline and to calculate the pressure drop. In the absence of complete tank-specific waste-property data for each tank and a final plan for waste transfers, the pressure drop for each of the Phase 1B and Phase 2 waste transfers could not be calculated.

As an alternative, this analysis calculates the pressure drop through each transfer route using three sets of typical slurry waste properties. Slurry waste properties, rather than supernate waste properties, were chosen to evaluate each transfer route to provide maximum flexibility for planning of the Phase 1B and Phase 2 transfers. If all of the transfer lines are capable of carrying slurry waste, the Phase 1B and Phase 2 transfers can use any of the transfer pipelines to transfer waste to the privatization contractor. The first set of waste properties was chosen to provide a conservative set of values that would encompass all of the tank waste transfers; it included 30 volume percent solids. The second and third sets of waste properties, while still somewhat conservative, were chosen to be more representative of actual slurry waste properties. These sets of properties included 15 and 10 volume percent solids, respectively, and are similar to the design bases used for projects that are currently making modifications to the waste-transfer system. In addition to the evaluation of the three sets of slurry waste properties, the pressure drop through each of the transfer routes was calculated for a total of 52 different combinations of waste properties. These combinations included supernate and slurry waste transfers. The waste combinations provide an indication of the sensitivity of the pressure drop in the pipeline to variations in the waste properties. The waste combinations also provide a tool to quickly evaluate the pressure drop through a particular transfer route with different waste properties. Waste-transfer routes were identified for all Phase 1B transfers using Hanford Tank Waste Operations Simulator (HTWOS) Cases 3S4 and 3S6D. The physical characteristics of each of these waste-transfer routes were identified and used as the basis for the pressure-drop calculations. Transfer system physical characteristics include pipe size, equivalent lengths of the carbon steel and stainless steel portions of the transfer pipeline, changes in elevation over the transport route, and pipe interior surface roughness. The transfer routes that were evaluated are comprised of the existing transfer-system infrastructure and the modifications that are planned by Projects W-211, W-314 and W-521.

This analysis is concerned with the hydraulics of the waste as it is transported through the transfer system. The waste is considered to be a slurry because it is composed of a mixture of solids and liquid. The Hanford Site tank wastes cover a broad range of chemical compositions with a diversity of physical and rheological properties. Additionally, some of these properties are very sensitive to changes in temperature, pH, and salt concentrations. The analysis that follows is based on slurry properties alone. Although changes in viscosity with temperature are addressed, the effects of changes in temperature, pH, or salt concentrations are in general beyond the scope of this analysis.

Correlations for determining critical velocity and pressure drop resulting from friction losses in the waste-transfer system are discussed in Section 2.0. In Section 3.0, the various sources of information for the waste properties and transfer system characteristics are identified and some of the shortcomings in the data are discussed. An evaluation of the results of the analysis is included in Section 4.0, along with a description of some of the uncertainties associated with the analysis. Conclusions and recommendations are presented in Section 5.0. References cited in the text are listed in Section 6.0.

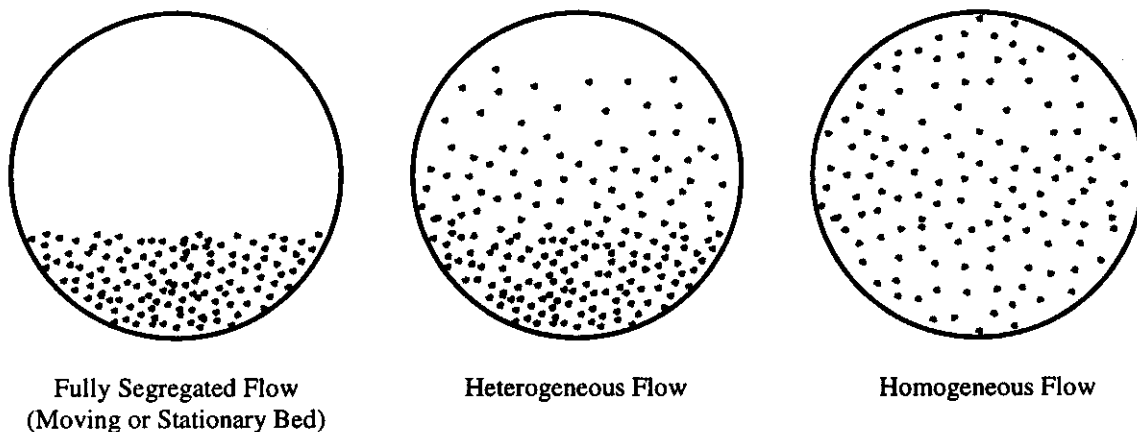
2.0 WASTE FLOW CHARACTERISTICS

Transfer of wastes from the tanks to the privatization contractor will require pumping a mixture of liquids and solids through the waste-transfer system. These mixtures are commonly referred to as suspensions or slurries. Prediction of the flow characteristics of a slurry often is more difficult than for a single-phase fluid because of the large number of variables required to describe the flow and the complexity of the solid-liquid interactions. Before the flow characteristics of the transfer system can be analyzed, the behavior of the slurry must be classified.

2.1 CLASSIFICATION OF SLURRIES

A large number of empirically based correlations have been developed to describe the behavior of slurries as they flow through piping systems. The correlations of interest in this analysis address flows with solids that are fully suspended. These correlations are divided broadly into two categories that treat the slurry as either a homogeneous mixture or a heterogeneous mixture. Within each of these categories, the slurry may be treated as either a single-phase or a multi-phase mixture. Although a slurry may be classified as either homogeneous or heterogeneous, it is far more common for the slurry to exhibit characteristics of both. Figure 2-1 shows a comparison of the distribution of solids for different categories of flow. The proper classification of slurry behavior is essential if the flow characteristics of the transfer system are to be modeled accurately.

Figure 2-1. Flow Regimes.



Homogeneous flow describes the condition where the solid particles are distributed uniformly throughout the cross section of the pipe. An inherently homogeneous flow generally is found in slurries containing high concentrations of solids and small particle sizes (i.e., particle diameter $<50 \mu\text{m}$). Homogeneous behavior also can be found in slurries with larger particle sizes where the solid-to-liquid density ratio is small or in slurries that flow with sufficient turbulence to distribute the particles evenly. The effect of the small particles in a homogeneous slurry is to increase the viscosity of the mixture as the solids concentration increases. At higher

concentrations, the mixture will likely begin to exhibit non-Newtonian behavior. The point at which non-Newtonian behavior begins is difficult to predict because it is a function of the particle size distribution, particle shape, solids concentration, solid-to-liquid density ratio, and particle surface forces. Another characteristic of homogeneous flow is that the viscous or turbulent forces are dominant and the particle inertial forces are small. In the case of inherently homogeneous slurries, this characteristic results in a slurry that can flow at very low velocities without settling and allows the slurry to be treated as a single-phase fluid at all flow velocities. Examples of slurries that are inherently homogeneous include concentrated suspensions of fine limestone, paint, and ink.

Heterogeneous flow occurs when the solid particles are not distributed evenly across the cross section of the pipe although the particles are in suspension. The flow is characterized by a gradient in concentration along the vertical axis of the pipe. Heterogeneous flow generally is found in slurries with low concentrations of solids and larger particle sizes. It also can be found in slurries with smaller particle sizes when the solid-to-liquid density ratio is large. The larger particles found in heterogeneous flow do not have a significant effect on the viscosity of the mixture, and the flow is usually Newtonian in nature. Slurries with a broad distribution of particle sizes may exhibit non-Newtonian behavior as the fine particles change the behavior of the liquid phase and the larger particles are carried along in a heterogeneous suspension. The particle inertial forces are more significant in heterogeneous flow because of the mass of the particles; therefore, the liquid and solid phases of the mixture are considered separately. Examples of slurries that behave heterogeneously include mixtures of sand or coal in water.

Although no single waste characteristic can be used to classify the behavior of a slurry, the particle size and the ratio of particle density to liquid density are important parameters. Documented particle size distributions for the double-shell tanks (DST) cover a wide range of values. Some tanks, such as 241-SY-101, contain particles that range from $<1 \mu\text{m}$ to $>1000 \mu\text{m}$ (*Results of Viscosity Measurements of Tank 241-SY-101 Samples* [O'Rourke 1999]). Similarly, Tank 241-AW-101 covers a range of $\sim 6 \mu\text{m}$ to $>1000 \mu\text{m}$ (*Results of Dilution Studies with Waste from Tank 241-AW-101* [HNF-4964]). Tank 241-AZ-101 has a range of $5 \mu\text{m}$ to $225 \mu\text{m}$ (*Tank Characterization Report for Double-Shell Tank 241-AZ-101* [WHC-SD-WM-ER-410]). Other tanks have narrower distributions, such as 241-AN-104 with a range of $<1 \mu\text{m}$ to $40 \mu\text{m}$ (*Results of Dilution Studies with Waste from Tank 241-AN-104* [HNF-3352]) and 241-AN-105 with a range of $<1 \mu\text{m}$ to $40 \mu\text{m}$ (*Results of Dilution Studies with Waste from Tank 241-AN-105* [HNF-SD-WM-DTR-046]). Based on the data available, the range of particle sizes is so broad that the waste slurry that will be transferred cannot be classified as exhibiting either homogeneous or heterogeneous behavior. The waste will exhibit some combination of homogeneous and heterogeneous behavior depending on the mixture velocity, the ratio of particle density to liquid density, and the particle size distribution by volume.

Within the categories of homogeneous and heterogeneous slurries, the rheology of the slurry can be classified as either Newtonian or non-Newtonian. Viscosity is a measure of a fluid's resistance to deformation and is defined as the ratio of shear stress to shear rate. For a Newtonian fluid, the ratio of shear stress to shear rate is constant over a defined range of shear rates. In a Newtonian slurry, the viscosity does not change as the flow velocity in the transfer system pipe changes. Typical non-Newtonian slurries are characterized by viscosities that increase (shear thickening) or decrease (shear thinning) with increasing flow velocity and often

have a yield strength that must be overcome before flow will occur. Viscosity measurements are not available for a majority of the tanks, and the data that are available do not necessarily reflect the condition of the waste when it will be pumped through the waste-transfer system. Some tanks will be mixed to homogenize the waste, which will then be pumped to the vitrification plant. For other tanks the supernatant will be pumped off first, the remaining solids will be dissolved, and the remaining waste will be transferred to the privatization contractor. Other tanks will be mixed together, potentially changing the rheology of the waste before it is transferred to the privatization contractor. To properly model the flow characteristics of the waste it is necessary to understand its rheology. In the absence of specific data, and given that it is extremely difficult to accurately predict the viscosity of a mixture based on its individual constituents only, some simplifying assumptions were made.

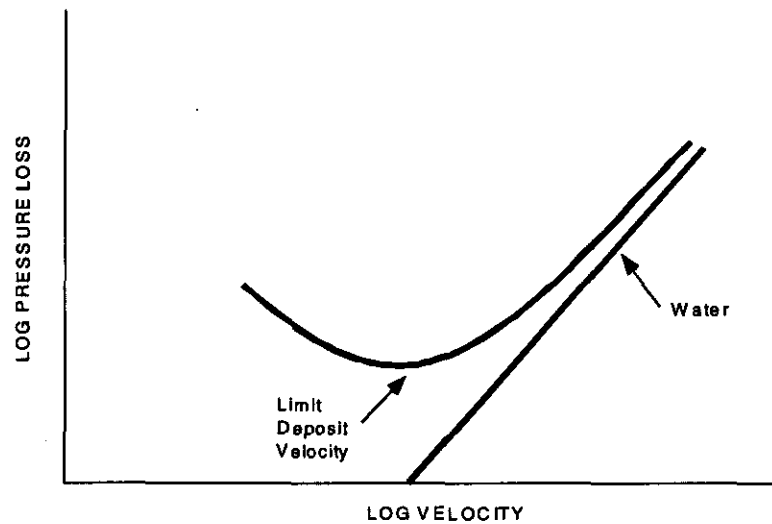
Documented viscosity measurements indicate that the waste samples are predominately Newtonian. However, some samples exhibit non-Newtonian behavior. When reviewing the non-Newtonian test data, it was noted that most of the measurements were performed on tank samples that are not representative of the condition of the waste when it will be pumped. The waste samples typically are representative of in situ tank conditions and do not duplicate the dissolution of salts and mixing of the waste that will occur before being transferred. Dissolution and mixing in the tank has the effect of reducing the solids volume percent below the levels that would be found in a sample taken from a settled, undiluted tank. As the volume percent of solids is reduced, the behavior of the waste will tend toward Newtonian behavior. The non-Newtonian behavior primarily is a result of the presence of the solids in the slurry. It is important to note that there is the possibility that some wastes may be non-Newtonian when they are transferred. As more representative sample data are obtained, it may be possible to predict non-Newtonian behavior. For the purposes of this analysis, based on the data available and the anticipated waste characteristics, the waste that will be pumped through the transfer system will be modeled as Newtonian.

In analyzing the waste-transfer system, it is necessary to determine the critical velocity and the relationship between flow rate and pressure drop. The critical velocity is the minimum velocity that should be maintained to ensure stable and reliable operation of the system. The pressure drop through the waste-transfer system is required to establish the pipe design pressure and pump head requirements. Because the number and range of variables affecting slurry flow are large and the mechanics of the solid-liquid interactions are complex, there are no purely analytical solutions available. Instead, the equations presented in this analysis are correlations based on published data that were developed to predict critical velocity and pressure drop in slurry pipelines. The correlation for critical velocity is described in Section 2.2. The correlation for predicting pressure drop is discussed in Section 2.3. For a more detailed discussion of slurry flow and the derivation of these correlations, see "The Critical Velocity in Pipeline Flow of Slurries" (Oroskar and Turian 1980); *Flow Velocity Analysis For Avoidance of Solids Deposition During Transport of Hanford Tank Waste Slurries* (HNF-2728); and "Solid-Liquid Flow Slurry Pipeline Transportation" (Wasp et al. 1977).

2.2 CRITICAL VELOCITY

In heterogeneous flow, operation of the waste-transfer system below a specified minimum velocity will result in solids settling or sliding along the bottom of the pipe. Continued operation below the critical velocity eventually will lead to plugging of the pipe. The critical velocity for a heterogeneous slurry is the velocity at which the solids begin to settle and is called the limit deposit velocity. The limit deposit or deposition velocity of a heterogeneous slurry is usually in the turbulent flow regime. Figure 2-2 shows the relationship between pressure loss and flow velocity for a heterogeneous slurry.

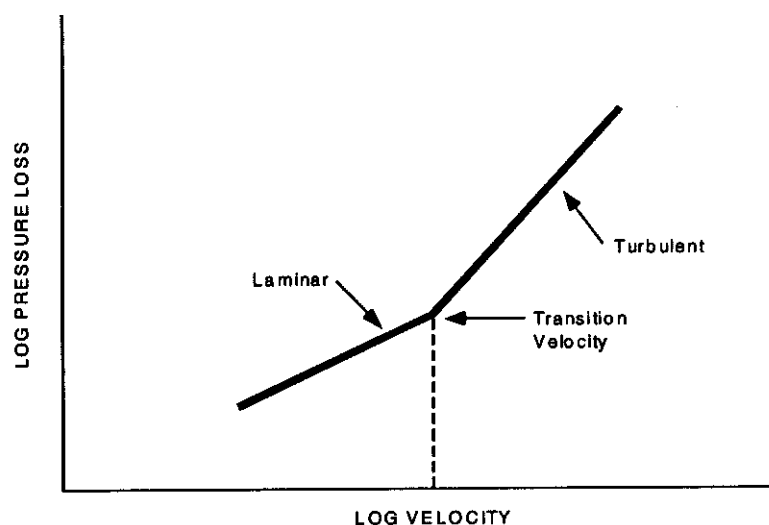
Figure 2-2. Pressure Loss vs. Velocity for a Heterogeneous Slurry.



In homogeneous flow, the critical velocity corresponds to the point at which the flow transitions to a turbulent flow regime. In purely homogeneous flow, deposition of solids does not occur below some minimum velocity. Nevertheless, it is desirable to maintain flow above the laminar-to-turbulent transition region (Reynolds number approximately > 4000) to ensure stable flow. In this analysis, both the limit deposit velocity and the transition velocity are calculated, and the greater of the two values is identified as the critical velocity. Figure 2-3 shows the relationship between pressure loss and flow velocity for an inherently homogeneous slurry.

The concept of critical velocity is very useful in designing slurry transport systems. If the flow rate is less than the critical velocity, buildup of solids will occur and the line will possibly plug. If flow is maintained above the critical velocity, plugging of the transfer line as a result of solids settling out will be avoided. All that remains is to predict the critical velocity accurately. When working with heterogeneous slurries, a large number of correlations are available to predict the limit deposit velocity. Most of the correlations are derived empirically for a specific range of slurry properties. If the correlations are applied to slurries other than those for which they were derived, there generally is poor agreement between the various correlations and actual operating and test data. When designing a transport system for a known slurry composition, it is always preferable to determine the critical velocity by test. In the absence of actual test data, some correlations have been shown to be more accurate than others. An evaluation of seven different correlations documented by HNF-2728 found that the Oroskar and Turian (1980) correlation was

Figure 2-3. Pressure Loss vs. Velocity for a Homogeneous Slurry.



the most sophisticated and yielded the most conservative results over the range of waste characteristics expected at the Hanford Site. Figure F-15 of Appendix F compares a number of correlations in addition to comparisons given in HNF-2728.

The Oroskar and Turian (1980) empirical equation was developed as an extension of their semitheoretical correlation to improve the accuracy of the critical velocity prediction. It is based on extensive slurry flow research and was slightly more accurate than the semitheoretical correlation. The Oroskar and Turian (1980) correlation for limit deposit velocity is given by

$$v_{cr} = \sqrt{gd_p \left(\frac{\rho_s}{\rho_L} - 1 \right)} 1.85C_s^{0.1536} C_L^{0.3564} \left(\frac{d_p}{D} \right)^{-0.378} \left(\frac{D\rho_L \sqrt{gd_p \left(\frac{\rho_s}{\rho_L} - 1 \right)}}{\mu_L} \right)^{0.09} \chi^{0.30} \quad (1)$$

where

- v_{cr} = critical velocity
- g = gravitational acceleration
- d_p = mean particle diameter
- ρ_s = solids density
- ρ_L = liquid density
- C_s = solids volume fraction
- C_L = liquid volume fraction = $1 - C_s$
- D = pipe inside diameter
- μ_L = liquid viscosity
- χ = eddy velocity factor.

In formulating their correlation for critical velocity, Oroskar and Turian (1980) developed the concept that the turbulent eddy currents in the slurry flow are responsible for keeping the solids suspended. Because only a fraction of the eddies are effective in lifting the particles, the eddy velocity factor was derived to account for this effect and is given by

$$\chi = \frac{2}{\sqrt{\pi}} \left[\frac{2\gamma}{\sqrt{\pi}} \exp\left(\frac{-4\gamma^2}{\pi}\right) + \frac{\sqrt{\pi}}{2} \left(1 - \operatorname{erf}\left(\frac{2\gamma}{\sqrt{\pi}}\right)\right) \right] \quad (2)$$

where

$$\gamma = \frac{v_s}{v_{cr}} \quad (3)$$

v_s = particle hindered settling velocity.

The hindered settling velocity, v_s , is the settling velocity of a particle that takes into account collisions with other particles, and is a function of the concentration of particles in the slurry:

$$v_s = v_{\infty} (C_L)^n \quad (4)$$

where

$$n = 4.65 - \frac{2.32}{\sqrt{0.5\pi}} \int_{-\infty}^{\infty} \exp\left(\frac{-\log(\operatorname{Re}_p)^2}{0.5}\right) d(\log(\operatorname{Re}_p)) \quad (5)$$

and the particle Reynolds number is defined as

$$\operatorname{Re}_p = \frac{d_p v_{\infty} \rho_L}{\mu_L} \quad (6)$$

The analytical expression for "n" given in Equation 5 was derived by Estey and Hu (HNF-2728) as an approximation to the graphical relation given by Oroskar and Turian (1980).

For slurry flow, the Reynolds number is approximated assuming that the particles are spheres.

The terminal (free fall) settling velocity, v_{∞} , of a spherical particle in a stagnant unbounded liquid is given by

$$v_{\infty} = \sqrt{\frac{\frac{4}{3} d_p \left(\frac{\rho_s}{\rho_L} - 1\right) g}{C_D}} \quad (7)$$

The drag coefficient, C_D , for a sphere is given by:

$$C_D = \left[\left(\frac{24}{Re_p} \right)^{1/2} + 0.34035 \left(Re_p^{0.06071} + \frac{1}{1.72013 + 0.018Re_p} \right) \right]^2 \quad (8)$$

Note that an iterative procedure is required to obtain v_∞ because it is implicitly defined in Equation 7 through the particle Reynolds number (Equation 6) as used in Equation 8 for the particle drag coefficient.

The above empirical correlation (Equation 1) is one of the better correlations for predicting critical velocity, with an overall 21.8% root mean square deviation compared to experimental data (Oroskar and Turian 1980). Note that the critical velocity, v_{cr} , is implicitly defined in Equation 1 with the introduction of Equations 2 and 3. Hence, an iterative procedure is required to obtain the critical velocity.

2.3 PRESSURE-DROP CORRELATIONS

As with critical velocity, there are a number of correlations that have been proposed for predicting pressure drop in slurry transport systems. The correlations generally fall into the semitheoretical or empirical categories. The weakness with most of the correlations is their inability to predict the pressure drop accurately for slurries with a wide range of slurry properties and flow regimes. Wasp et al. (1977) proposed a correlation for calculating pipeline pressure loss for a slurry with a broad band particle size distribution that accounts for a combination of heterogeneous and homogeneous flow. The correlation employs the concept of a "two-phase vehicle" to describe the homogeneous portion of the flow. It postulates that the concentration and size distribution of the particles that are found at the top of the pipe are equally distributed throughout the pipe. These equally distributed particles and the carrier liquid form the homogeneous "vehicle" portion of the slurry. The method adjusts the density and viscosity of the carrier liquid to account for the effects of the smaller particles in the homogeneous portion of the slurry. The particles that are not equally distributed across the pipe comprise the heterogeneous portion of the slurry. These particles are suspended and carried along by the homogeneous "vehicle" portion of the slurry. Using this concept, the total pressure drop in the system is calculated by summing the pressure-drop contribution from the homogeneous "vehicle" and the additional pressure drop resulting from the remaining heterogeneous suspension.

The Wasp et al. (1977) correlation defines a criterion for determining which portion of the slurry is treated as part of the "vehicle" based on experimental data from "Turbulent Transfer Mechanism and Suspended Sediment in Closed Channels" (Ismail 1952). The correlation considers a slurry to be homogeneous when the ratio of the solid concentration (ϕ) at $0.08D$ from the top of the pipe to the solid concentration (ϕ_s) at the pipe centerline is at least 0.8. Wasp et al. (1977) provided the following empirical relationship for determining the concentration ratio for each particle size in the particle size distribution that joins the carrier liquid to form the homogeneous "vehicle" phase:

$$\log_{10} \frac{\phi_{vj}}{\phi_{sj}} = - \left(\frac{1.8v_{\infty j}}{\beta \kappa v^*} \right) \quad (9)$$

where

ϕ_{sj} = solid concentration at pipe centerline for j^{th} particle size of the particle size distribution

ϕ_{vj} = solid concentration for j^{th} particle size of the particle size distribution that joins with the carrier liquid to form the homogeneous “vehicle” portion of the slurry

β = constant for a given particle size, varying from 1.0 for fine particles less than approximately 100 μm to 1.3 for coarse particles on the order of 0.1 mm. A value of $\beta = 1.0$ was used in this analysis.

κ = von Karman boundary-layer constant, varying from 0.4 for clear liquids to 0.25 for large solids concentrations. A value of $\kappa = 0.4$ was used in this analysis.

The terminal settling velocity, $v_{\infty j}$, for the j^{th} particle size in the particle size distribution is calculated from Equation 7 but using the “vehicle” modified carrier liquid density (ρ_v) and viscosity (μ_v) properties as defined below.

The friction velocity, v^* , is defined as

$$v^* = v \sqrt{\frac{f_w}{8}} \quad (10)$$

where

$$f_w = f_v + f_d \cos \theta$$

f_v = homogeneous “vehicle” contribution to the friction factor

$f_d = f_w - f_v$ = heterogeneous contribution to the friction factor

v = bulk velocity of the slurry.

The $\cos \theta$ factor accounts for pipe slope from horizontal per *Applied Fluid Dynamics Handbook* (Blevins 1984).

The homogeneous “vehicle” friction factor, f_v , is calculated using an equation developed in “Friction Factor Equation Spans All Fluids Regimes” (Churchill 1977) which is valid for the laminar, transition, and turbulent flow regimes and is given by

$$f_v = 8 \left[\left(\frac{8}{\text{Re}_{\text{pipe}}} \right)^{12} + (A + B)^{-3/2} \right]^{1/12} \quad (11)$$

where

$$A = \left[2.457 \ln \left(\frac{1}{\left(\frac{7}{Re_{\text{pipe}}} \right)^{0.9} + 0.27 \left(\frac{\varepsilon}{D} \right)} \right) \right]^{16} \quad (12)$$

ε = roughness of pipe

$$B = \left(\frac{37530}{Re_{\text{pipe}}} \right)^{16} \quad (13)$$

The pipe Reynolds number, Re , is calculated as follows:

$$Re_{\text{pipe}} = \frac{\rho_v v D}{\mu_v} \quad (14)$$

where

ρ_v = density of the homogeneous “vehicle” phase of slurry
 μ_v = viscosity of the homogeneous “vehicle” phase of slurry.

The viscosity of the “vehicle”, μ_v , is calculated using the correlation given in “Transport Characteristics of Suspensions: Part VIII. A Note on the Viscosity of Newtonian Suspension of Uniform Spherical Particles” (Thomas 1965) as

$$\mu_v = \mu_L (1 + 2.5\Phi_v + 10.05\Phi_v^2 + 0.00273 \exp(16.6\Phi_v)) \quad (15)$$

where

Φ_v = total “vehicle” solids concentration = $\sum \phi_{vj}$, summed over each particle size in the particle size distribution.

The viscosity of the carrier liquid, μ_L , is described in Appendix A and is given as

$$\mu_L = \mu_{\text{water}} \left[x_{\text{salt}} \left(1 + 1.071 \left(\frac{\rho_L}{\rho_{\text{water}}} - 1 \right) \right) + x_{\text{caustic}} \exp \left(\left(7.143 \left(\frac{\rho_c}{\rho_{\text{water}}} - 1 \right) \right)^{1.15} \right) \right] \quad (16)$$

where

μ_L = viscosity of carrier liquid

μ_{water} = viscosity of water at temperature

ρ_{water} = density of water at temperature

ρ_L = density of carrier liquid

x_{salt} = the fraction of dissolved solids composed of sodium and other salts = 0.9

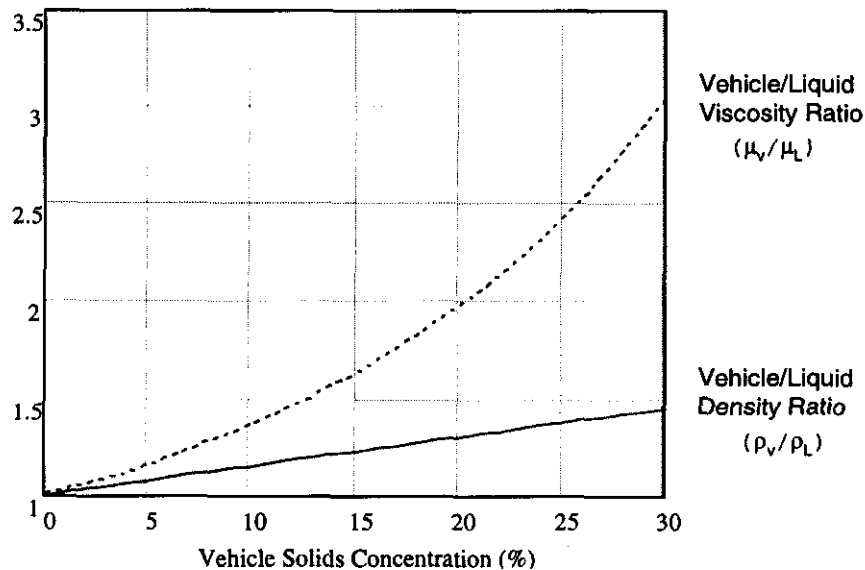
x_{caustic} = the fraction of dissolved solids composed of sodium hydroxide = 0.1.

The "vehicle" liquid density is given by

$$\rho_v = \rho_s \Phi_v + \rho_L(1 - \Phi_v)$$

A graph of "vehicle" to liquid viscosity and density ratios versus solids content is shown in Figure 2-4.

Figure 2-4. "Vehicle" to Liquid Density and Viscosity Ratios vs. "Vehicle" Solids Concentration.



The determination of pressure loss in a piping system using the Wasp et al. (1977) correlation involves an iterative process. The first iteration assumes that all of the solids form part of the "vehicle." The head loss in the system that is attributable to the "vehicle" is calculated using Darcy's formula

$$\Delta h_v = \frac{h_L}{L} = \frac{f_v v^2}{2gD} \quad (17)$$

where

f_v = pipe friction factor (Darcy) for "vehicle" (Equation 11)

Δh_v = head loss per unit length of pipe attributed to homogeneous "vehicle"

h_L = head loss

L = length of pipeline.

The ratio of ϕ_v/ϕ_s for each size fraction of particles is calculated and multiplied by the volume percent solids of that size fraction to determine the "vehicle" portion of the slurry. The balance of the solids is assumed to be part of the heterogeneous suspension. A second iteration is performed to determine the head loss attributable to the "vehicle." The head loss attributable to the heterogeneous portion of the slurry is calculated for each size fraction of particles using the correlation in "The Hydraulic Transportation of Coal and Other Materials in Pipes" (Durand 1953) as

$$\Delta h_{dj} = 82 \cdot \Delta h_{carrier} \phi_{dj} \left[\frac{gD \left(\frac{\rho_s}{\rho_L} - 1 \right)}{v^2 \sqrt{C_{Dj}}} \right]^{1.5} = \frac{f_{dj} v^2}{2gD} \quad (18)$$

where

f_{dj} = heterogeneous contribution to the friction factor for j^{th} particle size in particle size distribution

$\Delta h_{carrier}$ = head loss per unit length attributable to "vehicle" relative to carrier liquid, i.e.,

$$= \Delta h_v \frac{\rho_v}{\rho_L}$$

ϕ_{dj} = heterogeneous volume percent solids for j^{th} particle size fraction of the particle size distribution

$$= \phi_{sj} - \phi_{vj}$$

C_{Dj} = drag coefficient of j^{th} particle size using Equation 8 with carrier liquid density (ρ_L) and viscosity (μ_L) properties.

The head losses for each of the size fraction of the particle size distribution are added together to determine the total head loss attributable to the heterogeneous portion of the slurry. This is added to the head loss for the "vehicle" to obtain the total friction head loss. The "vehicle" head loss must be converted to the carrier liquid before adding it to the heterogeneous portion of the slurry to maintain a consistent basis. Additional iterations are performed until the difference in head loss between successive iterations converges within some small tolerances. The described methodology has been modified slightly for this analysis to include the effects of elevation changes along the pipe route. The equation for pressure loss includes a $\cos \theta$ factor to account for friction losses as the slurry travels through an inclined pipe per Blevins (1984), see Equation 10 above. The resulting total head loss, including the change in elevation, is given by

$$h_L = \frac{Lv^2}{2gD}(f_w) + \left[\left(\frac{\rho_s}{\rho_L} - 1 \right) C_s + 1 \right] L \sin \theta \quad (19)$$

where

h_L = head loss

θ = angle of inclination of pipeline from horizontal.

Because some transfer routes contain a combination of carbon steel and stainless steel pipe lengths, the pressure drop is calculated for each respective length with the corresponding pipe roughness for carbon and stainless steel (see Section 3.2.2) to obtain the total head (or pressure) loss.

2.4 EFFECT OF VARIATIONS ON CRITICAL VELOCITY AND PRESSURE DROP

The variables required to determine critical velocity and pressure drop have been identified in Sections 2.2 and 2.3, respectively. While it is evident that the equations are nonlinear, it is not obvious what effect a change in these variables will have on the critical velocity and pipeline pressure drop. Table 2-1 lists the key variables and indicates the general impact that a variation will have on critical velocity and pipeline pressure drop. The impact on differential pressure assumes that the flow velocity is above the critical velocity.

Table 2-1. Key Variables.

Variable	Change	Critical Velocity	Differential Pressure
Liquid density	raise	↓	↑
	lower	↑	↓
Liquid viscosity	raise	↓	↑
	lower	↑	↓
Solids size	raise	↑	↑
	lower	↓	↓
Solids density	raise	↑	↑
	lower	↓	↓
Solids volume fraction	raise	↑	↑
	lower	↓	↓
Pipe inside diameter	raise	↑	↓
	lower	↓	↑
Pipe roughness	raise	n/a	↑
	lower	n/a	↓
Pipe length	raise	n/a	↑
	lower	n/a	↓
Pipe incline to horizontal	raise	n/a	↑
	lower	n/a	↓
Flow velocity	raise	n/a	↑
	lower	n/a	↓

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3.0 WASTE TRANSPORT BEHAVIOR

The rheological and physical properties of the waste and the physical characteristics of the transfer system define the behavior of the waste during transport. The correlations developed for predicting pipeline pressure drop and estimating critical velocity are based on a combination of heterogeneous and homogeneous flow. The correlations predict the waste behavior based on the individual characteristics of the liquid and solid components of the slurry. Waste properties include liquid density, liquid viscosity, solids density, solids size and distribution, and solids volume fraction. A discussion of these properties is provided in Section 3.1. Physical characteristics of the waste-transfer system include pipe inside diameter, length of pipe in the transfer route, elevation changes in the transfer route, and surface roughness of the inside of the pipe. These characteristics are discussed in Section 3.2. The pressure ratings of the existing pipelines are also identified in Section 3.2.

3.1 WASTE PROPERTIES

Accurate measurement of waste properties is essential to the accurate prediction of critical velocity and system pressure losses. The group of waste transfers identified in Phase 1B of the *Tank Waste Remediation System Operation and Utilization Plan to Support Waste Feed Delivery* (HNF-SD-WM-SP-012) focuses on transfers from DSTs in the 200 East Area. An initial review of characterization data for the DSTs was performed to determine if sufficient data were available to support an analysis of waste transfers based on tank specific data. It was found that supernatant density data were available for most of the tanks. Solids volume percent data were available from the Hanford Tank Waste Operating Simulator (HTWOS) for all of the Phase 1B transfers. HTWOS maintains an inventory of the liquid and solid components of the waste as it is transferred between tanks. Solids density data were available for ~40% of the tanks. Data for supernatant viscosity and solids diameter were available for <30% of the DSTs. Based on the lack of necessary input data, Process Engineering was requested to provide an estimate of slurry properties. The estimate, attached in Appendix A, provides a summary of nominal and bounding in situ waste properties to be used as a basis for the analysis of the waste-transfer system. The estimated properties are summarized in Table 3-1.

Table 3-1. In Situ Waste Properties.

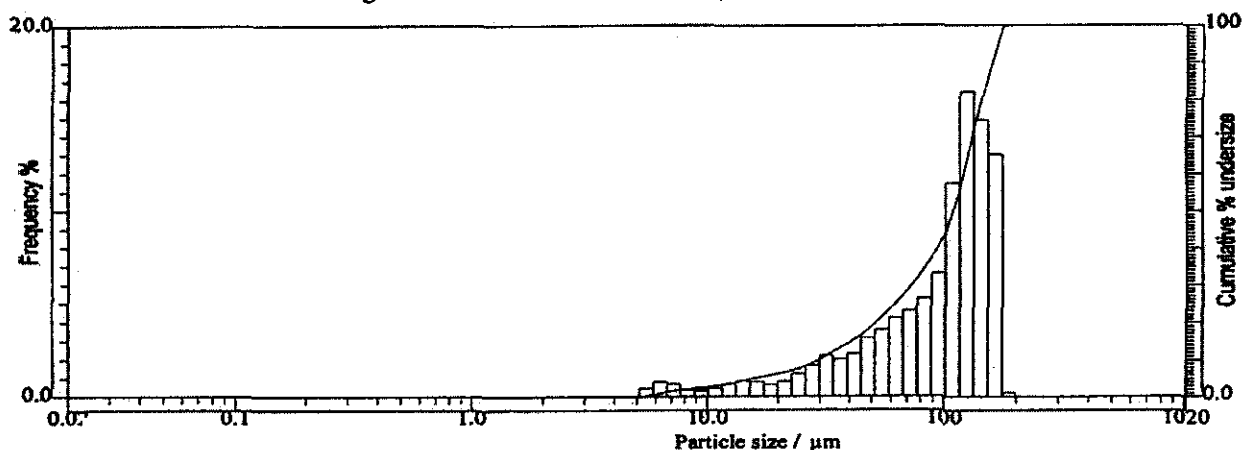
Property	Nominal	Bounding
Median particle size (d_p)	40 μm	400 μm
Volume percent solids (C_s)	15%	30%
Liquid density (ρ_L)	1.21 kg/L	1.46 kg/L
Solids density (ρ_s)	2.50 kg/L	3.00 kg/L
Liquid viscosity (μ_L)	*	*

* The viscosity of the carrier liquid is a function of the density of the carrier liquid, water, and the fraction of dissolved salts and caustic. The equation for viscosity is provided in Appendix A.

Some of the properties listed in Table 3-1 (i.e., particle size and liquid density) are based on actual test data. Other properties (i.e., volume percent solids, solids density, and viscosity) are based on known correlations or engineering judgment. It should be noted that the nominal and bounding cases are not defined quantitatively. Nominal properties are intended to reflect an average tank, not any specific tank. Likewise, the bounding case does not envelop the most extreme values of the individual properties listed.

The specified particle size is the median value from a distribution of particle sizes. The analysis assumes that the distribution of particle sizes approximates a log-normal curve. In performing the pressure-drop analysis, the shape of the particle distribution remains the same. Only the median value of the distribution changes with changing median particle size. A graph of particle size distribution for Tank C-104 is shown in Figure 3-1. The shape of the distribution curve is typical of the distribution used in the analysis.

Figure 3-1. C-104 Particle Size Distribution.



3.2 TRANSFER SYSTEM CHARACTERISTICS

3.2.1 Pipeline Diameter, Length, and Elevation Changes

The HTWOS models for Cases 3S4 and 3S6D were used to identify all waste transfers in Phase 1B. Additionally, a sampling of transfer routes from Case 3S6D, Phase 2 were included in the list of waste transfers. HTWOS identifies the source and receiver tank for each transfer but does not provide information related to the actual route that the waste travels during the transfer. This analysis focused on the transfer routes in the 200 East Area. The transfer routes that were analyzed are identified in Figure B-1 of Appendix B. Pipeline routing, pipe diameter, length of pipe for each transfer route, and elevation changes were identified from the physical piping drawings and from a previous study documented in HNF-2938, *Evaluation of 241-AN Tank Farm Supporting Phase 1 Privatization Waste Feed Delivery*. All pipelines in the transfer routes identified were found to be 80 mm (3-in.) nominal-diameter, schedule-40 pipe. Transfer routes for pipelines that have not been installed were based on preliminary information from the

projects. For the purpose of this analysis, it was assumed that all new pipe will be 80 mm (3-in.) nominal-diameter, schedule-40 pipe.

Equivalent pipe lengths for individual transfer routes are shown in Appendix D. Equivalent pipe lengths are used to simplify the analytical model for determining pressure drop while accounting for the friction losses in pipe fittings and valves. Equivalent lengths are based on the number of lineal feet of pipe in each transfer route plus the number of feet of pipe that will provide a pressure drop equivalent to the pressure drop that would be experienced through a pipe fitting or valve. Because the future configuration of fittings and valves in the pump and valve pits is not well defined, a standard configuration of valves and fittings was assumed for these locations. Transfers through valve pits were modeled by assuming that the flow passes through two 3-way valves (run), one 3-way valve (branch), three Plutonium-Uranium Extraction (PUREX) connectors, and five long radius elbows. Transfers through pump pits were modeled by assuming that the flow passes through one 3-way valve (run), two PUREX connectors, and two long radius elbows. If additional in-line components are present, then a greater pressure drop through the system would be expected. Equivalent pipe lengths were determined using the methodology described in *Flow of Fluids Through Valves, Fittings, and Pipes* (Crane 1981). The interface with the privatization contractor requires that the transferred waste move some distance within the vitrification facility. The equivalent lengths identified in the privatization contractor's calculations have been included in the lengths of pipe listed in Appendix D for all transfers to the vitrification facility.

Changes in elevation between end points of the transfer lines were identified from HNF-2938, physical piping drawings, preliminary project drawings, and the privatization contractor's calculations. The elevations of end points of the transfer line were determined from the elevations of the nozzles inside the pump or valve pits. Elevation changes are identified in Appendix D.

3.2.2 Pipeline Roughness

All pipelines in the transfer routes were found to be constructed from either carbon steel (ASTM A106 Grade B or A53 Type S, Grade B) or stainless steel (ASTM A312). Pipeline material for pipes that have not been installed were based on preliminary information from Projects W-314 and W-521. For the purpose of this analysis, it was assumed that all new pipe will be stainless steel.

The surface roughness of the interior of a pipe can be affected by corrosion and scale buildup because of the chemistry of the waste and flush water. The interior surface roughness can also be affected by erosion caused by abrasive solids in the waste and high waste flow rates. As the interior surface of the pipe deteriorates it becomes irregular and rough, and there may be a simultaneous reduction in the thickness of the pipe wall. Although the waste chemistry is controlled to reduce the effects of corrosion and scale buildup, waste-transfer lines have failed in the past. The metallurgical analysis of a failed jumper in the 241-A-B valve pit in 1984 (*Metallurgical Analysis of Leak Failure of 241-A-B Valve Pit Jumper* [WHC-SD-RE-TI-148]) is a good example of the combined effects of erosion/corrosion on the interior of a pipe. The photographs in WHC-SD-RE-TI-148 document the rough surface condition of the pipe even at locations where the pipe did not fail. The effects of this increasing surface roughness can change

the flow characteristics of a pipe system significantly. Because there are no data documenting the condition of the interior of the pipelines, surface roughness values were based on an evaluation by Anantamula and Divine (1999), which is contained in Appendix C. For the purpose of this analysis, the values shown in Table 3-2 were used.

Table 3-2. Pipe Surface Roughness Estimates.

Pipe Material	Surface Roughness	
	(μm)	(mil)
Carbon Steel , EOL (50 yr)	3810	150
Stainless Steel, EOL (40 yr)	254	10
Stainless or Carbon Steel, New	51	2

EOL = end of life.

Erosion is expected to increase corrosion near elbows in the pipeline, however, the increase in corrosion is a result of removal of corrosion products. Under these conditions, a significant increase in the friction factor is not expected even though a rough uneven surface is produced. Recent laboratory studies indicated that even at velocities approaching 4.6 m/s (15 ft/s), corrosion products are not removed from the steel surface and, therefore, the friction factors at the elbows are not expected to be significantly different from the straight sections of the pipe. As indicated in Appendix C, the surface roughness values reported are considered conservative for carbon steel since 98% relative humidity and lack of inhibiting effect from nitrite are assumed.

3.2.3 Pipeline Design Pressure

Most of the existing pipelines that will be used for waste retrieval have a design pressure of 2.7 MPa (400 lbf/in²). Some carbon steel lines in AN and AW tank farms have design pressure ratings of 1.9 MPa (275 lbf/in²) but have been hydrotested to 3.1 MPa (450 lbf/in²) (HNF-2938). The pressure rating of the pipe provides an upper bound that limits the flow velocity through the system and the allowable shut-off head of the transfer pump.

4.0 RESULTS

This section presents the results of the critical velocity and pressure-drop calculations. A general discussion of waste parameters investigated is provided in Section 4.1. Results of critical velocity and pressure-drop analyses are given in Sections 4.2 and 4.3, respectively. The sensitivity of the waste and pipe parameters on the pressure drop at critical velocity is presented in Section 4.4. Uncertainties in the waste and pipe parameters are discussed in Section 4.5.

4.1 GENERAL

The critical velocity and transfer line pressure drop were calculated for each of the transfers identified under Cases 3S4 and 3S6D of the HTWOS model for Phase IB waste feed delivery. Case 3S4 was chosen because it is consistent with the latest multi-year work plan. Case 3S6D also was analyzed to evaluate the effects of the changes between the two cases. Approximately 30% of the transfer routes were changed when going from Case 3S4 to 3S6D. The calculations were performed using the length of pipe specific to each transfer route, pipe wall roughness values for new and old pipe at end of life (EOL), and different combinations of waste characterization data. Equivalent pipe lengths were used to account for the various valves and fittings in the transfer routes, and changes in elevation were factored into the calculations. The transfer pipe within the privatization contractor's facility also was included in the pipe routes. A total of 52 different combinations of waste properties and transfer routes were analyzed. These different combinations served to confirm that the analytical model was providing reasonable results and to provide an indication of the sensitivity of the transfer system to variations in waste properties. Of the 52 different combinations, three sets of data were identified for further evaluation. These three sets of data are described below.

Data Set 1

The first set of data is based on the in situ waste properties identified as the bounding case in Table 3-1. This case represents a set of waste properties that would be considered conservative for all of the waste-transfer routes. The intent was to determine whether one set of data could be used as the basis for specifying all new transfer system equipment. If it could be shown that the required critical velocity and associated pressure drop were within the design pressure limits of the pipe, no further evaluation of the pipe would be required. This set of data would then define the design bases for the waste-transfer system. The waste for this case has the following properties (Case 3b, Group 2 Properties – Tables D-11 and D-25 in Appendix D for HTWOS Cases 3S4 and 3S6D, respectively):

- Liquid Density 1.2 kg/L
- Liquid Viscosity 0.01 to 0.1 g/cm-s (1-10 cP)
- Volume Percent Solids 30%
- Solids Density 3.0 kg/L
- Solids Median Size 400 μ m
- Temperature 10 °C to 60 °C

Data Set 2

The second set of data is based on the same waste properties identified in the bounding case, with the exception of the liquid density and the volume percent solids. The liquid density was reduced to 1.2 kg/L to reflect the actual waste characteristics more accurately. The volume percent solids was reduced based on values predicted by HTWOS. Cases 3S4 and 3S6D were reviewed to determine the maximum volume percent solids that would be present in a slurry transfer. This value was increased by a factor of two to account for any density gradient that might exist during a transfer because of inadequate mixing or stratification in the tank. The waste for this case has the following properties (Case 4, Group 2 Properties – Tables D-14 and D-28 in Appendix D for HTWOS Cases 3S4 and 3S6D, respectively):

- Liquid Density 1.2 kg/L
- Liquid Viscosity 0.01 to 0.1 g/cm-s (1-10 cP)
- Volume Percent Solids 15%
- Solids Density 3.0 kg/L
- Solids Median Size 400 μm
- Temperature 10 °C to 60 °C

Data Set 3

Low-activity waste (LAW) feed transfers to the privatization contractor are primarily supernatant and have very low solids content. However, as part of the waste-staging process, waste with higher volume percent solids are transferred between tanks that are subsequently used for LAW feed to the privatization contractor. The third set of data is based on the same values used in the second case, with the exception of the volume percent solids. This property was revised to simulate performing a transfer between tanks. This was considered a bounding condition for a tank-to-tank transfer. The waste properties for this case are as follows (Case 4, Group 1 Properties – Tables D-14 and D-28 in Appendix D for HTWOS Cases 3S4 and 3S6D, respectively):

- Liquid Density 1.2 kg/L
- Liquid Viscosity 0.01 to 0.1 g/cm-s (1-10 cP)
- Volume Percent Solids 10%
- Solids Density 3.0 kg/L
- Solids Median Size 400 μm
- Temperature 10 °C to 60 °C

The three sets of data were compared to design criteria for ongoing or recently completed projects to determine if there were any inconsistencies. The waste properties for Projects W-058, W-211, and W-314 are shown in Table 4-1. Each of these projects has or is in the process of upgrading the waste-transfer system. The waste properties for the three sets of data in this analysis were found to be similar to the project design criteria, with the exception of the solids density data and the data that Project W-058 used for non-Newtonian flow.

The waste characteristics, transfer line properties, and results of the analyses are tabulated in Appendix D. The first data set is presented in Tables D-11 and D-25 (Case 3b, Group 2 Properties) for HTWOS Cases 3S4 and 3S6D, respectively. The second and third data sets are

presented in Tables D-14 (Case 4, Group 2 Properties) and D-28 (Case 4, Group 1 Properties) for HTWOS Cases 3S4 and 3S6D, respectively.

Table 4-1. Waste-Property Comparison.

Property	Project W-058		Project W-211	Project W-314
Specific gravity	1.5	1.25	1.25	1.5
Liquid density, kg/L	--	--	--	--
Liquid viscosity, g/cm-s (cP)	0.3 (30)	0.1 (10)	0.1 (10)	0.3 (30)
Volume percent solids	30%	20%	20%	30%
Solids density, kg/L	--	--	--	--
Solids size, μm	0.5 – 4000	0.5 – 4000	0.5 – 4000	--
Temperature, $^{\circ}\text{C}$	2 – 93	2 – 93	27 – 93 ^a	27 – 116
Friction factor	0.0404 ^b	--	--	--

^a10 $^{\circ}\text{C}$ to 130 $^{\circ}\text{C}$ for aging waste tanks

^bNon-Newtonian.

4.2 CRITICAL VELOCITY ANALYSIS

Critical velocity is a function of the carrier liquid density and viscosity, solids diameter, density and volume fraction, and pipe diameter. Critical velocity values are tabulated in Appendix D for each set of data. The values of critical velocity are independent of transfer pipeline length. The critical velocity is calculated using Equation 1. A comparison is also performed to ensure that the flow is above the laminar-to-turbulent transition region. The calculated value of critical velocity is adjusted by a factor, κ_{cv} , which is equal to 1.3. This factor accounts for the percent root mean square deviation from experimental data for the critical velocity correlation (Oroskar and Turian 1980) and provides a margin above the calculated value of critical velocity to ensure stable operation.

Table 2-1 shows the general effect that raising or lowering a process variable has on the critical velocity. A review of the calculated values for critical velocity in Appendix D indicates that it is more sensitive to changes in some variables than in others. Appendix F contains a number of graphs that illustrate the effects of adjusting one process variable while holding the others constant. The variable that has the most significant impact on critical velocity is the ratio of carrier liquid density to solids density. Figure F-5b shows the effect on critical velocity for changes in the carrier liquid density, particle density, and temperature for a median particle size of 400 μm with a solids concentration of 10% by volume. Figures F-6a, F-7a, and F-8a of Appendix F show that a moderate percent increase in carrier liquid density will cause a moderate percent decrease in critical velocity. Likewise, Figures F-6b, F-7b, and F-8b of Appendix F demonstrate that a moderate percent decrease in solids density will cause a moderate percent decrease in critical velocity.

The other three process variables (i.e., volume percent solids, solids diameter, and carrier liquid viscosity) require larger percentage changes to have an equivalent effect on critical velocity. Figures F-6a and F-6b of Appendix F show that large percent increases in volume percent solids are necessary to make moderate percent increases in critical velocity. Changes in solids diameter have a small impact on critical velocity. A comparison of Figures F-9a, F-9b, F-9c, and F-9d of Appendix F indicates that a large percent reduction in solids diameter is required to affect a moderate percent decrease in critical velocity. However, the Oroskar and Turian (1980) critical velocity correlation overestimates the critical velocity as the particle size decreases from 400 μm when compared to the Wasp et al. (1977) predicted pressure drop vs. flow velocity curves as shown in Figure E-6c of Appendix E. This is expected because the Oroskar and Turian (1980) correlation was based on large size particle data with a narrow particle size distribution. Note however, as shown in Figure E-6c, that this overestimate on the critical velocity does not significantly affect the pressure drop at the critical velocity. Viscosity also has a small effect on critical velocity. Figures F-6a, F-8a, F-10a, and F-10b of Appendix F show that a significant percent decrease in carrier liquid temperature and corresponding increase in viscosity per Equation 16 is necessary to cause a small percent decrease in critical velocity. The results of the analysis show that it is possible to predict the direction and approximate magnitude of the change in critical velocity when a single variable is raised or lowered, but it is difficult to predict the change when multiple variables are adjusted simultaneously.

As shown in the tables in Appendix D, the calculated values for critical velocity ranged from 0.79 m/s (2.6 ft/s) to 2.26 m/s (7.4 ft/s) for the sets of data that did not use HTWOS data for volume percent solids. The variation in critical velocity for these cases is attributable to the variation in particle size and the change in the ratio of solids density to liquid density. In general, the critical velocity decreased as the solids volume percent and solids diameter decreased.

4.3 PRESSURE-DROP ANALYSIS

The tables in Appendix D show the total equivalent length of pipe for each transfer route, the type of pipe material, and the total pressure drop at the critical velocity using values of surface roughness for old pipe at EOL and new pipe. The value for pipeline pressure drop was calculated using Equations 17, 18, and 19. The tables in Appendix D also show the maximum flow rate at a 2.7 MPa (400 lbf/in²) pipe design pressure using values of surface roughness for old and new pipe. The values for pressure drop for old and new pipe were included to provide a range of possible values that might be seen during operation. Analysis of the adequacy of the transfer system is based on the old pipe values, which represent the condition of the pipe just before the end of life. The maximum flow rate at design pressure data is based on a pipeline with a design pressure of 2.7 MPa (400 lbf/in²).

Appendix E, Figures E-1 through E-5 show waste-transfer system curves for the transfer route from Tank 241-AN-107 to the privatization contractor. Each set of curves shows the system pressure drop for a variety of waste properties at three different concentrations and at the beginning and end of the pipe design life. For each set of figures, the particle size is held constant so that the effects of changes in the other variables can be identified. The figures also identify the critical velocity, system operating pressure for the old and new pipe conditions, and

the maximum flow rate at the design pressure of the pipe. Figure E-6c shows the system pressure-drop curves at 400, 200, 100, and 40 μm median particle sizes compared to clear liquid system curve for new pipe roughness condition. Also shown is the corresponding best-estimate critical velocity curve based on Oroskar and Turian (1980) and a modified Oroskar and Turian critical velocity based on the "vehicle" concept introduced by Wasp et al. (1977). Both methods appear to overpredict the critical velocity as the median particle size decreases as discussed above. Neither of these predictive results have been verified for this range of smaller particle sizes.

Pressure drop for a slurry is a function of the flow velocity, liquid density and viscosity, solids diameter, density and volume fraction, pipe diameter, pipe length (equivalent length), change in elevation, and pipe roughness. Pressure drop in the system is calculated at the critical velocity. The effects of changing the value of a waste property on the pressure drop in the pipeline can be difficult to predict because the critical velocity also changes. A change in a variable that results in an increase in critical velocity may lead to a decrease in pressure drop. As the viscosity of the carrier liquid is lowered, the critical velocity increases, but the pressure drop in the system decreases. However, as the solids diameter is decreased, both the critical velocity and the pressure drop decrease. Similarly, as the solids density increases, the critical velocity and the pressure drop also increase. When one variable is changed, it is possible to predict the direction of the change in pressure drop if all other variables are held constant. The above effects can be seen through a careful study of the sensitivity analysis given in Appendix E and Figures F-5 through F-10 of Appendix F.

Using the first set of data based on in situ waste properties (Table D-25 in Appendix D), 48 of 63 transfer routes will exceed the rated pressure of at least a portion of the transfer line. The transfer route with the highest pressure drop will be required to operate at a pressure of 7.6 MPa (1,115 lbf/in²) at a critical velocity of 2.26 m/s (7.4 ft/s). Figure B-2 in Appendix B shows the lines that exceed the rated pressure of the pipeline.

For the second set of data (Table D-28 in Appendix D), 38 of 63 transfer routes will exceed the rated pressure of some portion of the transfer line. The route with the highest pressure drop will be required to operate at a critical velocity of 2.16 m/s (7.1 ft/s) and a pressure of 5.2 MPa (759 lbf/in²). If all of the old pipe were to be upgraded to a design pressure of 2.7 MPa (400 lbf/in²), 34 of 63 transfer routes would still exceed the rated pressure of the pipe. Figures B-3 and B-4 in Appendix B show the lines that exceed the pipeline pressure rating.

Using the third set of data (Table D-28 in Appendix D), 37 of 63 routes exceed the rated pressure of at least some portion of the transfer route. The transfer route with the highest pressure drop will be required to operate at a pressure of 4.2 MPa (618 lbf/in²) and a critical velocity of 2.07 m/s (6.8 ft/s). If all of the old pipe were to be upgraded to a design pressure of 2.7 MPa (400 lbf/in²), 31 of 63 transfer routes would still exceed the rated pressure of the pipe. Figures B-5 and B-6 in Appendix B show the lines that exceed the pressure rating of the pipeline.

Calculated values for pressure drop in individual transfer routes for the three sets of data range from 338 kPa (49 lbf/in²) to 7.6 MPa (1,115 lbf/in²). Using the pipe roughness values for old pipe, Tables D-25 and D-28 in Appendix D show that a majority of transfer routes cannot meet the required critical velocity without exceeding the operating pressure of the system. Using the

roughness values for new pipe reduces the number of lines that will exceed their rating, but a majority of the routes still exceed their design pressure.

The pressure drop for supernate transfers was not calculated, but can be evaluated by reviewing the results presented in Appendix D. Cases 1c and 1d of Appendix D provide the calculated pressure drop using median solid particle sizes of 5 and 10 μm , and HTWOS volume percent solids values multiplied by a factor of 1.5 or 2.0. These sets of waste properties bound the pressure drop that would be calculated for supernate waste. The highest pressure drop through any transfer route in Cases 3S4 or 3S6D is 0.8 Mpa (123 lbf/in²), assuming an end of life pipe roughness value. All of the transfer routes can be used to transfer supernate. It should be noted that the small pressure drops are based on low transfer flow velocities. The maximum calculated flow rate is 5.5 L/s (87 gpm) which is equivalent to velocity of 1.2 m/s (3.8 ft/s) in a 80 mm (3 in.) pipe. However, these values for pressure drop are conservative because they are based on waste with some small particles for an actual supernate transfer, the pressure drop will be lower.

In the preceding discussion, the adequacy of each transfer system route is evaluated by comparison of the design pressure rating of the pipe to the calculated pressure drop. It should be noted that this comparison alone is not sufficient to verify that a particular route is designed adequately. The transfer system pipe also must be designed to withstand the shutoff head of the transfer pump. The transfer pump will be designed with a shutoff head that is ~30% above the pressure drop calculated for each route. This additional design requirement will result in more transfer routes exceeding the rated pressure of the system than identified in the discussion above.

4.4 SENSITIVITY

In reviewing the pressure-drop results in Appendix D for various combinations of waste properties, it was noted that changes in some of the waste properties can have a significant impact on the calculated pressure drop. Figure 4-1 provides a comparison of the effect that changes in waste properties have on pressure drop. Figure 4-1 is based on the longest transfer route (241-AN-107 to BNFL) with a volume percent solid of 15%, 400 μm median size particle distribution, 60 °C, and old pipe. Figure 4-1 shows the magnitude of the change in pressure drop when a single waste parameter is varied from one end of its range to the other. Only one parameter is changed at a time, while the other parameters are held constant. It can be seen that the range of solids volume concentration and particle size have the most impact on the calculated pressure drop.

Figure 4-2 provides a more detailed comparison of the effects of variations in several waste properties and pipe parameters on the required system pressure. Figure 4-2 is based on the same transfer route as Figure 4-1, but also indicates values of pressure drop in the pipe at specific waste properties. The values of pressure drop are calculated by varying one variable while the others are held constant. Figure 4-2 shows that a reduction in solids density, solids size, or volume percent solids will result in a significant reduction in pressure drop. It should also be noted that the variables with the least amount of characterization data have the most significant effect on calculation of pressure drop. A more complete evaluation of the effects of the waste parameters on pressure drop is given in Appendix E for the transfer route from Tank 241-AN-107 to BNFL.

Figure 4-1. Waste-Property Sensitivity (AN-107 to BNFL).

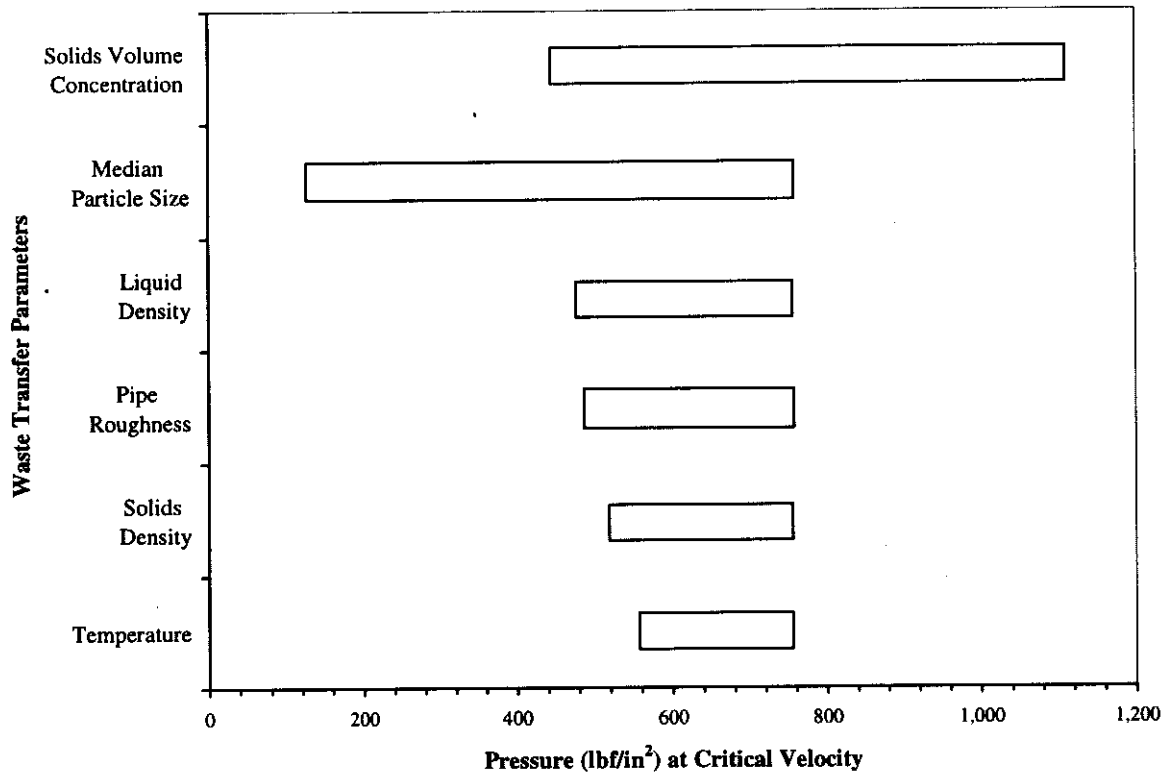
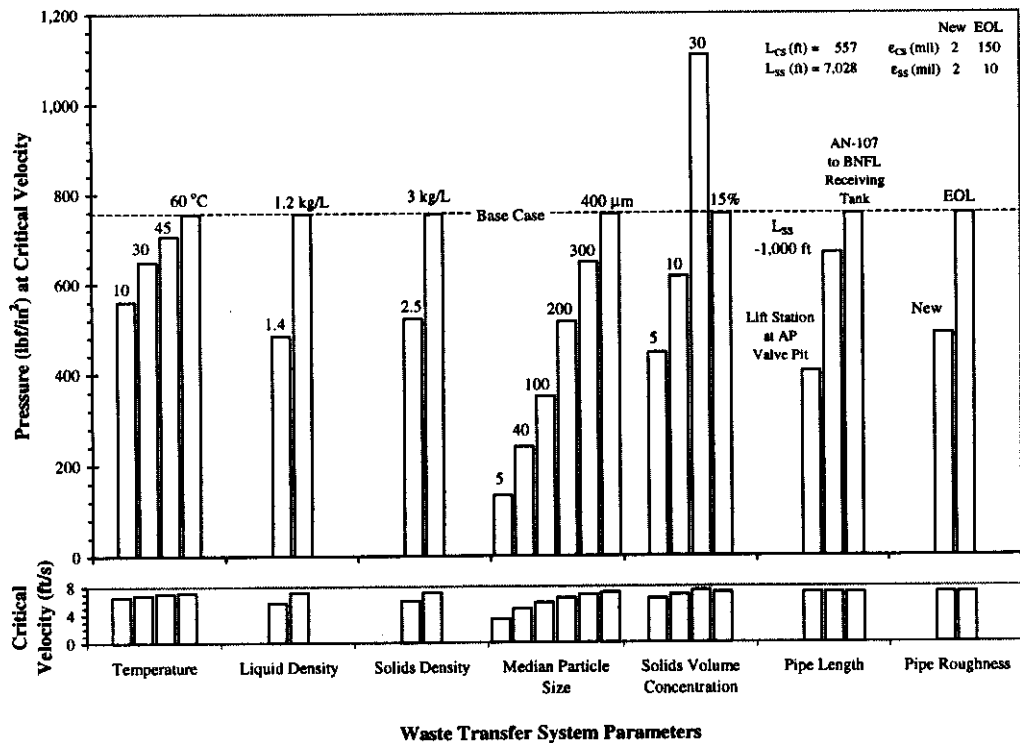


Figure 4-2. Waste-Transfer System Sensitivity (AN-107 to BNFL).



4.5 UNCERTAINTIES

This analysis is based on a number of assumptions that are critical to a proper understanding of the results. The areas of greatest uncertainty are discussed in this section.

4.5.1 Waste Characterization

Waste characteristics from the process flow sheets, the latest revisions of the tank characterization reports, letter reports from the laboratories, and the HTWOS model were reviewed. It was found that liquid viscosity data and solids diameter data for the DSTs generally are not available. Because of the lack of tank-specific data, physical property estimates were provided by Process Engineering. In performing this analysis, some simplifying assumptions had to be made regarding the particle size and particle density. It has been assumed that the density of all solid particles is 3.0 kg/L. This density value is based on densities of dominant chemical constituents in the tanks. The potential effect of this assumption is to overstate the pressure drop in the transfer system significantly if the larger particles are less dense or will be dissolved in the tanks before transfer.

The volume percent solids in the waste for Data Sets 2 and 3 has been estimated by doubling the highest solids content of any transfer identified in HTWOS. This value was then used for all transfers. The rationale for doubling the HTWOS value was to account for density gradients in the tanks during waste transfers. The rationale for using this value for all transfers was to accommodate future changes in the tank retrieval sequence that could affect the volume percent solids for a particular transfer route. In going from Case 3S4 to Case 3S6D, for instance, approximately 30% of the transfer routes were changed.

This analysis was intended to address Phase 1B and Phase 2 transfers from the AN, AP, AW, AY, and AZ tank farms using tank specific waste properties. Because there is very little tank-specific information describing the Phase 2 transfers at this time, the analysis was performed using estimated slurry properties. HNF-SD-WM-SP-012, Revision 1 contains some assumptions for Phase 2 waste regarding volume percent solids, solids density and supernatant sodium content but does not address waste properties. The waste-transfer system is intended to be used for Phase 1B and Phase 2 transfers. The adequacy of the waste-transfer system for Phase 2 transfers cannot be verified without better data.

4.5.2 Pipe Surface Roughness

The pipe surface roughness was determined by analysis based on assumed waste conditions, a 50-yr life for the carbon steel lines and a 40-yr life for the stainless steel lines. Pipe surface roughness has a large impact on the calculation of pressure drop in the carbon steel transfer pipes. The roughness values provided in Appendix C may be non-conservative, but it is not possible to know without inspecting the inside of the pipe.

4.5.3 Critical Velocity and Pressure-Drop Correlations

Although the critical velocity and pressure-drop correlations have been developed from the analysis of extensive test data, these correlations have not been validated through testing for the Hanford-specific waste characteristics.

The correlation used for determining critical velocity is empirically derived from data that does not include particles <100 μm in diameter. HNF-2728 recommends the use of the Oroskar and Turian (1980) correlation. HNF-2728 compares the correlation to another correlation that is based on flows with smaller particles (< 80 μm) and shows that the Oroskar and Turian (1980) correlation is conservative. It is possible to overstate the critical velocity requirements for slurries with small-size particles without test results using wastes typical of the Hanford Site.

Additionally, the Oroskar and Turian (1980) correlation uses a median particle size to represent the distribution of all particle sizes. This is a reasonable simplification for a slurry with a narrow range of particle sizes. It can lead to underestimation of the velocity required to keep all particles in suspension for a slurry with a wide range of particle sizes.

4.5.4 Waste Rheology

The characterization data that are available indicate that the viscosity of the carrier liquid is very sensitive to temperature. This analysis has made the assumption that the transfers will occur at a temperature that will not cause the waste to begin to gel or cause large changes in the liquid viscosity, which is consistent with current practice. Variations in temperature during a waste transfer resulting from cooling in the pipeline could change the results of this analysis substantially. The effects of non-Newtonian waste also have not been evaluated in this analysis.

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5.0 CONCLUSION

Section 5.1 provides a discussion of the analysis and identifies alternatives to address apparent deficiencies in the waste-transfer system. Section 5.2 provides recommendations to meet program needs and to evaluate the adequacy of the waste-transfer system more accurately.

5.1 DISCUSSION

The intent of this analysis was to provide a documented basis for the design pressure rating of the waste transfer system pipeline and a documented basis for the design and operating pressure of the transfer system pumps. Slurry correlations for heterogeneous and homogeneous flow were identified for estimating the critical velocity and the pressure drop through the waste-transfer pipelines. Transfer system characteristics were reviewed for each of the transfers identified in Phase 1B of Cases 3S4 and 3S6D. These transfer cases were consolidated into distinct transfer routes for evaluation of the transfer system fluid flow characteristics. More than 50 different combinations of waste properties were analyzed for each of the transfer routes in Cases 3S4 and 3S6D. Three different sets of slurry waste properties were identified as being representative for all of the waste transfers. One set of data was evaluated to determine whether a single set of waste properties could envelop all waste combinations and meet the design constraints of the transfer system. The other two sets of data were used to address specific types of waste transfers.

Use of a single waste composition that envelops all waste combinations requires a robust transfer system design. The results of the analysis for the first set of data indicates that approximately 75% of the transfer pipelines would exceed their design pressure at the critical velocity. Use of waste properties that more closely approximate expected waste conditions showed that the waste-transfer system would require fewer pipeline replacements. The results of the analysis for the second set of data found that over 50% of the lines could not meet the critical velocity without exceeding the design pressure of the pipe. The analysis for the third data set found that about 50% of the lines would exceed their design pressure at the critical velocity. Additionally, if some of the conservatism can be removed from the predicted surface roughness values by performing inspections of the pipe, fewer pipelines would have to be replaced in the system. Based on the results of the first three sets of slurry waste data, all of the 1.9 MPa (275 lbf/in²) rated lines will have to be rated at a higher design pressure or replaced. Supernate transfers will not exceed the design pressure rating of any of the transfer routes that were evaluated.

Several alternatives exist to address the apparent deficiencies in the waste-transfer system and to reduce some of the uncertainty in the waste properties. Some of the alternatives would completely resolve the pressure limitations of the pipe if implemented. Others would contribute to a reduction in the required design pressure of the pipe. These alternatives are addressed below.

Alternative 1

Design the waste-transfer system to the highest calculated pressure, plus a 30% rise to pump shutoff, for the first set of data, 10 MPa (1,450 lbf/in²). This alternative is not realistic because it assumes that the jumper connectors can be rated for a significantly higher pressure than their present rating. It also requires that the existing pipe be re-rated or replaced.

Alternative 2

Particle size and density have a large impact on the determination of pressure drop in the transfer system. Refinement of the particle size and density data has the potential to provide substantial reductions in the amount of rework that would be required by the present analysis. If it can be shown that the larger particles (>100 µm) are not very dense and will not settle, a significant reduction can be made in the number of lines that exceed their design pressure.

Alternative 3

A large contribution to the overall pressure drop in the system is because of the long transfer routes that are required to pump waste to the privatization contractor. A method of reducing line lengths is to add a lift station adjacent to the new AP valve pit or to stage all transfers through the AP tank farm. A lift station would comprise storage tanks, pumps, instrumentation, power, and ventilation equipment. All transfers to the privatization contractor and between Tank Farms AN, AY, and AZ, and Tank Farms AP and AW would be routed through the lift station or Tank Farm AP. Table D-29 in Appendix D lists the pressure drops through the transfer system assuming that waste is staged through a lift station at the new AP valve pit. With the addition of a lift station, the critical velocity is 2.16 m/s (7.1 ft/s) and the largest pressure-drop estimate for any transfer is 2.8 MPa (409 lbf/in²). A velocity of 2.16 m/s (7.1 ft/s) corresponds to a volumetric flow rate of 10 L/s (160 gal/min). A discharge pressure of 2.8 MPa (409 lbf/in²) corresponds to a discharge head of approximately 195 m (640 ft).

To account for wastes of different densities, the pressure at the discharge of the pump must be adjusted. The bulk density of the waste listed above is 1.47 kg/L. If, however, the liquid density were to be 1.46 kg/L (bounding value per Table 1 of Appendix A), the bulk density of the waste would be 1.69 kg/L. A centrifugal pump is rated in meters (or feet) of head. At a specified flow rate, the discharge head will remain constant, but the discharge pressure will vary if the density of the pumped liquid changes. The discharge pressure from a pump is estimated for a new density by the ratio of specific gravities:

$$2.8 \text{ MPa (409 lbf/in}^2\text{)} \times 1.69 / 1.47 = 3.2 \text{ MPa (470 lbf/in}^2\text{)}$$

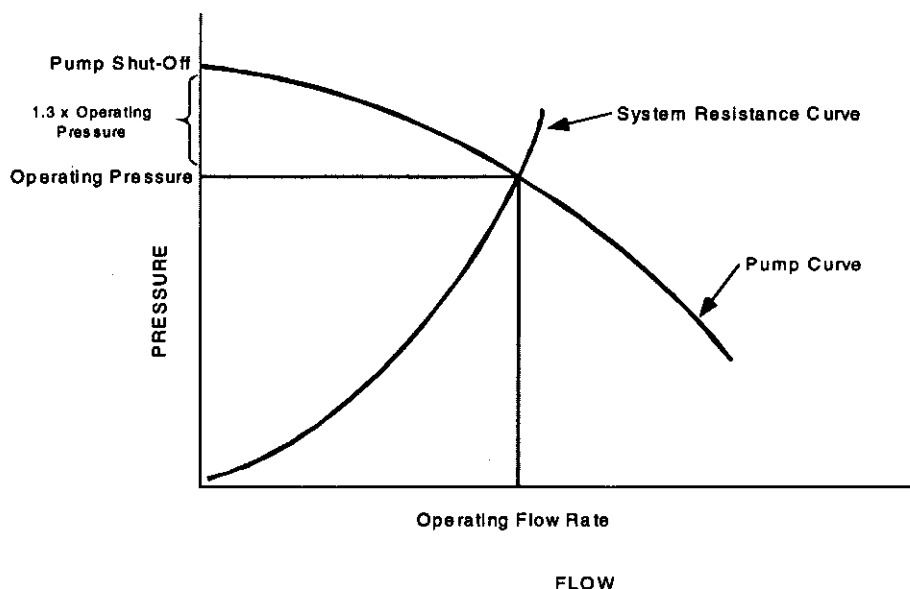
The pressure also is adjusted to account for a centrifugal pump with a head-flow characteristic that rises continuously to pump shutoff. Figure 5-1 shows a typical pump/system curve. For a centrifugal pump, a typical rise from operating point to shut-off is approximately 30%. Therefore, the new discharge pressure is:

$$3.2 \text{ MPa (470 lbf/in}^2\text{)} \times 1.3 = 4.2 \text{ MPa (611 lbf/in}^2\text{)}$$

Based on the above, the design pressure of the piping system would be specified as 4.5 MPa (650 lbf/in²). The higher pressure will allow for some variability in waste density and provide

some margin for the design of the pump. The pump would be designed to operate at 10 L/s (160 gal/min) at a discharge head of 195 m (640 ft), with a shut-off head not to exceed a head of 268 m (880 ft). Installation of a lift station or staging through AP farm would require upgrading the transfer system to a design pressure of 4.5 MPa (650 lbf/in²). However, the waste-transfer system would have some design margin, and no further work would have to be performed to better characterize the waste. Other actions that are associated with this alternative include verifying that the jumper connectors and existing system can operate at 4.5 MPa (650 lbf/in²).

Figure 5-1. Typical Pump/System Curve.



Alternative 4

A fourth alternative is to place more emphasis on defining the waste properties for each specific transfer. The analysis results demonstrated that reducing the conservatism of the waste stream characterization data will reduce the amount of pipe rework required significantly. This alternative requires obtaining additional waste characterization data to be able to accurately predict the waste composition for Phase 1B and Phase 2 waste feeds. It also assumes that changes in the retrieval sequence will not affect the waste composition for a particular transfer significantly.

Alternative 5

A fifth alternative is to inspect the pipelines that have been identified as exceeding the design pressure to determine the actual surface roughness of the pipes. A reduction in the assumed roughness of the pipe will provide a direct reduction in the pressure drop through the carbon steel transfer lines.

5.2 RECOMMENDATIONS

Evaluation of the waste-transfer system begins with an accurate characterization of the waste. It also requires accurate information regarding the condition of the existing pipeline. Preparation

of specifications to procure waste-transfer system equipment before having accurate characterization data involves the risk that the equipment will not meet the transfer system requirements. It also can lead to unnecessarily modifying or replacing portions of the transfer system that may be adequate if accurate waste characterization data were available.

The results of the analysis using the three data sets indicate that the existing transfer system can not reliably deliver slurry waste to the privatization contractor. It is recommended that several parallel paths be pursued to meet program needs and to evaluate the adequacy of the waste-transfer system more accurately. The first recommendation addresses the immediate need of providing a basis for the design of the transfer system pipe and transfer pumps. Recommendations 2 through 4 involve additional investigations to determine the structural capability of the existing transfer system. Recommendations 5 through 8 address activities that have the potential to reduce the uncertainties associated with this analysis.

Recommendation 1

Alternative 3 calls for adding a lift station. If no additional information is made available regarding waste properties or the condition of the existing pipe, Alternative 3 provides a technical solution that will meet the waste feed delivery system requirements. It is recommended that the waste properties, pipe, and pump design criteria from this analysis be used as a basis for the DST Subsystem specifications. Use of these criteria will allow conceptual design to proceed based on a technically defensible solution. As additional waste characterization data are obtained, it is highly likely that the pressure requirements will decrease rather than increase. This recommendation provides a near-term path forward while minimizing impacts to projects currently in conceptual design. These criteria should be used only for planning and conceptual design until more accurate information is available on which to base detailed design. Table 5-1 is a list of the waste-property design criteria.

Table 5-1. Waste-Property Design Criteria.

Liquid Density	1.21 to 1.46 kg/L
Liquid Viscosity	0.01 to 0.1 g/cm-s (1 to 10 cP)
Solids Density	2.5 to 3.00 kg/L
Solids Mean Diameter	40 to 400 μ m
Solids Volume Percent	10 to 30 %

Because the transfer pump is selected based on the pressure drop in the system and the pressure drop is a function of waste properties, the pipe and pump sizing information should also be used only for planning and conceptual design. Table 5-2 is a list of criteria for the pipe and pump.

Table 5-2. Pipe and Pump Design Criteria.

Pipe Design Pressure	4.5 MPa (650 lbf/in ²)
Pump Shutoff Head	not to exceed 268 m (880 ft)
Pump Operating Point	10 L/s (160 gal/min) at a discharge head of 195 m (640 ft)
Waste Density	1.69 kg/L
Viscosity	10 cP
Solids Content	15% by volume
Surface Roughness, Carbon Steel	51 to 3810 μm (2 to 150 mil)
Surface Roughness, Stainless Steel	51 to 254 μm (2 to 10 mil)

Recommendation 2

The viability of increasing the design pressure rating of existing pipe rated at 1.9 MPa (275 lbf/in²) should be investigated. Unless there is a substantial change in the waste properties used in this analysis, the design pressure of these lines is not high enough to support waste feed delivery. If the pressure rating of the lines cannot be increased, the lines will have to be replaced.

Recommendation 3

The design capacity of the Project W-314 pipe should be evaluated to determine if it can operate at a pressure higher than 2.7 MPa (400 lbf/in²). Figure B-7 of Appendix B shows that the scope of Project W-314 comprises a substantial portion of the waste-transfer system. If the design pressure of the system must be increased, it would be prudent to establish the capacity of the pipe before it is installed.

Recommendation 4

Because the PUREX connectors are an integral part of the transfer system, a concurrent assessment was performed (*Waste Feed Delivery PUREX Process Connector Design Pressure* [RPP-5793]) to determine their design pressure retaining capacity, which was found to be 2.7 MPa (400 lbf/in²). If the pipe design pressure ultimately must be increased, this maximum design pressure will be challenged. A test program will be required to establish a design pressure higher than the generally accepted maximum pressure of 400 psig.

Recommendation 5

Alternative 2 addresses a more intensive evaluation of particle properties. A reduction in the size or density of the particles will reduce the critical velocity and the pressure drop in the system. Reductions in particle size and density have the potential to significantly reduce the number of lines that exceed their design pressure at the critical velocity. It is recommended that particle data and the particle measurement procedure be reviewed to identify methods of reducing the uncertainty associated with the size and density of the particles. The measurement procedure should simulate the shearing that is inherent in mixing and pumping the waste. It is also

recommended that the difference between measured values of solids density and calculated values of solids density be investigated.

Recommendation 6

The inside of the existing transfer pipelines should be inspected. If increasing the pressure rating of the existing pipelines is a viable alternative, some type of inspection will be required to establish a baseline condition of the pipe. Inspection should be performed after it has been confirmed by analysis that the pipe pressure rating can be increased. Additionally, if the existing level of corrosion is less than assumed, many of the transfer lines will be acceptable. If the projected corrosion is as extensive as assumed, a knowledgeable decision can then be made on the most cost-effective way to proceed.

Recommendation 7

Additional data are required for evaluating the transfer system performance. Information such as supernatant viscosity, bulk viscosity, and density gradients in mixed tanks are required to confirm that the proper hydraulic models are being used and to perform the critical velocity and pressure-drop analyses. The characterization data should be requested when sampling the tanks, in addition to data typically obtained. Neither the Oroskar and Turien model, used to predict critical velocity, nor the Wasp model used to predicted pressure drop have been validated for Hanford wastes. Validation of these models with actual waste or with simulant material under controlled conditions is recommended.

Recommendation 8

Changes in some waste properties have a more significant impact on pressure drop in the pipeline than others. Discussions should be initiated with the Process Development group to determine the feasibility of limiting or controlling these properties before the transfer of waste. An example would be solids volume fraction in the waste. Increases in solids volume fraction directly increase the pressure drop in the pipeline, yet solids volume fraction can be adjusted. A reduction in this value will reduce the pressure drop in the transfer lines and provide an expanded operating range.

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APPENDIX A

HANFORD TANK WASTE PHYSICAL PROPERTY ESTIMATES

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MEMORANDUM**CH2MHILL**

74B50-00-001R2

**ALTERNATIVES GENERATION AND ANALYSIS FOR WASTE FEED DELIVERY
TRANSFER PUMP DISCHARGE PRESSURE: HANFORD TANK WASTE PHYSICAL
PROPERTY ESTIMATES****TO:** W. L. Willis R3-73

COPIES: T. J. Conrads R3-73
 F. F. Erian K7-15
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 SDE LB/File

FROM: Process Control**DATE:** April 10, 2000

- References:**
- (1) Internal Memorandum, "Alternatives Generation and Analysis for Waste Feed Delivery Transfer Pump Discharge Pressure: Hanford Tank Waste Physical Property Estimates," memorandum number 74B50-00-001R1, dated February 11, 2000.
 - (2) Internal Memorandum, "Memorandum of Understanding Requirements Action Memo for the Alternatives Generation and Analysis for Feed Delivery Transfer Pump Discharge Pressure Control," memorandum number 82400-99-093, dated December 30, 1999.
 - (3) Textbook, Shook, C.A. & M.C. Rocco, "*Slurry Flow - Principles and Practice*," Butterworth-Heinemann, Stoneham, Massachusetts," published 1991.
 - (4) Handbook, Weast, C.W. & M.J. Astle, "*CRC Handbook of Chemistry and Physics*," 61st ed., CRC Press, Boca Raton, Florida," published 1980.

This memorandum revises Reference (1) that was created in response to Reference (2) direction. This revision incorporates text changes and corrects information presented in the Attachment.

W. L. Willis
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 April 10, 2000

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Pipeline transport of wastes containing solids (a slurry) can be considered a two-phase flow, as required to estimate a critical velocity. Additionally, such a slurry can be considered to have bulk, or homogeneous properties, that are useful in (but not the only method available for) determining pressure drops and power requirements for the respective piping system. This memo attempts to give bounds and best estimates of the waste properties at shear rates applicable to typical Hanford pipeline transport velocities.

The following definitions are needed to describe both two-phase and homogeneous slurry flow:

Subscript "c" indicates the carrier liquid (i.e., the continuous phase) of the slurry. Also included in the carrier liquid are any dissolved soluble solids.

Subscript "d" indicates the solid particulates (i.e., the dispersed phase) in the slurry. These particles can be both insoluble solids and precipitated soluble solids.

Subscript "m" indicates the bulk property of the slurry (i.e., the mixed property of the two-phase fluid).

D = diameter of the spherical solid particulates in the waste

α = volume fraction

ρ = density

μ = Newtonian dynamic viscosity (this estimate does not describe the potential non-Newtonian behavior of some of the Hanford tank wastes. However, the non-Newtonian nature of these wastes should not be such as to create conditions that could terminate a waste transfer provided the waste can be initially mobilized and kept in suspension via sufficient flow velocity for the duration of the transfer.)

Physical Relationships

$$\begin{aligned}\alpha_c &= \alpha_{\text{water}} + \alpha_{\text{dissolved solids}} \\ \alpha_d &= \alpha_{\text{insoluble solids}} + \alpha_{\text{precipitated soluble solids}} \\ \alpha_c + \alpha_d &= 1 \\ \rho_m &= \alpha_c \rho_c + \alpha_d \rho_d\end{aligned}$$

The summary of the nominal and bounding in-situ waste properties are shown in Table 1:

Table 1: Slurry Physical Properties

Case ⇒ Property ↓	Nominal	Bounding
D_d ①	40 μm	400 μm
α_c	0.85	0.70
α_d	0.15	0.30
ρ_c ②	1.21	1.46
ρ_d ③	2.50	3.00
ρ_m	1.40	1.92
μ_c	④	④
μ_m	⑤	⑤

Notes

① These solid particulate diameters are stated as the **median** of the distribution, meaning that one-half of the total particulate mass is contained in particles with diameters smaller than the stated diameter, with the other half of the total particulate mass being contained in particles with a larger diameter.

The solid particulates in the waste are assumed to be spherical. The spherical shape gives the minimum isotropic drag coefficient for a particle of a given mass. This property should result in minimizing slurry flow pressure drop (a non-conservative result) while maximizing slurry flow critical velocity (a conservative result). There are many Hanford tank waste solids size distribution data available that indicate, at least in a qualitative sense, that the mass distribution of the majority of solids are under 40 μm in size. However, there is data suggesting particulate sizes exist at, and may exceed, the limit of instrument detection (in some cases, approximately 1 mm - see Attachment).

② Values from analysis of Tank Characterization Data (TCD) liquid wastes with the nominal value corresponding to the 50th percentile and the bounding value corresponding to the 90th percentile. (see Attachment)

③ Waste true solids densities qualitatively estimated as an engineering judgement. As a comparison, densities of some dominant chemical constituents that are known to or may exist as solids in the tank wastes are shown in Table 2:

Table 2: True (Particulate) Solid Densities (gm/cc)*

Anion→ Cation↓	(OH) _x	NO ₃	NO ₂	PO ₄	C ₂ O ₄	CO ₃	Fe(CN ₆)
Na _x	2.13	2.26	2.17	1.62	1.46	1.44 - 2.53	2.34
Al _x	2.42	?	?	2.57	?	?	?
Fe _x	3.40	~1.68	?	2.58 - 2.74	2.28	?	1.60
Cr _x	?	?	?	2.12 - 2.42	~2.47	?	?
(NH ₃) _x	?	1.73	1.69	1.80 - 2.21	1.50	?	?

*density ranges indicates various waters of hydration
? density is unknown or compound is not documented

Table 2 gives some idea of a qualitative comparison to the estimated nominal and bounding solid densities of 2.50 and 3.00 gm/cc.

④ These μ_c estimates are conservative for pressure drop calculations but are not conservative for typical critical velocity correlations. In determining critical velocities, the most conservative assumption for μ_c is that it is equivalent to water. In contrast, for pressure drop calculations, higher values of μ_c result in higher pressure drops.

The viscosity of the carrier liquid is stated as:

$$\mu_c = \mu_{\text{water}} \left((x_{\text{salt}}) * \left(1 + 1.071 * \left(\frac{\rho_c}{\rho_{\text{water}}} - 1 \right) \right) + (x_{\text{caustic}}) * \exp \left(\left(7.143 * \left(\frac{\rho_c}{\rho_{\text{water}}} - 1 \right) \right)^{1.15} \right) \right)$$

utilizing the following definitions:

x_{salt} = the fraction of **dissolved solids** composed of sodium nitrate, sodium nitrite, and other water soluble salts.

x_{caustic} = the fraction of **dissolved solids** composed of sodium hydroxide.

$$\text{Assume: } x_{\text{salt}} = 0.9; \quad x_{\text{caustic}} = 0.1; \quad (x_{\text{salt}} + x_{\text{caustic}}) = 1$$

Over the temperature range of 0^oC to 100^oC, this assumption gives:

$$\rho_c = \rho_{\text{water}} * (1 + 0.00848 * (\text{wt\% dissolved solids}))$$

W. L. Willis
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Over the temperature range of 0°C to 100°C (from Reference 4):

$$\rho_{\text{water}} = \frac{999.83952 + 16.945176(T) - 7.9870401 \cdot 10^{-3}(T)^2 - 46.170461 \cdot 10^{-6}(T)^3 + 105.56302 \cdot 10^{-9}(T)^4 - 280.54253 \cdot 10^{-12}(T)^5}{1000 \cdot (1 + 16.879850 \cdot 10^{-3}(T)^3)}$$

Over the temperature range of 0°C to 20°C (from Reference 4):

$$\mu_{\text{water}} = 100 \cdot cP \cdot \exp\left(\frac{2996}{998.333 + 8.1855 \cdot (T - 20) + 0.00585 \cdot (T - 20)^2} - 7.60390\right)$$

And, over the temperature range of 20°C to 100°C (from Reference 4):

$$\mu_{\text{water}} = 1.002 \cdot cP \cdot \exp\left(\frac{3.0560(20 - T) - 0.002425(T - 20)^2}{T + 105}\right)$$

⊙ No exact method exists for determining μ_m . Various researchers have proposed models that predict μ_m as a function of one or more variables in addition to μ_c , and the resultant value of μ_m is typically expressed as a multiple of μ_c . If the model utilizes only one variable, it is usually α_d .

As an example, several slurry viscosity models are documented in Reference (3). These include models from Einstein, Eiler, Thomas, Kreiger, and Vocadlo. For these referenced models that are applicable to concentrated slurries, a reasonable selection of the independent variables predicts results within a factor of 2.0 for a slurry when $\alpha_d = 0.3$.

The model of Thomas is reproduced here because it has only one independent variable (α_d):

$$\mu_m = \mu_c \cdot (1 + 2.5 \cdot \alpha_d + 10.05 \cdot \alpha_d^2 + 0.00273 \cdot \exp(16.6 \cdot \alpha_d))$$

S. D. Estey

S. D. Estey, Engineer
Process Control

sde/mjg

Attachment

RPP-5346 REV 0

Attachment to 74B50-00-001-R2

Comments by L. Jensen, February 8, 2000

This following summary data for PARTICLE VOLUME DISTRIBUTION is from Table 2-2, HNF-2728 by S Estey and AT Hu. The units are μm .

SST	Mean(μm)	min(μm)	max(μm)	date	Lab
B-106	3.64	0.2	6	1997	LANL
B-111	3.66	0.4	9	1995	PNL
BX-103	1.5	0.2	7	1997	LANL
BX-107	5.67	0.4	35	1995	PNL
BY-104	6.98	0.4	75	1996	PNL
BY-108	6.33	0.2	39	1997	PNL
BY-110	6.15	0.25	40	1996	PNL
C-103	1.06	0.4	2	1995	PNL
C-104	2.05	0.2	7	1997	LANL
C-105	2.16	0.2	7	1997	LANL
S-101	6.8	0.29	38	1997	PNL
S-104	6.78	0.2	30	1997	PNL
S-107	13.48	0.35	100	1996	PNL
S-111	47.7	0.9	120	1997	PNL
SX-108	12.8	0.2	112	1996	PNL
SX-113	1.84	0.2	6	1997	LANL
T-104	4.85	0.4	13	1995	PNL
T-111	4.82	0.4	12	1995	PNL
DST	Mean(μm)	min(μm)	max(μm)	date	Lab
AN-104	5.68	0.7	19	1997	PNL
SY-102	9.32	0.2	105	1996	PNL
SY-103	9.71	0.4	30	1995	PNL

The following summary data for PARTICLE VOLUME DISTRIBUTION is from multiple internal memos by JF O'Rourke. The units are μm .

DST	median(μm)	sd(μm)	mode(μm)	mean(μm)	min(μm)	max(μm)	date	Inst.Max	Lab	Comments
SY-101	183.60	82.70	237.80	164.80	0.22	451	1999	1020	222S	replicate samples?
SY-101	125.80	264.10	159.00	197.30	0.19	1019	1999	1020	222S	
AW-101	314.00	162.00	427.00	307.00	6.7	1020	1999	1020	222S	undiluted
AW-101	946.00	224.00	952.00	884.00	29.9	1020	1999	1020	222S	diluted
AW-101	940.00	283.00	952.00	833.00	11	1020	1999	1020	222S	diluted
AN-104	4.85	12.80	31.30	13.40	0.5	31	1998	150	222S	
AN-105	4.28	13.48	41.61	10.47	0.5	45	1998	150	222S	replicate samples?
AN-105	5.76	23.80	66.90	19.35	0.5	70	1997	150	222S	
AN-105	3.24	2.87	4.25	3.72	0.5	30	1997	150	222S	
AN-105	4.95	12.35	31.30	12.00	0.5	50	1997	150	222S	
AN-105	4.48	14.70	45.70	12.40	0.5	50	1997	150	222S	
SST	median(μm)	sd(μm)	mode(μm)	mean(μm)	min(μm)	max(μm)	date	Inst.Max	Lab	Comments
S-111	107.40	46.60	156.60	100.00	7.7	174.6	1999	1020	222S	replicate samples?
S-111	190.00	79.70	235.40	168.10	11.6	451	1999	1020	222S	
S-111	92.90	58.00	121.60	92.10	6.7	592	1999	1020	222S	
U-109	13.16	5.90	18.60	12.80	2.2	26	1999	150	222S	replicate samples?
U-109	17.30	9.00	24.60	15.90	2.6	34.3	1999	150	222S	
C-106	1.74	2.20	1.25	2.74	0.5	11	1996	150	222S	replicate samples?
C-106	4.00	5.26	4.75	6.40	0.5	28	1996	150	222S	

Comments:

1. There are not very many tanks with particle distribution data.
2. The analytical methods do not give results greater than 150 and 1020 μm . There may be particles larger than 150 and 1020, but not detected by the analytical method.
3. The DSTs appear to be separated in to two classes, one with a small median μm and the other with a large median μm . The reason is unknown.
4. The bounding value of 200 μm . is an underestimate of the median particle volume distribution or the maximum value. The analytical method maximum value of 1020 is reported for four samples.
5. Without having electronic copies of the original particle size data, it is not possible to give statistical measures of uncertainty associated with particle quantiles such as the median (50th percentile of the distribution) or values for the 25th or 75th percentile of the distribution.

Summary of Specific Gravity Results from the Tank Characterization Database.

A comparison between the (nominal, conservative, and bounding) density values given in memorandum 74B50-00-001, and the analytical results from the Tank Characterization Database (TCD) has been completed. The values in the memorandum compare well with the 50th, 75th, and 90th percentiles of values from laboratory assays.

The tanks used in the analysis were those which had liquid results for the following eight constituents: aluminum, hydroxide, nitrate, nitrite, percent water, sodium, specific gravity, and total organic carbon. Twenty-one single shell, and twenty double shell tanks, met this criteria. These tanks are listed in Table 1.

Table 1. Tanks used in Analysis.

Single Shell Tanks	Double Shell Tanks
241-A-101	241-AN-101
241-AX-101	241-AN-102
241-BY-103	241-AP-101
241-BY-105	241-AP-102
241-BY-106	241-AP-103
241-S-102	241-AP-104
241-S-103	241-AP-105
241-S-104	241-AP-106
241-S-106	241-AP-107
241-S-109	241-AP-108
241-S-111	241-AW-102
241-SX-101	241-AW-103
241-SX-103	241-AW-104
241-SX-105	241-AW-105
241-SX-106	241-AW-106
241-U-102	241-AY-101
241-U-103	241-AY-102
241-U-105	241-AZ-101
241-U-107	241-AZ-102
241-U-108	241-SY-102
241-U-109	

The pairwise correlations of the eight constituents are given in Table 2. A correlation of one (or negative one) indicate that the constituents have a linear relationship. When this occurs, the value of one constituent can be used to predict the value of another. As an example, the -0.98 correlation between percent water and sodium, means that percent water decreases, in a predictable manner, as sodium increases. Similarly, the 0.92 correlation between specific gravity and sodium, means that specific gravity increases as the sodium concentration increases.

Note that many of the constituents have correlations of 0.75 or greater. This is an indication that the waste tends to be either dilute, or concentrated, while the ratios of individual values to one another are fairly constant (at least for the analytes with large concentrations).

Table 2. Correlations.

Constituent	Al	OH	NO3	NO2	H2O	Na	SPG	DOC
Aluminum	1.00	0.76	0.52	0.7	-0.80	0.80	0.80	0.081
Hydroxide	0.76	1.00	0.62	0.62	-0.77	0.78	0.75	-0.005
Nitrate	0.52	0.62	1.00	0.71	-0.84	0.85	0.77	0.30
Nitrite	0.70	0.62	0.71	1.00	-0.85	0.85	0.80	0.38
Percent Water	-0.80	-0.77	-0.84	-0.85	1.00	-0.98	-0.95	-0.34
Sodium	0.80	0.78	0.85	0.85	-0.98	1.00	0.92	0.35
Specific gravity	0.80	0.75	0.77	0.80	-0.95	0.92	1.00	0.28
Total organic carbon	0.081	-0.005	0.30	0.38	-0.34	0.35	0.28	1.00

The strong correlations given in Table 2 suggest that percentile estimates, calculated on a constituent by constituent basis, can be combined to produce reasonable estimates of dilute, or concentrated, waste.

Table 3. Percentile Estimates for Dilute, or Concentrated, Waste*.

Constituent	10 th	25 th	50 th	75 th	90 th
Aluminum	0.01%	0.14%	0.97%	2.50%	3.19%
Hydroxide	0.11%	0.36%	1.40%	2.89%	3.60%
Nitrate	0.67%	2.78%	8.28%	12.72%	15.71%
Nitrite	0.13%	1.17%	3.68%	6.60%	9.67%
Percent Water	96.2%	85.9%	63.1%	51.0%	47.3%
Sodium	1.02%	4.12%	11.81%	15.69%	17.12%
Total organic carbon	0.02%	0.05%	0.14%	0.27%	0.40%
Specific gravity	1.01	1.09	1.28	1.43	1.47

- Results are on a weight basis.

By using the results in Table 3, and the equation on page three of the memorandum, viscosity estimates were calculated. The results are given in Table 4 (the weight percentages are approximate). Note that the viscosity associated with the 50th percentile, and the viscosity associated with the 90th percentile, are reasonably close to the "conservative" and "bounding" values given in the memorandum (1.7 versus 1.86 and 3.3 versus 3.04 respectively).

Attachment to 74B50-00-001-R2

Table 4. Viscosity Estimates.

Constituent	10 th	25 th	50 th	75 th	90 th
NaOH	0.44%	1.7%	5.3%	8.1%	9.3%
NaNO ₂ + NaNO ₃	1.48%	10.9%	19.8%	29.7%	36.8%
Percent Water	96.2%	85.9%	63.1%	51.0%	47.3%
Viscosity*	1.03	1.28	1.86	2.58	3.04

- Multiply this number by the viscosity of water.

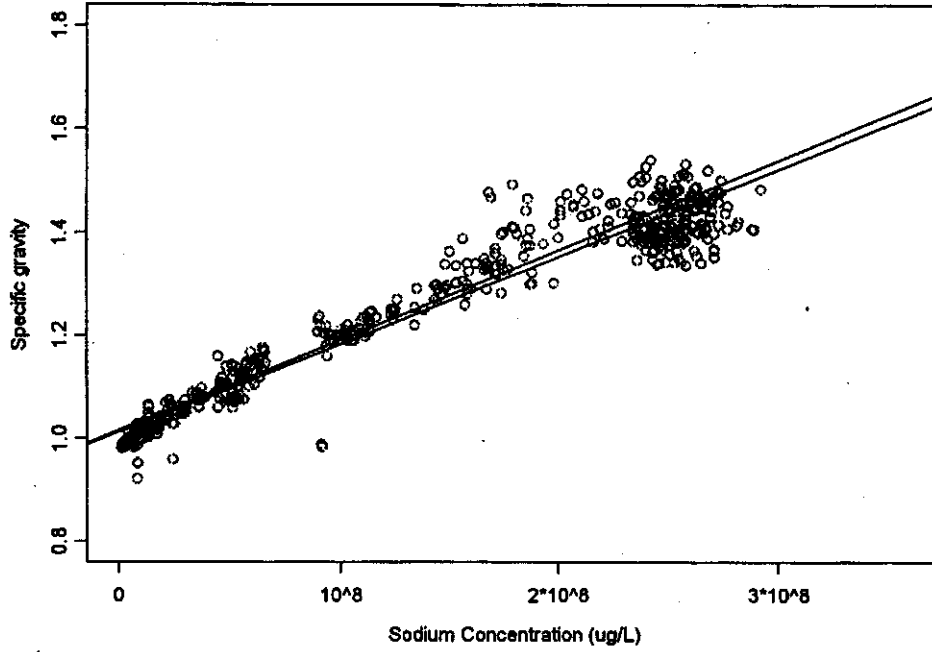
In the preceding analysis, all of the results from the 21 single shell, and 20 double shell tanks were combined. The 41 tanks used in the calculations were tanks that had data for each of eight major constituents.

An examination of all specific gravity results on TCD was also made. Except for tanks AN-106 and SY-103, all of the double-shell tanks had specific gravity data. In addition, fifty four of the 149 single shell tanks had specific gravity measurements. Note that the 75th and 90th percentiles are similar for the two tank types, but the double shell tanks have lower values for the other percentiles. These results indicate that, if the results above are applied to double shell tanks only, then the 10th, 25th, and 50th percentiles may be biased high (i.e. they are "conservative" estimates)

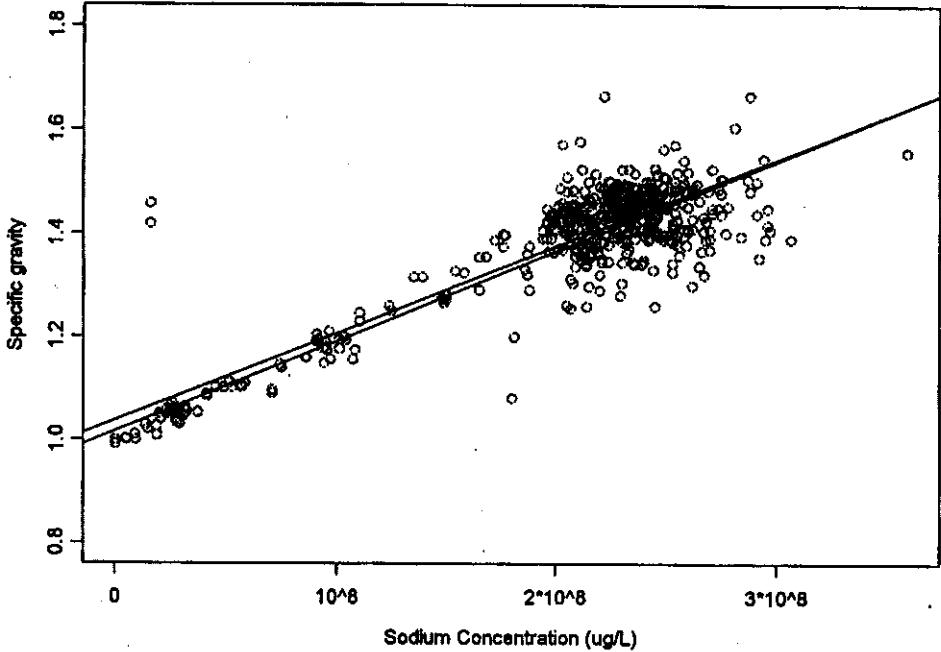
Table 5. Specific Gravity Results, All Tanks.

Tank Type	10 th	25 th	50 th	75 th	90 th
Single Shell Tank	1.15	1.35	1.43	1.46	1.49
Double Shell Tank	1.00	1.05	1.21	1.40	1.46

Double-Shell Tanks: Specific Gravity versus Sodium Concentration



Single-Shell Tanks: Specific Gravity versus Sodium Concentration



APPENDIX B

TRANSFER ROUTES IN 200 EAST AREA

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Figure B-1. Waste Transfer System.

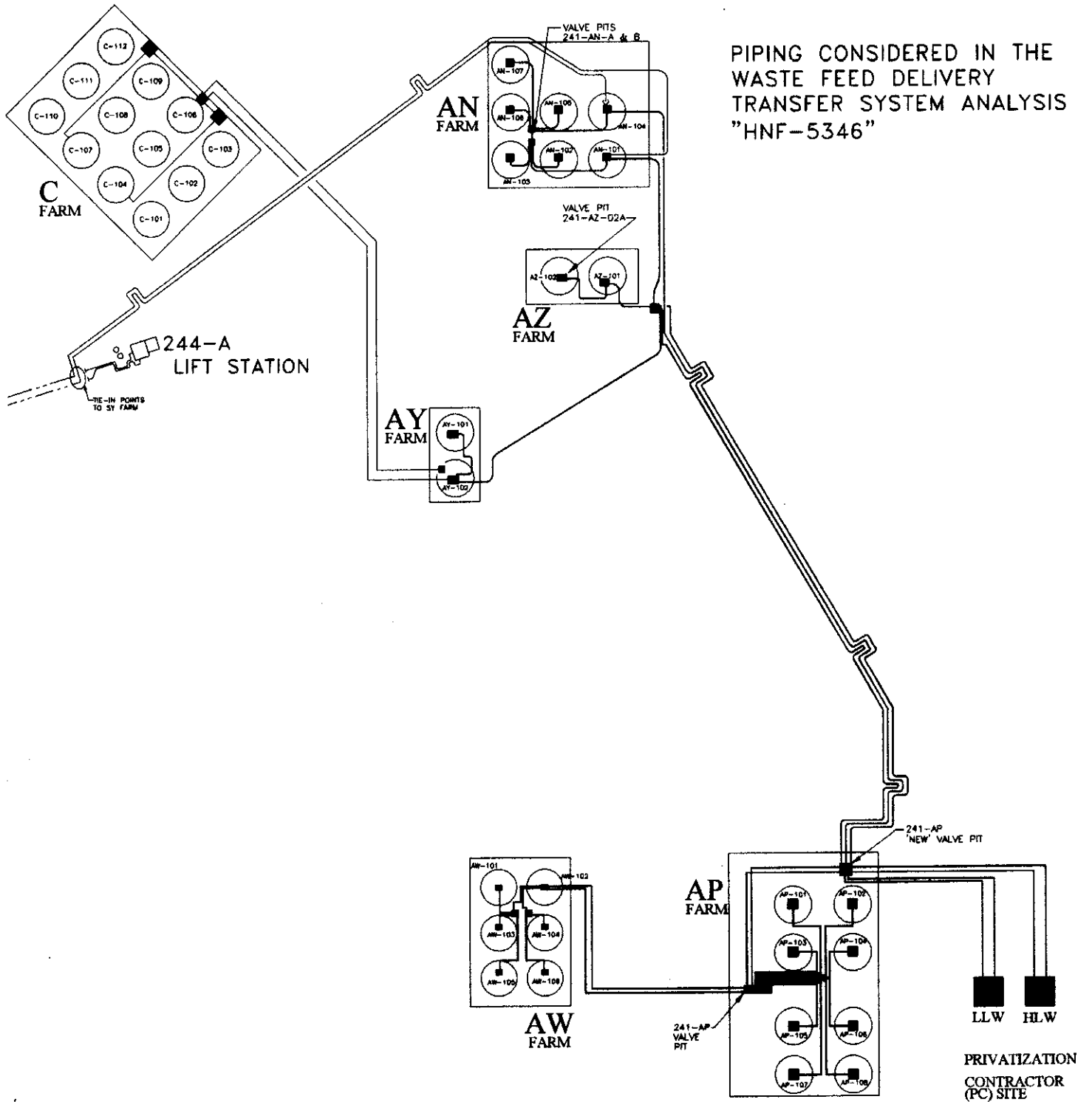


Figure B-2. 30 Volume Percent Solids/No Pipe Upgrades.

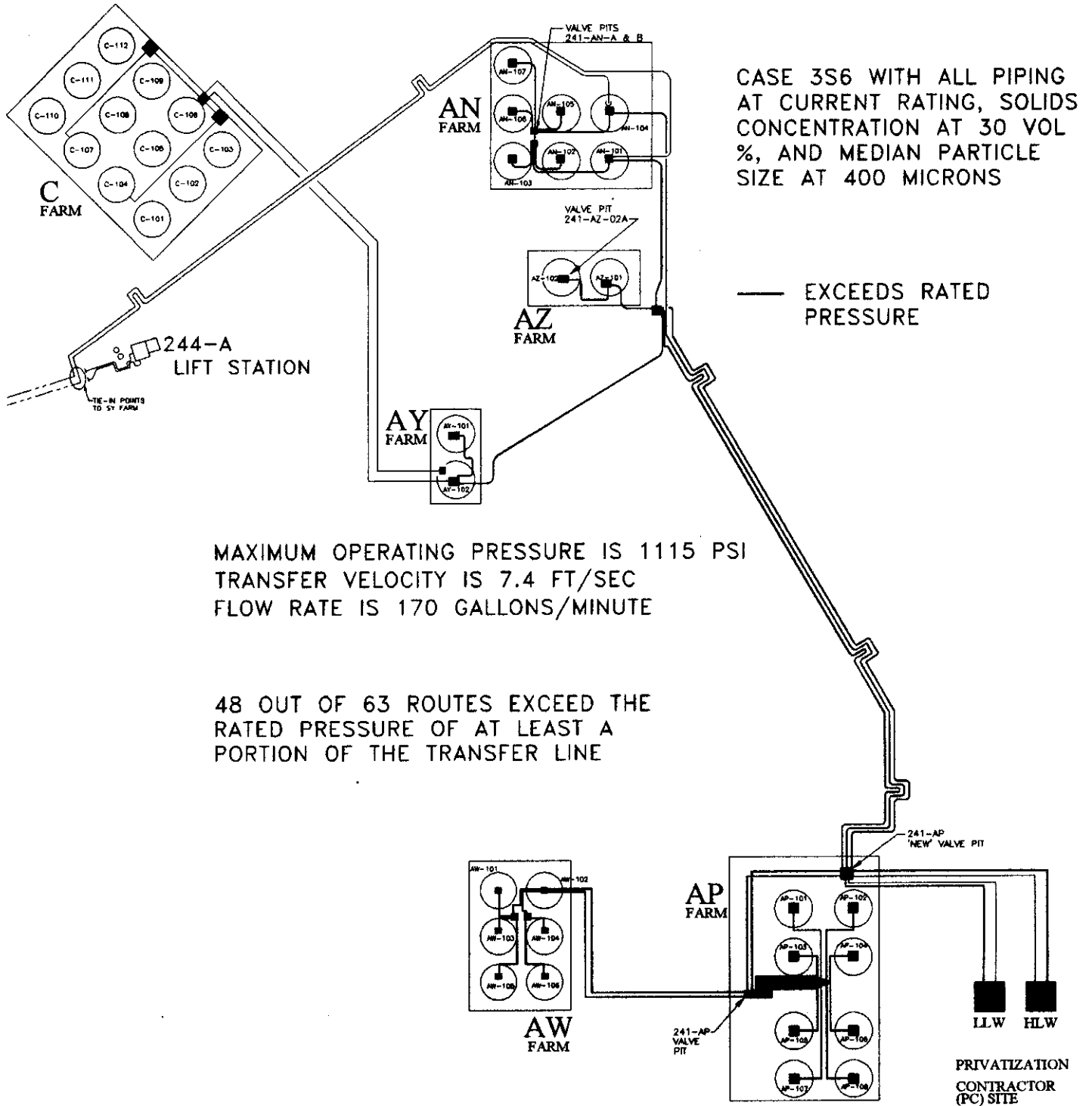
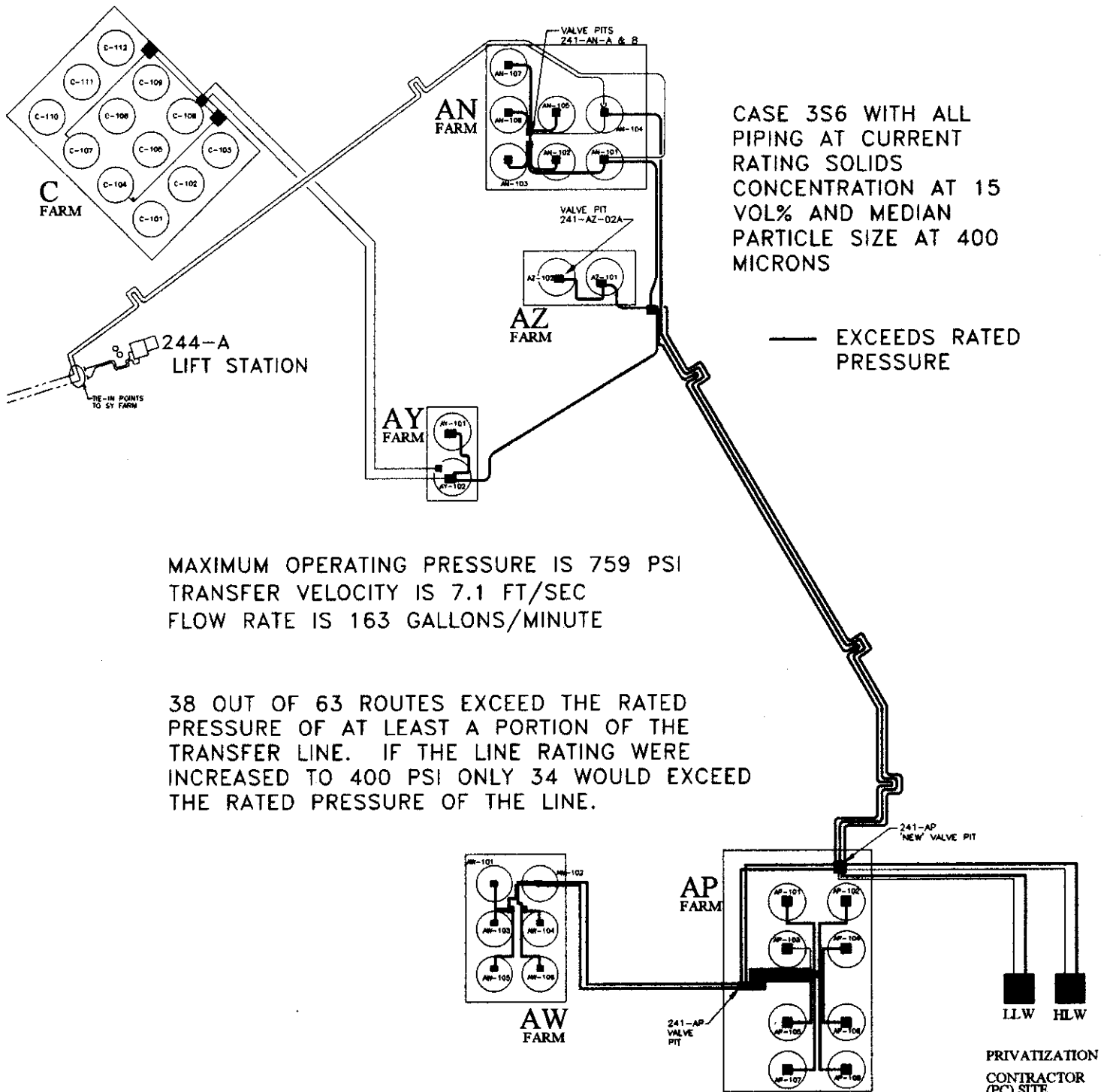
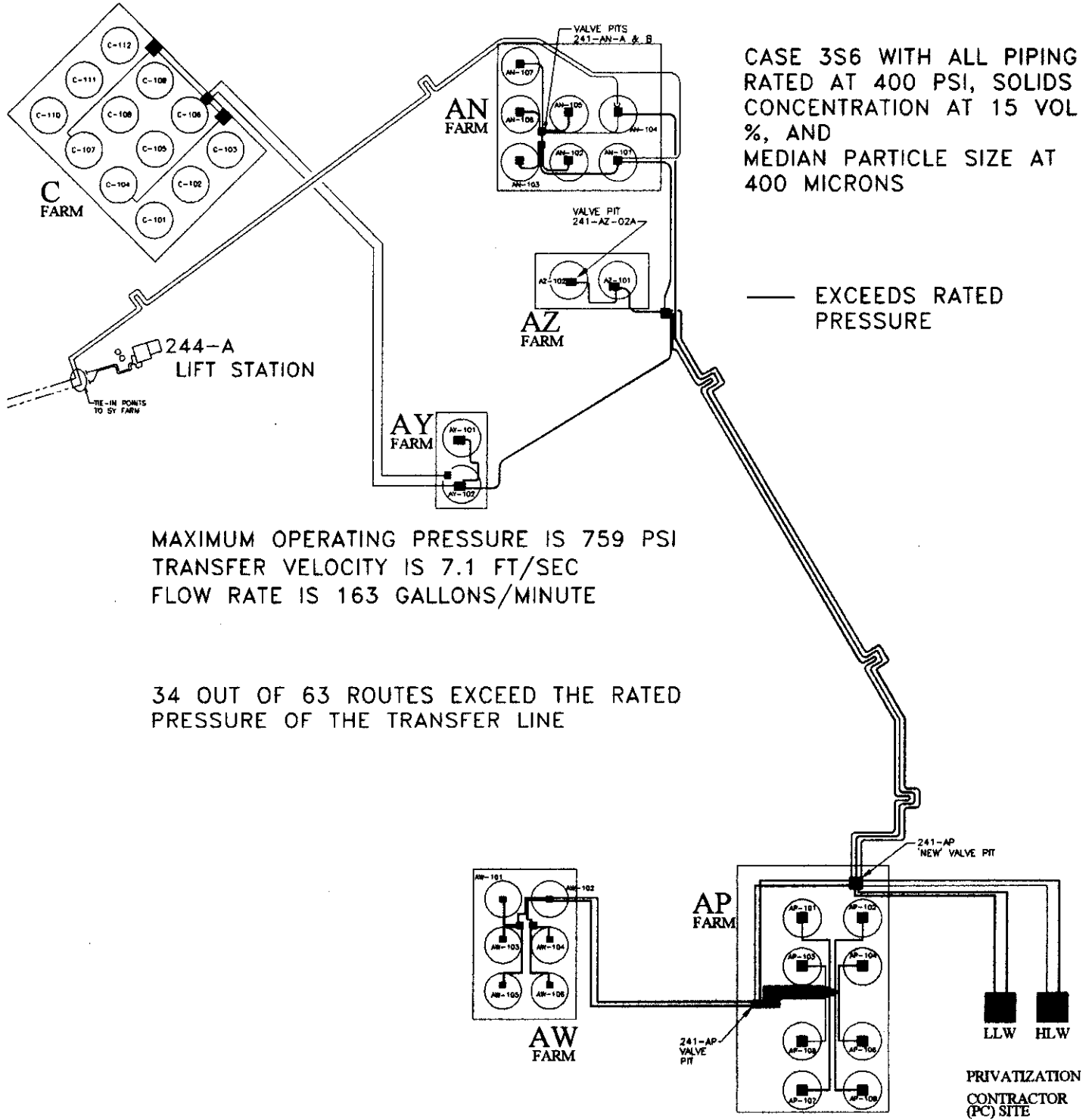


Figure B-3. 15 Volume Percent Solids/No Pipe Upgrades.



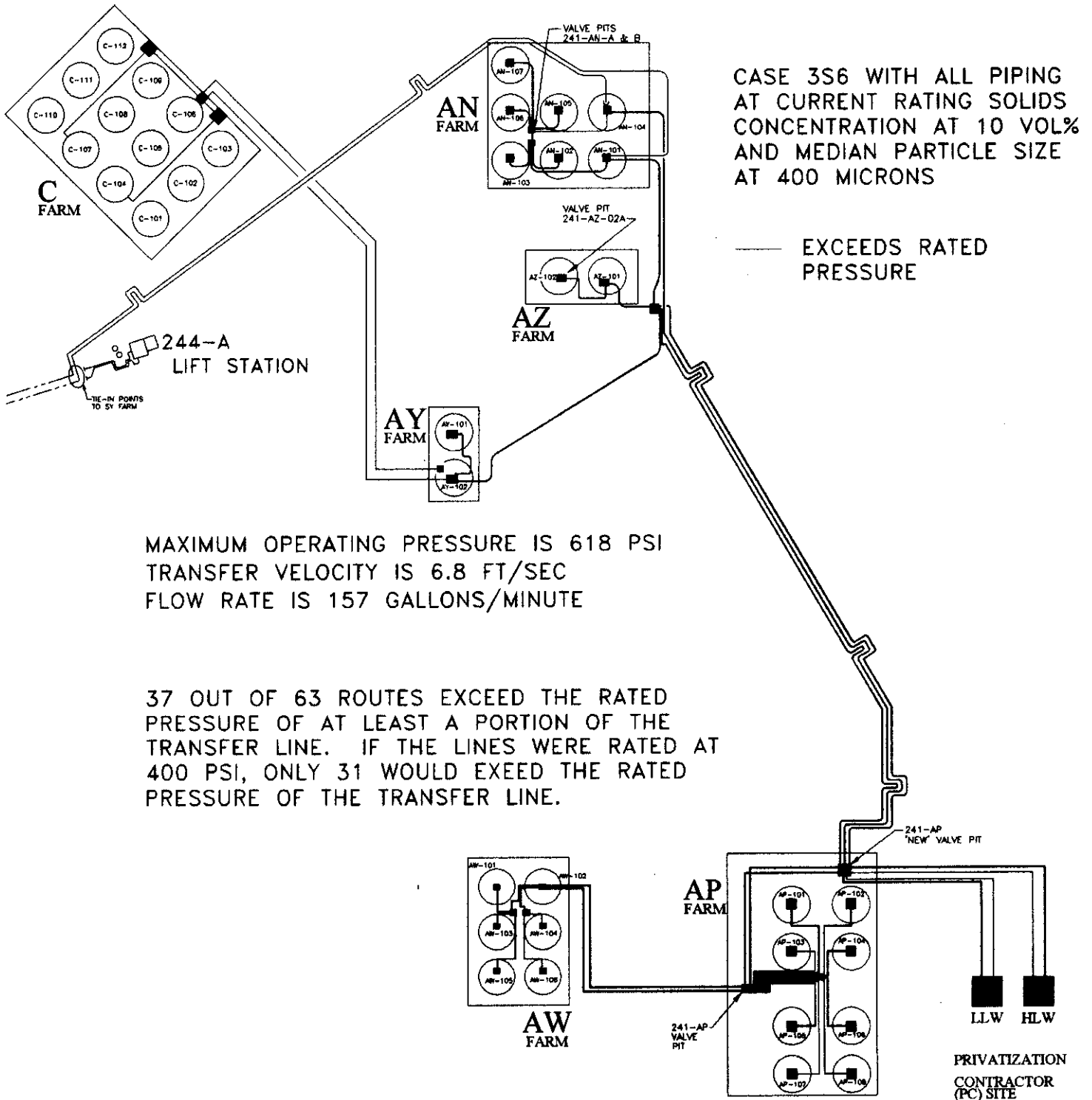
3S64B60.DWG

Figure B-4. 15 Volume Percent Solids/Pipe Upgraded to 400 lbf/in².



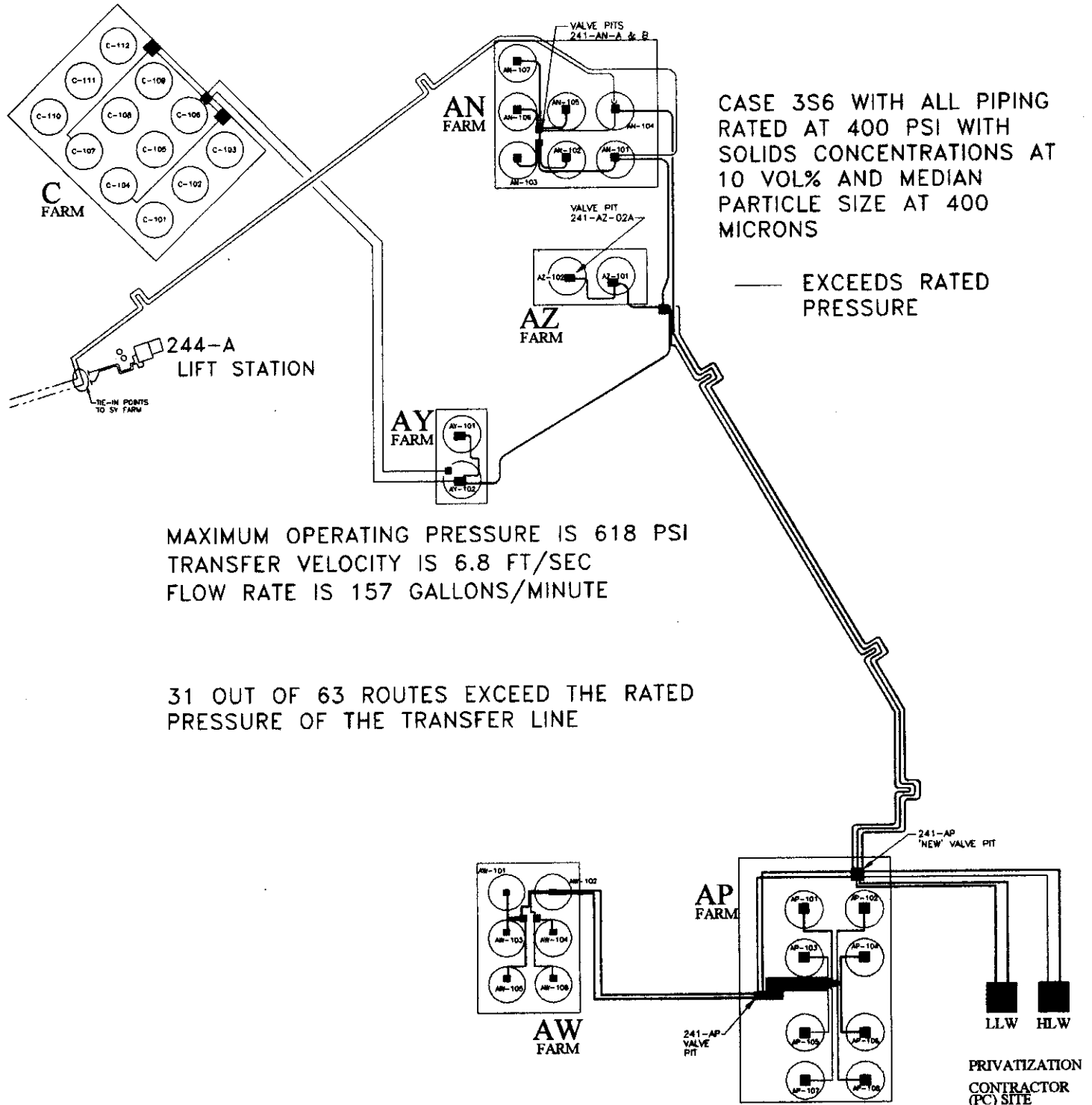
3S615VOL%400400PSLDWG

Figure B-5. 10 Volume Percent Solids/No Pipe Upgrades.



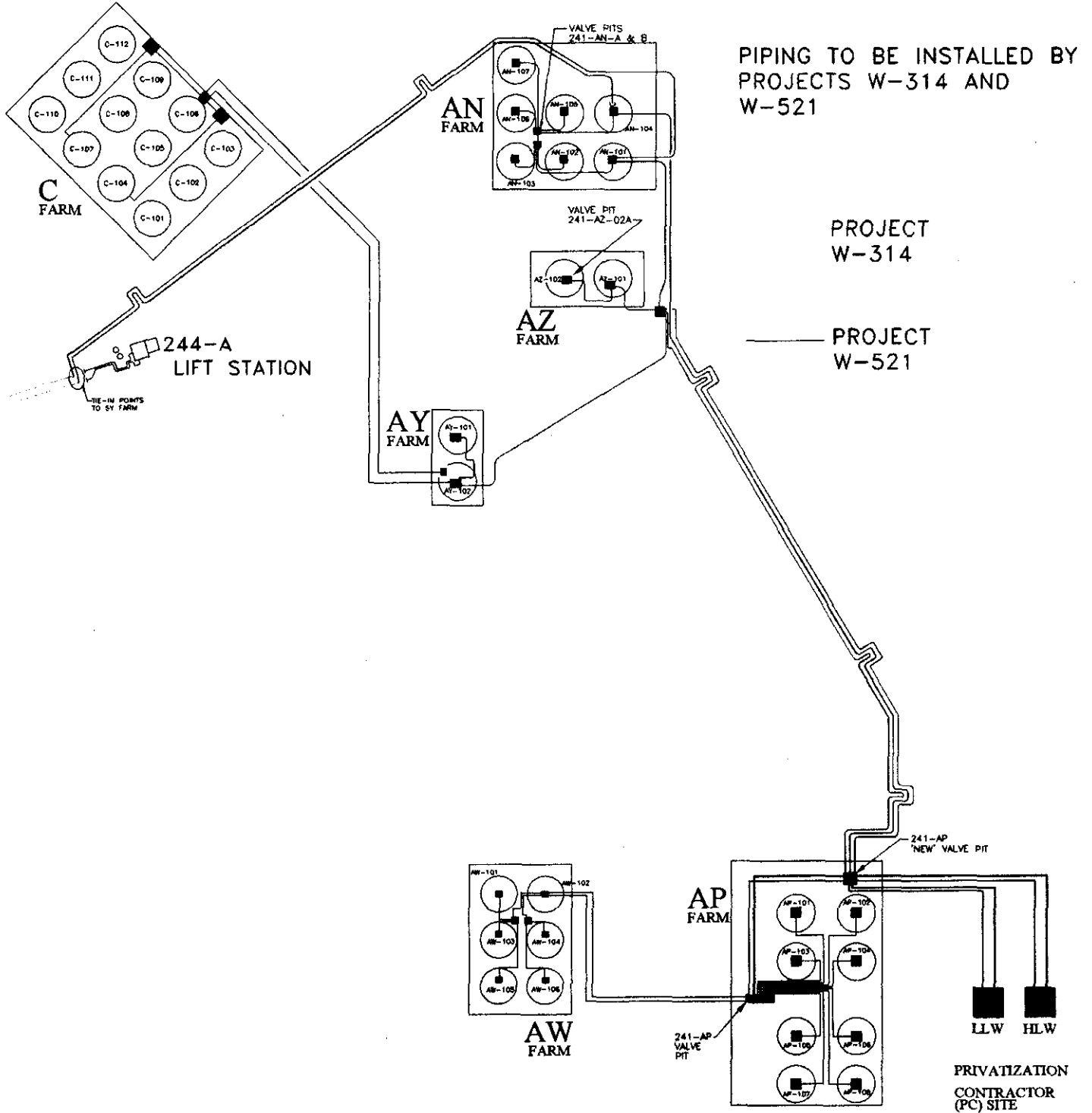
64B6010.DWG

Figure B-6. 10 Volume Percent Solids/ Pipe Upgraded to 400lb/in².



64B60400.DWG

Figure B-7. Projects W-314 and W-521.



PROJECTW314.DWG

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APPENDIX C

**SURFACE ROUGHNESS OF CARBON STEEL AND
STAINLESS STEEL WASTE TRANSFER LINES CAUSED
BY CORROSION AND SCALING**

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APPENDIX C

**SURFACE ROUGHNESS OF CARBON STEEL AND
STAINLESS STEEL WASTE TRANSFER LINES CAUSED
BY CORROSION AND SCALING**

November 5, 1999

Prepared By:

R. P. Anantamula
And
J. R. Divine

LOCKHEED MARTIN HANFORD CORPORATION
for the
U.S. Department of Energy
Richland Operations Office

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SURFACE ROUGHNESS OF CARBON STEEL AND STAINLESS STEEL WASTE TRANSFER LINES CAUSED BY CORROSION AND SCALING

C1.0 INTRODUCTION

The waste transfer system in the 200 East Area of the Hanford Site operated by Lockheed Martin Hanford Corporation (LMHC) has several pipelines of carbon steel and stainless steel. The two materials are mixed in some pipelines while other lines are of one material only. The waste transfer system contains 1000 ft of 3 in. carbon steel pipe and 6000 ft of 3 in. stainless steel pipe believed to be AISI Type 304 or 304L. Much of the line has been used for up to fifteen years and plans are to use it for an additional 35 years. Both radioactive waste and "raw" water, essentially untreated Columbia River water, will be passed through the lines which will then be drained before being used again. Because pump requirements depend strongly on the pipe roughness, LMHC has requested information on changes in roughness due to corrosion and scaling. Surface roughness estimates were requested over a time period of 50 years for carbon steel and over a period of 40 years for stainless steel. This report discusses the contributions to surface roughness from corrosion and scaling.

C2.0 BACKGROUND

The waste transfer lines from Tank Farms to British Nuclear Fuel Limited, Inc. (BNFL) will be used more than three times annually, while the intertank pipelines will be used on an average once per year. The transfer piping is sloped in such a way that at the end of the waste transfer the waste water will drain into the tank in which the transfer is made with no possibility of stagnant water remaining in the pipe. Each waste transfer campaign is expected to last a total of ten days. The raw water will be heated in skid-mounted "boilers" and hot water will be used when it is deemed necessary. Corrosion inhibitors will be added to the water prior to its being pumped into the transfer lines. No heating, other than perhaps trace-heating to maintain the temperature, will be done on the line. There are two types of wastes that will be processed through the system: a high level waste (HLW) feed at 80°C containing 5 vol% solids and a low activity waste (LAW) feed at 45°C containing 2 vol% solids. The pH of the waste solutions is expected to be > 7. The waste solutions are generally expected to be $\geq 1\text{M NaNO}_2$, $\geq 3\text{M NaNO}_3$ and $\geq 3\text{M NaOH}$. Between waste transfers, however, flush water containing raw water (Columbia River water) inhibited with 0.01M NaOH and 0.011M NaNO₂ will be used to flush the transfer lines after each transfer.

C3.0 CORROSION

C3.1 CORROSION BEHAVIOR OF CARBON AND STAINLESS STEELS

At the compositions quoted above, the waste solutions are benign toward carbon steel from a corrosion perspective (Anantatmula et al. 1994). Corrosion of stainless steel is expected to be negligible at the high pH and low temperature of the waste solutions containing low amounts of chloride. The inhibited flush water will not be aggressive toward carbon steel resulting in negligible corrosion. Raw water is usually corrosive to carbon steel unless it is inhibited or there is significant amounts of calcium and magnesium salts in the water that will form a protective scale. The Columbia River water, based on 1991 and 1994 analyses and the analysis in the scaling section of this report, has a Langelier index value of < 0 and Ryznar index value of > 6 . Waters with Langelier index value of < 0 and Ryznar index value of > 6 tend not to form scales and be corrosive to steel (a discussion of Langelier and Ryznar indices is given in section 4.2). However, the addition of inhibitors to the raw water is expected to make the flush water benign towards steel. Therefore, only the carbon steel pipeline carrying the raw water to the inhibitor feed point will be subjected to a high general corrosion rate of ~ 4 mils per year (0.1 mm/y) [1mil = 0.001 in.] (Truitt and Wagner 1985). Stainless steel corrodes at a low general corrosion rate of < 0.1 mil per year (0.0025 mm/y) when exposed to fresh water at normal temperatures. The presence of inhibitors in the water is not expected to have any deleterious effects on the stainless steel, because the material has excellent resistance to general corrosion in solutions containing hydroxide, nitrate and nitrite. The chloride content and temperature of the raw water and waste solution is low enough to preclude pitting and stress corrosion cracking (SCC) of stainless steel pipe and preserve its integrity.

Corrosion of materials in aqueous systems is influenced by the following factors:

- Physical configuration of the system
- Water chemistry
- Presence of solids in the water
- Flow rate
- Temperature and
- Presence of microbes.

For the present surface roughness assessment, contribution to corrosion by microbes will be ignored since microbial corrosion primarily involves pitting corrosion. Depending on their abrasiveness, the presence of solids in the water is generally presumed to have a profound effect on corrosion if flow rates are high. However, studies performed by Pacific Northwest Laboratory (Smith and Elmore 1992) on simulated aging wastes at up to 15 ft/sec fluid velocity indicated that the corrosion products were not removed from the carbon steel surface in spite of an increase in corrosion rate. The increase in corrosion rate was attributed to the ease with which fresh reactants were brought to the corroded surface by flowing waste as a result of a thinner boundary layer.

C3.2 SURFACE ROUGHNESS ESTIMATES - CORROSION

C3.2.1 Carbon Steel

The millscale (produced during hot rolling and forging) on carbon steel consists of three superimposed layers of iron oxides, viz., ferrous oxide (FeO) on the inside, magnetite (Fe₃O₄) in the middle, and ferric oxide (Fe₂O₃) on the outside. The relative portions of the three oxides vary with rolling temperature. A typical millscale on 9.5 mm plate would be about 50 μm thick and contains approximately 70% FeO, 20% Fe₃O₄, and 10% Fe₂O₃. Therefore, the surface roughness of as-received carbon steel pipe is ~ 2 mils (0.05 mm).

The contribution to transfer pipe surface roughness from corrosion is primarily from general corrosion mechanism, although from structural integrity standpoint pitting and SCC are very important mechanisms. Because higher corrosion rates will be experienced by the least used carbon steel transfer lines, to retain conservatism, it will be assumed that the waste transfer system will be used on an average once per year. Furthermore, each campaign is expected to last ten days. Because the waste solutions are compliant with double-shell tank waste specifications and the flush water is inhibited, with the exception of the short length of carbon steel pipeline carrying raw water, the general corrosion of carbon steel pipelines will be minimal. The corrosion contribution to surface roughness primarily occurs during the long waiting periods between the waste transfers. Immediately after the waste transfer, the flush water is drained into the receiving tank. The presence of any stagnant water in the pipe is detrimental to carbon steel because the inhibition characteristics of the water will be lost by a reaction of carbon dioxide in the air in the pipe with the hydroxide in the water. This could lead to pitting of carbon steel in near neutral pHs and if the water contains small amounts of chloride, pitting of the stainless steel is possible. Moreover, raw water may contain sufficient sulfate reducing bacteria to cause microbiologically influenced corrosion (MIC) in the stainless steel line. Savannah River Site and others have had MIC penetrate a pipe from the inside in a matter of weeks.

Assuming all the water drains into the receiving tank, a moist film of inhibited water will be left on the pipe surface. Some of the inhibition characteristics of the film will quickly be lost by a reaction of carbon dioxide in the air in the pipe with the hydroxide in the film. The effectiveness of the residual nitrite in the film as an inhibitor, however, is unknown. Because the pipe is open to the receiving tank, the interior of the pipe will be exposed to humid air conditions with a high relative humidity (RH).

Surface roughness of carbon steel under the humid air conditions described above can be estimated from the humid air corrosion models developed for carbon steel in the literature (Lee et al. 1996). The model, as modified by Anantatmula and Ohl (1996) was used for the estimates. The penetration by general corrosion is expressed as a function of time, RH and temperature as follows:

$$\ln D_g = a_0 + a_1 \ln(t) + a_2/RH + a_3/T$$

where D_g is corrosion depth in μm , t is time in years, T is temperature in $^\circ\text{K}$, and a_0 , a_1 , a_2 and a_3 are constants with the following values:

$$\begin{aligned} a_0 &= 16.9865 \\ a_1 &= 0.6113 \\ a_2 &= -893.76 \text{ and} \\ a_3 &= -833.53 \end{aligned}$$

The depth of penetration was calculated for times up to 50 years assuming 98% RH. An average temperature of 35°C was assumed although the temperature of waste solutions is expected to be in the range $45^\circ\text{-}80^\circ\text{C}$. This is because the total waste transfer operation lasts for ten days and the pipe temperature is expected to drop to ambient temperature well before the next waste transfer.

Based on the above assumptions, the depth of penetration was calculated for carbon steel as a function of time from the model equation above. Depending on the availability of oxygen, the corrosion product on the inside surface of the carbon steel transfer lines could be either Fe_3O_4 or Fe_2O_3 . Regardless of which oxide is assumed to be formed, the corrosion product thickness will be approximately twice that of the thickness of the metal lost. Using this information, conservative surface roughness estimates were made for the carbon steel pipe and are listed as a function of time in Table 1.

Table 1. Surface Roughness as a Function of Time for Carbon Steel.

Time (Years)	Surface Roughness (mils)
15	70
20	85
25	100
30	110
35	120
40	130
45	140
50	150

The surface roughness values in the above table should be considered conservative because the tanks are ventilated resulting possibly in $< 98\%$ RH in the transfer pipes. In addition, at the present time, it is not known just how long the moist nitrite film will protect the carbon steel after each waste transfer. For the present calculations, it has been assumed that the nitrite film will not offer any protection. Based on a limited number of visual examinations, the roughness may be much less than estimated in Table 1.

C3.2.2 Stainless Steel

Unlike carbon steel, negligible amount of millscale is expected on as-received stainless steel pipe since the surface is normally protected by a chromium oxide film. Based on the foregoing, stainless steel corrodes at a low general corrosion rate of < 0.1 mpy (0.003 mm/y) when exposed to fresh water at normal temperatures including temperatures above 100°C. Assuming a maximum general corrosion rate of 0.1 mpy (0.003 mm/y), a surface roughness value of 8 mils (0.203 mm) is estimated for a 40-yr time period. Including the thickness of the millscale on the as-received pipe, a maximum surface roughness value of 10 mils (0.254 mm) is predicted for the stainless steel line through 40 years of waste transfer operations.

C4.0 SCALING

C4.1 SCALING PROCESSES

Scaling is the deposition of soluble products from the liquid. When a chemical species exceeds its solubility limit, it has a tendency to precipitate. This tendency is increased by the amount the solubility is exceeded and by the presence of deposition nuclei. Other factors will include temperature and liquid shear forces (Becker 1998; Lane 1993).

If the precipitation occurs in the bulk liquid and forms a slurry, then deposits will be loose and easily removed unless they agglomerate while on the surface. On the other hand, if the deposition nucleus is on the surface, then the deposit will occur there and be more firmly attached.

- Waste Composition

As mentioned earlier, the waste composition is very benign from a corrosion standpoint. It is known that if the temperature of the waste stream is too low, some waste components will precipitate. In discussions with Mr D. A. Reynolds, LMHC, it was determined that flushing with hot water will remove such precipitates.

- Water Composition

The composition of the river water varies with the season and exact location of the sampling as well as uses of the river and surrounding lands change. Table 2 shows a composition measured in 1994.

Table 2. Columbia River Water Composition.

Component	Concentration
Calcium, as CaCO ₃	47.50 ppm (mg/l)
Magnesium, as equivalent CaCO ₃	18.50
Bicarbonate, as equivalent CaCO ₃	67.6
Chloride	1.00
Nitrate	<1.00
Sulfate	9.00
Silica, as SiO ₂	6.20
Total Dissolved Solids, TDS	71.1
Methyl Orange alkalinity, as CaCO ₃	55.4
pH	7.90

Laboratory Analysis 94-08393, ID 260170072, Collected 05/15/94, 200E Raw Water. Analyzed by WR Grace & Co., Lake Zurich, IL 60047.

- Solubility Effects

The solubility of most compounds increases with increasing temperature. CaSO₄ solubility, however, increases to about 40°C and then decreases with increasing temperature (Latimer and Hildebrand 1951); the solubility is about the same at both 25 and 60°C. Above about 63.5°C, the solubility decreases rapidly. At 25°C, the solubilities of CaSO₄ and CaCO₃ are given in Table 3. The solubility product of Fe(OH)₃ is also listed.

Table 3. Solubility Products.

Compound	Solubility Product
CaSO ₄	4.93×10^{-5}
CaCO ₃	2.8×10^{-9}
Fe(OH) ₃	2.79×10^{-39}

¹Dean 1999

The solubility of Ca⁺², given in Table 2 is expressed in terms of CaCO₃. Expressing it as Ca⁺², the concentration is about 19 mg/l or 4.8×10^{-7} mol/l. Sulfate, SO₄⁻, is 9 mg/l or about 9.4×10^{-8} mol/l. Thus the product of the calcium and sulfate concentrations is 4.5×10^{-14} which is much less than the solubility product for CaSO₄ given in Table 3. Consequently no CaSO₄ is expected to precipitate from the Hanford Raw Water.

In a similar calculation, Fe(OH)₃ is expected to be effectively totally insoluble. With a pH of 7.9, the OH⁻ concentration will be about 8×10^{-7} mol/l. As a result, the concentration of Fe⁺³ will not exceed about 5×10^{-21} mol/l which, effectively, is zero. Thus all the iron that

corrodes will react with excess oxygen and precipitate in the immediate location of the corrosion site and form scale.

C4.2 DISCUSSION OF SCALING

Based on information from the various sources, it is not believed that scaling from the waste will be a problem because it is feasible to re-dissolve the material with hot water.

Langelier developed an empirical relationship to predict the potential for scaling. It considers carbonate solubility, pH, conductivity, and temperature. It is a crude estimate because other factors such as other salts are ignored. Ryznar modified the relationship for use in flowing streams. There are also modifications for use when the total dissolved salts exceed about 7,000 ppm.

These empirical relationships are called the Langelier Index and the Ryznar Index and provide very crude estimates. When using them it is best to err on the conservative side so that only if the prediction is strongly in favor of scaling should it be assumed to occur. Calculating the Langelier (stagnant or low flow rate water) and the Ryznar (flowing water) Indices (Kemmer 1998) for the raw water, the following were obtained, Table 4.

Table 4. Scaling Indices.

Temperature	Langelier	Ryznar
10°C	-7	9.3
50°C	0.12	8.4
90°C	0.7	7.4

If the Langelier Index is negative, the water is considered non-scaling and corrosive. If the Ryznar Index is >6.0, the water is considered non-scaling and corrosive.

Because the water and the pipe will be hot only when the water is flowing, the conclusion is that the water will be somewhat corrosive (towards carbon steel) and non-scaling.

Consequently, most of the scaling due to carbonates would be expected to occur in the "boiler".

C5.0 CONCLUSIONS AND RECOMMENDATIONS

The surface roughness from corrosion for carbon steel is conservatively estimated as 150 mils for 50 years of waste transfer operations, while the corresponding value for stainless steel is expected to be a maximum of 10 mils. Scaling due to deposition from fluids in the pipe will not be a serious concern in the piping.

Based on the above estimates, the majority of the pressure drop will occur as a result of flow through the carbon steel part of the transfer pipeline. In order to check the validity of the estimates for carbon steel, it is recommended to inspect the inside surfaces of the carbon steel pipes that have been in operation for a known time period. Air drying of the pipes between waste transfers is recommended to reduce the surface roughness of the carbon steel pipes. The short section of carbon steel pipe carrying the raw water should also be checked periodically for its integrity. It is recommended to not let stagnant water remain in the transfer pipes, especially the stainless steel lines. Finally, if pressure drop must be minimized by minimizing the surface roughness of the transfer pipes, it is recommended to replace the carbon steel portion of the transfer line with stainless steel.

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ATTACHMENT C-1

**SUMMARY OF SEARCH FOR
TRANSFER LINE SOLIDS
BUILDUP/SCALING
INFORMATION**

OCTOBER 1999

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Summary of Search for Transfer Line Solids Buildup/Scaling Information, October 1999

Because of the long length of the transfer route, the pipe internal roughness is a major factor in determining the pump and piping design. For new lines, a significant increase in surface roughness with time in operation will significantly increase the pressure drop and must be accounted for in the design specification. Since there is no readily available information about the change in pipe roughness with time for a caustic waste transfer line, a brief and focused effort was made to find such information. The change in pipe roughness due to corrosion is investigated in a separate report.

Several sources of information were investigated and the results are included in the attached pages.

- Expert opinion and experience were collected from personnel at Hanford, Savannah River, and Idaho Falls.
- Hanford transfer line failure studies were reviewed
- Savannah River transfer line failure studies were discussed
- Hanford internal video inspections of transfer lines (no videos made available)

No quantitative information was found. However, there was also no event reported of chronic scale buildup.

To continue this effort to provide quantitative information, the following are recommended:

1. Review video inspections of Hanford transfer lines for evidence of adherent solids.
2. Evaluate two-phase transport of food products (sugars, salts, etc.) and problems/solutions to transport mechanisms.
3. Evaluate crystal formation, crystal size, adherence and solubility of waste salts.
4. Evaluate chemical production in high percent solids liquors and resultant production concerns (i.e., electro-chemical chlorine production in $\frac{1}{2}$ NaCl, $\frac{1}{2}$ NaOH solution).
5. Evaluate pipe linings and resistance to abrasion (in high percent solids), temperature, and thermal cycling and solids adherence after prolonged periods of non-use.
6. Evaluate available pipe interior finishes for weldments and general piping including mechanical and acid etching as a means to reduce solids adherence.
7. Evaluate stainless steel 300 series as the piping material of choice providing definitive data on its performance to: a) chloride attack in neutral or low pH solutions, b) scaling/solids build-up as related to increase in flow resistance, and c) solids build-up after long periods of non-use and dissolution of solids in water rinses.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND
LINE SELECTION**

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**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Charles Jenkins, SRS 803/725-1699

Finally got through to Chuck on October 13, 1999 to discuss Savannah River's transfer line scaling, related to roughness and line type. Chuck indicated that all cross-site or inter-area transfer lines are 304L stainless steel. A few intra-tank farm transfer lines are carbon steel. Stainless steel, by its nature, will not allow scaling buildup at the temperatures waste will be transferred under. If the temperature were 800° C, where oxidation could occur, scaling would be a significant problem.

Three stainless steel transfer lines have failed over the years, while no carbon steel lines have failed. The first SST line failure was due to chloride stress corrosion cracking. The transfer line was hydro-tested and the water was drained but the line was never dried, as called for in procedures. The pipe passed testing and set unused for years. The line was poorly sloped and was near the surface in 60 to 70 ° F soil. Retesting of the line prior to use showed the line had failed. The results of the inspection indicated chloride stress corrosion cracking caused the transfer pipe failure. The second incident of stainless steel transfer pipe failure was a result of a steam leak. A steam line leak had occurred near a transfer pipe and heated the surrounding soil for some time. The heating and evaporation of water from within and outside the transfer line caused failure. The failure was most probably due to continued chloride attack over several months and the resultant stress corrosion cracking. (This failure is most likely the same as reported in the Wiersma write-up.) The third failure of a stainless steel transfer line was caused by thermal fatigue from continued pipe expansion and subsequent contraction. Waste transfers at SRS are conducted using a steam-lift principle. Steam is injected into the waste and the resultant vacuum lifts the liquid waste. Therefore, steam becomes part of the liquid waste stream. The continued expansion and contraction of transfer piping lead to the thermal failure. The cause of this failure, transfer lines too rigidly attached, has since been corrected at SRS.

Chuck also provided some general information: (1) all inspected transfer lines at SRS have appeared clean and clear of build-up, (2) SRS always flushes lines after transfers with several volumes of water, (3) SRS would never consider installation of anything other than a stainless steel for a waste transfer line, (4) certain pipe lines that will see high levels of chloride will be fabricated from Hastelloy C or other high Ni alloys, (5) no scaling on stainless steel lines has been observed and (6) for caustic waste transfers use a 300 series, 304L, stainless steel pipe.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Bruce Wiersma, SRS 803/725-5439

Finally got with Bruce on October 12, 1999 to discuss any knowledge he has on waste transfer line scaling buildup as it relates to pipe roughness and adherence properties. Bruce indicated he is aware of only two instances of transfer pipe plugging or failure at SRS. The first instance concerned the plugging of a stainless steel gravity drain line from the evaporator. (Bruce noted that all waste transfer lines at SRS are stainless steel.) An inspection using an in-line viewing camera was conducted of the plugged line and concluded the evaporator clean out liquor was drained into the line where it cooled rapidly and crystallized, plugging the line. The internal surfaces of the pipe did not show any damage and in fact appeared quite smooth and clean.

The second waste transfer pipe mishap occurred when a steam line broke and caused a thermal failure in the pipe. An adjacent steam line that heated the stainless steel waste transfer line failed. The steam line heated the ground around the pipe and caused evaporation of the water flush in the transfer pipe. The high chloride attacked the piping welds causing stress corrosion cracking and failure of the pipe. The waste transfer pipe was made of 304L stainless steel. A write-up of the pipe failure and inspection is contained in the SRS internal report WSRC-TR-960356.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Scott Werry, LMHC, Richland, WA 509/373-1831

I had called Dave Smet to inquire about videos of waste transfer lines. Dave indicated that I should speak with Scott Werry concerning any video work. I finally spoke with Scott on October 12, 1999 about any videos he may have of tank transfer line inspections or in-line inspections. Scott indicated his database consists of tank and pit inspections and does not contain any transfer line inspections. We spoke of the SL-119 work and he again indicated that video work is not in his database. He indicated that if any NDE inspection equipment was put in transfer lines it would have to pass the flammable gas controls and the line would have to be purged, which would go through Scott. Also the Flammable Gas Board would review and approve the work and most probably a USQ would have been conducted. Scott recalls no such actions ever being conducted. Scott indicated that several years ago, under Westinghouse, he was in the process of assembling the entire video library of all tank farm work. After the site contract change, that effort was split and now no central group possess all the videos or even knows of their whereabouts. Scott suggested I call Mike Sumsion (376-4643) for additional 200 East Area videos and Joe Gonzalez (373-3056) for 200 West Area videos.

Scott called back and indicated there was a document that had "TEEM" in the title by EJ Walter that was done in the early 1990s that may be of some help. (The document was WHC-SD-WM-TEEM-001, *Pressure Testing of Underground Transfer Piping for Single-Shell Tank Salt Well.*) He also indicated that another document, HNF-SD-WM-ER-623, Rev 0 may have some data in a table at the end of the document. Scott again called and said I might also call Dan Pfluger (376-6164) as a source for additional information.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Earl Schwenk, COGEMA Engineering, Richland, WA 943-4438

Talked with Earl on October 12, 1999 and he indicated that he had conducted a failure analysis of a stainless steel transfer line about 20 years ago and the line was smooth other than the pitting. The analysis did not involve evaluation of pipe scaling, pipe roughness and associated adherence or pipe plugging but was a corrosion assessment. Earl provided an assessment of conditions that needed to be met, both materials and conditions, to assure successful transfer in waste piping. He first pointed out that stainless steel should be the pipe material of choice. Stainless steel will show very little general corrosion attack over time and therefore should provide no surface anomalies for scaling or solids build-up. No carbon steel should be used as it will pit and corrode too badly. A 300 series (304L or 316) stainless steel may not be the best choice of stainless if high chloride and neutral pH conditions may occur. A stainless steel that is higher in Ni or Cr may be needed to combat these conditions. The pH must be maintained well above 8.0. Even pH in the neutral range (6.0 -7.0) can cause chloride stress corrosion cracking and pitting, degradation of welds, and pipe failure. Suspended solids at 30 wt% will require turbulent flow for transfer. If waste streams with this amount of solids are transferred in laminar flow the solids will undoubtedly settle out or collect. Special piping, such Teflon-lined, would undoubtedly suffer from abrasion with all the solids and Earl has no quantitative data on how such lined piping would hold up in this condition. It was agreed that a smooth surface of the piping would be essential for transfer of the tank wastes. Earl indicated Jim Divine would know much more concerning tank waste transfer line attack by chloride and corrosion cracking in neutral pH conditions and I should contact Jim for more on that subject. Earl also suggested I contact previous Grout Project engineers to ascertain what wt% solids and what specific conditions of the waste or equipment bounded grout waste transfers. Earl indicated I should also contact other industries to obtain two-phase material transfer data.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Richard W. Johnson, INEEL 208/526-0955

A phone conversation was held with Richard on October 6, 1999 to discuss pipe roughness and adherence properties that may be in the *Handbook Of Fluid Dynamics* he had recently edited. (Richard's name was given to me by Jim Divine, ChemMet LTD, as a possible contact that may have information on caustic waste transport issues.) Richard indicated no such material is contained in the edition and he has no knowledge or working experience with caustic waste transport, pipe roughness or piping adherence properties. Richard did indicate that I might check with Dick Rice, INEEL, (208/526-1992) who has worked in the field of two-phase transport and may have some information that may be of help to me.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Fred Reich, COGEMA Engineering, Richland, WA 509/371-8046

I talked with Fred on October 12, 1999 about any past experience he may have had with NDE testing of waste transfer lines and results he obtained concerning detection of corrosion, scaling products, etc. Fred indicated his only experience has been in the petrol-chemical field detecting corrosion in iron piping. A pipe crawler using pulse-echo ultrasonics was used in petroleum cross-site transfer lines to detect pitting and general corrosion. No work was ever done for the detection of corrosion products or scaling buildup. In petroleum lines "sour crude" contained sulfur that would react with the iron creating corrosion. The crawler was designed to operate in 18-inch, or larger lines, while oil, gasoline, or diesel were running in the lines. Fred indicated no work was done with stainless steel pipe lines. Fred has had no experience with external NDE of waste transfer lines and does not know of anyone who has had such experience.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Dick Rice, INEEL, Idaho Falls, ID 208/526-1992

On October 6, 1999 I left a message for Dick to call me on his two-phase transport work as it may relate to caustic waste transfer. On October 7, Tish Stoots (208/526-1764) returned my call to obtain more information for Dick. Tish indicated she would get back to me with any information on the subject. On October 12, I again called Tish to inquire of any progress on the subject. She indicated she had spoken with Dick and he had no quantitative data on two-phase transport that would support Hanford high-level waste transport.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

James Divine, ChemMet Ltd, West Richland, WA 509/967-2309

On October 4, 1999 I spoke with Jim Divine concerning his knowledge of scaling products deposition in tank transfer lines and associated roughness and adherence properties. Jim indicated that that no PNL studies were done in the past to evaluate the type of corrosion and resultant deposition. He indicated that if the transfer piping were to be made of stainless steel there would be no pitting corrosion only deposit accumulation. A carbon steel transfer line would exhibit pitting corrosion over time. Jim has had experience in the 300 Area with 6 and 8-inch water lines that have closed to 2-inch with deposition but nothing akin to tank wastes. Jim indicated that I should call Richard W. Johnson at INEEL since he had either written or edited a *Handbook of Fluid Dynamics* and may have some information.

On October 5, 1999 Jim called back to state he didn't know of other contacts at any other DOE sites that would have any quantitative data concerning this subject. Jim did suggest that some simple experiments could be proposed that would produce data on pipe roughness and scale adherence affects on pump pressure output. But these data would be nothing different than calculations found in standard publications on momentum transfer.

Jim was very confident that no quantitative data exists on tank waste transfer lines scaling and adherence properties and was not aware of any specific work on adherence to stainless steel.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Darrel Duncan, NHC, Richland, WA 509/372-1013

On October 4, 1999 I spoke with Darrel Duncan concerning his Hanford corrosion history and any knowledge he may have concerning tank transfer line corrosion product buildup or assessments done on removed piping. Darrel's pre-Hanford experience was in the petrol-chemical industry determining pipeline corrosion and weld longevity. At Hanford his corrosion work has centered on materials, container integrity and longevity and has not involved any work with tank transfer lines. He indicated that any scaling data from removed piping would be suspect because of the many flushes required to reduce the dose so that the material could be examined. These repeated flushes would undoubtedly remove much of the scale buildup rendering results questionable.

We spoke of previous NDE work and came to the conclusion that it was doubtful that any inline transfer data gathered could be quantitative. He indicated I should still speak with the NDE group in the 306 Building.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

David Lini, SAIC, Richland, WA 509/373-9102

On October 5, 1999 Dave returned my call to discuss his 1975 document "*Compilation of Hanford Corrosion Studies*", ARH-ST-111. Dave indicated the corrosion assessment in the document was centered on finding data to support either the "leave in place option" or the "retrieval option" for wastes contained in the high-level waste tanks. These two options were on the forefront at Hanford since deactivation of the 149 single-shell tanks was underway and future storage of all high-level liquid wastes was designated for double-shell tanks. The document focused on tank integrity assessments in an attempt to determine tank longevity. As Dave remembers, there is no information on tank transfer lines or any assessment of transfer line corrosion, product buildup and subsequent line plugging, or transfer line failure within the document. Dave did not know of any work done with caustic waste transfer lines concerning scaling buildup or transfer line pipe roughness assessments nor did he know of anyone on site that had such information.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Wesley Nelson (509/376-5403) and John Keve (509/376-8061), COGEMA Engineering Corporation, Richland, WA

On October 6, 1999 I spoke with John and Wes concerning their work experience concerning any non-destructive examinations of removed or in-place tank transfer line piping. Neither had any experience conducting such examinations and stated that the 306 NDE shop had never done this type of work for tank transfer piping.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Daniel Reynolds, LMHC, Richland, WA 509/373-3115

Finally caught up with Dan on October 13, 1999. Dan indicated that almost all waste transfers in the last several years have been evaporator feed that contain very little solids. Volume % solids have not been traditionally reported in waste transfers. In approximately the last five years compatibility assessments of the transfer and receiving tanks are to be done. The assessment may contain SPG but claim no solids. A grab sample may be taken but is not necessarily representative since mixer pumps do not have to run for any period of time. Tank 101-SY solution transfer will utilize bulk density versus solution density to determine volume % suspended solids. Inline dilution instrumentation will be utilized to maintain the slurry <25 vol% solids. It is assumed that 60% of the solids in 101-SY, mainly NaNO₃ and NaNO₂, will dissolve as water is added. Tank C106 will be monitored for % solids as it is sluiced out. The same vol% solids measuring system will be used as 101-SY. However, the solids will be maintained at <15 vol% during the transfer of the C106 slurry.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Jim Douglas, COGEMA Engineering, Richland, WA 509/372-0989

I spoke with Jim on October 7, 1999 to find out what data was provided concerning waste tank transfers. The data Jim has been provided as he has compiled tank waste characterization documentation are:

- Total alpha
- Sodium (Na)
- Aluminum (Al)
- Nitrate (NO₃)
- Nitrite (NO₂)
- Hydroxide (OH)
- Specific gravity
- Sulfate (SO₄)
- Phosphate (PO₄)

Jim believes the average temperature for waste being transferred is approximately 75 °C. He indicated I should contact Jim Field of LMHC to double check the data provided with waste transfers. Jim indicated he has never seen a wt% solids value in any of the waste transfer data.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Jim Field, LMHC, Richland, WA 509/376-3753

Talked with Jim on October 13, 1999 concerning data available concerning waste transfers. Jim supported Jim Douglas's conversation in the constituents reported in waste transfers. Jim did indicate that grab samples were conducted to determine % solids. He also indicated that following waste transfers, three pipe volumes of water are used to flush the line(s). Flush volumes are being contested in lines that are of great length. Jim also stated that recommendations are continually made to turn on transfer line trace heaters to match the temperature of the tank the transfer is coming from. Average transfer temperature is something hard to quantify. The range of transfer liquids can vary by almost 150 °F as line length, trace heater condition/settings, initial waste temperature, water added to waste prior to transfer and other parameters can affect delivery temperature.

**HANFORD TRANSFER LINE PIPE ROUGHNESS AND SCALING
ADHERENCE PROPERTIES AFFECTING PUMPING EQUIPMENT AND LINE
SELECTION**

TELEPHONE CONFERENCE WITH:

Bill Morris, LMHC, Richland, WA 509/372-3724

Bill called on October 13, 1999 and indicated Joe Gonzalez had no knowledge of transfer line videos but he did. Bill is the camera group lead and recalls videos of the east-west transfer line video and the AY Tank Farm transfer line video inspection with a boroscope. He recalls that the videos were taken in the early 1990s. Bill will review his video records on transfer line inspections this afternoon and call me back with information concerning the videos.

**LIST OF DOCUMENTS CONCERNING INSPECTIONS OR TESTING OF
UNDERGROUND WASTE TRANSFER LINES**

1. WHC-SD-WM-ANAL-014, 1994, *S Tank Farm SL-119 Saltwell Piping Failure Analysis Report*, Rev. 0, Westinghouse Hanford Company, Richland, Washington.
2. WHC-SD-WM-ES-259, 1993, *Single-Shell Saltwell Transfer Piping Evaluation*, Rev. 0, Westinghouse Hanford Company, Richland, Washington.
3. WHC-SD-WM-TEEM-001, 1992, *Pressure Testing of Underground Transfer Piping for Single-Shell Tank Saltwell*, Rev. 0, Westinghouse Hanford Company, Richland, Washington.
4. HNF-SD-WM-ER-623, 1997, *DST Waste Transfer Piping and Pit System Integrity Assessment Report*, Rev. 0, Westinghouse Hanford Company, Richland, Washington.
5. WSRC-TR-96-0356, 1996, *Materials Degradation Evaluation of High Level Waste Transfer Line System (U)*, Westinghouse Savannah River Company, Aiken, South Carolina.

APPENDIX D

**CRITICAL VELOCITY AND PRESSURE LOSS EVALUATION RESULTS
FOR DOUBLE-SHELL TANK TRANSFER PIPING IN 200 EAST AREA FOR
TANK-TO-TANK AND TANK-TO-BNFL INC. TRANSFERS BASED ON
HANFORD TANK WASTE OPERATION SIMULATOR CASES 3S4
AND 3S6 DISTINCT TRANSFER ROUTE IDENTIFICATION FOR
PROPOSED PHASE 1B WASTE FEED DELIVERY TRANSFERS**

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CONVERSIONS

ft	feet = 0.3048 meter (m)
in.	inch = 2.54 centimeters (cm)
lbf/in ²	pounds per square inch = 0.06895 megapascals (MPa)
mils	0.001 in. = 0.0254 millimeter (mm)
gpm	0.0631 liter/second (L/s)
cP	centipose = 0.001 kilogram/meter-second (kg/m-s)

Table D-1. 3S4 Case Summary.

Case	Group 1 Properties						Group 2 Properties						Results			Table
	Liquid			Solids			Liquid			Solids			Critical Velocity (ft/s)	Temperature (°C)		
	PL Density (kg/L)	P _s Density (kg/L)	C _s Volume (%)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	P _L Density (kg/L)	P _s Density (kg/L)	C _s Volume (%)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	Critical Velocity (ft/s)	Temperature (°C)				
													10	60		
1a	1.21		15		1.4	1.46			1.92	2.7	2.9		D-2			
1b	1.1	2.5		5	1.31	1.2			1.74	3.7	4		D-3			
1c	1.21		1.5 x HTWOS		1.21-1.35	0.3-2.5	0.1-2.7	1.46	1.46-1.68	1.0-2.6	0.4-2.8		D-4			
1d	1.1				1.1-1.25	0.2-2.8	0.1-3.1	1.2	1.2-1.45	0.3-3.5	0.1-3.8		D-5			
2a	1.21		15		1.4	3.7	4	1.46	1.92	4	4.3		D-6			
2b	1.1	2.5		40	1.31	4.1	4.5	1.2	1.74	5.4	5.9		D-7			
2c	1.21		1.5 x HTWOS		1.21-1.35	0.3-3.6	0.1-3.9	1.46	1.46-1.68	1.0-3.8	0.4-4.1		D-8			
2d	1.1				1.1-1.25	0.2-4.0	0.1-4.4	1.2	1.2-1.45	0.3-5.2	0.1-5.6		D-9			
3a	1.21		15		1.4	4.8	5.3	1.46	1.92	5	5.4		D-10			
3b	1.1	2.5		200	1.31	5.4	5.9	1.2	1.74	6.8	7.4		D-11			
3c	1.21		1.5 x HTWOS		1.21-1.35	0.3-4.7	0.1-5.1	1.46	1.46-1.68	1.0-4.8	0.4-5.2		D-12			
3d	1.1				1.1-1.25	0.2-5.2	0.1-5.7	1.2	1.2-1.45	0.3-6.5	0.1-7.1		D-13			
4	1.2	3	10	400	1.38	6.3	6.8	1.2	1.47	6.5	7.1		D-14			
	3S4 HTWOS 0.0-7.07						1 ft/s = 0.3048 m/s									

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Case No.	Transfer		Transfer Dates (M/Y)	3-in. Schedule 40 Pipe (3.068 in. inside dia.)	Total Change in Elevation (ft)	Group 1 Properties										Group 2 Properties										Notes																			
	From Tank	To Tank				Liquid	Solids	Liquid	Solids	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)																								
Transfer		Transfer		Start	End	Density (kg/L)	Density (kg/L)	C _v (%)	d _p (µm)	d _p Media Dia. (µm)	ρ _m Bulk Density (kg/L)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Notes																					
A	B	C	D																						E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U	V	W	X	Y
1	AN-101	AP-103	83-86	1	3/03	3/03	400	306	2,774	12	1.21	2.50	15	5	1.40	2.6	60	27	35	256	237	2.8	65	27	38	311	240	1.48	3.00	30	10	1.92	2.7	62	54	82	216	188	2.9	67	50	61	237	196	New or like new pipeline with maximum expected pipe roughness of 2 mils (0.002 inches).
2	AN-101	AP-103	83-86	2	3/03	3/03	400	442	626	0	1.21	2.50	15	5	1.40	2.6	60	7	13	525	343	2.8	65	7	15	539	345	1.48	3.00	30	10	1.92	2.7	62	15	22	395	284	2.9	67	14	23	425	290	
3	AN-101	AP-103	83-86	3	9/05	8/12	400	557	626	0	1.21	2.50	15	5	1.40	2.6	60	8	15	498	317	2.8	65	8	17	511	319	1.46	3.00	30	10	1.92	2.7	62	17	26	373	263	2.9	67	15	27	402	268	
4	AN-101	AP-103	83-86	4	10/08	12/14	400	641	40	0	1.21	2.50	15	5	1.40	2.6	60	66	75	198	171	2.8	65	66	79	208	175	1.46	3.00	30	10	1.92	2.7	62	126	133	2.9	67	117	129	153	402	268		
5	AN-101	AP-103	83-86	5	8/06	8/06	400	319	626	0	1.21	2.50	15	5	1.40	2.6	60	6	11	559	379	2.8	65	6	12	573	381	1.46	3.00	30	10	1.92	2.7	62	14	19	423	313	2.9	67	12	19	453	320	
6	AN-101	AP-103	83-86	6	10/08	10/08	400	405	421	0	1.21	2.50	15	5	1.40	2.6	60	5	11	599	377	2.8	65	5	12	614	378	1.46	3.00	30	10	1.92	2.7	62	12	18	455	314	2.9	67	11	19	487	319	
7	AN-101	AP-103	83-86	7	12/12	12/12	400	329	626	0	1.21	2.50	15	5	1.40	2.6	60	6	11	599	377	2.8	65	6	12	570	378	1.46	3.00	30	10	1.92	2.7	62	14	19	420	310	2.9	67	12	20	451	317	
8	AN-101	AP-103	83-86	8	10/11	10/11	400	300	626	0	1.21	2.50	15	5	1.40	2.6	60	6	10	565	385	2.8	65	6	12	579	387	1.46	3.00	30	10	1.92	2.7	62	13	18	427	318	2.9	67	12	19	458	325	EOI = pipeline at end of life (50 years for carbon steel line, 40 years for stainless steel line) with pipe roughness due to corrosion estimated at 150 mils for carbon steel and 10 mils for stainless steel.
9	AN-101	AP-103	83-86	9	5/13	5/18	400	376	626	0	1.21	2.50	15	5	1.40	2.6	60	7	14	605	324	2.8	65	7	16	518	325	1.46	3.00	30	10	1.92	2.7	62	17	25	379	288	2.9	67	15	26	408	273	
10	AN-101	AP-103	83-86	10	6/09	4/11	400	6	6,275	40	1.21	2.50	15	5	1.40	2.6	60	65	74	200	173	2.8	65	65	66	78	210	177	1.46	3.00	30	10	1.92	2.7	62	124	131	2.9	67	115	127	155	408	273	
11	AN-101	AP-103	83-86	11	6/09	6/09	400	562	1,560	5	1.21	2.50	15	5	1.40	2.6	60	17	25	365	281	2.8	65	17	28	378	263	1.46	3.00	30	10	1.92	2.7	62	35	45	268	211	2.9	67	32	45	292	218	
12	AN-101	AP-103	83-86	12	6/09	6/09	400	489	421	0	1.21	2.50	15	5	1.40	2.6	60	6	12	570	352	2.8	65	6	14	585	363	1.46	3.00	30	10	1.92	2.7	62	13	21	431	283	2.9	67	12	22	463	298	
13	AN-101	AP-103	83-86	13	4/11	4/11	400	439	626	0	1.21	2.50	15	5	1.40	2.6	60	7	13	528	344	2.8	65	7	15	540	345	1.46	3.00	30	10	1.92	2.7	62	15	22	396	285	2.9	67	14	23	423	290	
14	AN-101	AP-103	83-86	14	9/11	9/11	400	1,012	3,046	21	1.21	2.50	15	5	1.40	2.6	60	39	55	268	188	2.8	65	39	60	268	188	1.46	3.00	30	10	1.92	2.7	62	76	94	148	149	2.9	67	70	95	201	155	
15	AN-101	AP-103	83-86	15	8/12	8/12	400	994	3,046	21	1.21	2.50	15	5	1.40	2.6	60	39	55	257	188	2.8	65	39	60	268	188	1.46	3.00	30	10	1.92	2.7	62	75	93	148	149	2.9	67	70	94	202	155	
16	AN-101	AP-103	83-86	16	6/13	10/13	400	410	626	0	1.21	2.50	15	5	1.40	2.6	60	7	12	533	351	2.8	65	7	14	547	353	1.46	3.00	30	10	1.92	2.7	62	15	21	402	291	2.9	67	13	22	432	297	
17	AN-101	AP-103	83-86	17	1/14	5/14	400	374	421	0	1.21	2.50	15	5	1.40	2.6	60	5	10	611	387	2.8	65	5	12	626	389	1.46	3.00	30	10	1.92	2.7	62	11	17	485	323	2.9	67	10	18	497	328	
18	AN-101	AP-103	83-86	18	1/15	12/15	400	1,003	3,046	21	1.21	2.50	15	5	1.40	2.6	60	39	55	257	187	2.8	65	39	60	268	189	1.46	3.00	30	10	1.92	2.7	62	76	93	148	149	2.9	67	70	95	202	155	
19	AN-101	AP-103	83-86	19	7/15	7/15	400	384	421	0	1.21	2.50	15	5	1.40	2.6	60	5	10	608	384	2.8	65	5	12	622	385	1.46	3.00	30	10	1.92	2.7	62	10	18	461	320	2.9	67	10	18	484	325	
20	AN-101	AP-103	83-86	20	2/17	2/17	400	419	2,636	12	1.21	2.50	15	5	1.40	2.6	60	27	36	300	232	2.8	65	27	38	312	235	1.46	3.00	30	10	1.92	2.7	62	54	63	217	185	2.9	67	50	63	238	193	
21	AN-101	AP-103	83-86	21	2/17	2/17	400	302	2,636	12	1.21	2.50	15	5	1.40	2.6	60	27	36	300	232	2.8	65	27	38	312	235	1.46	3.00	30	10	1.92	2.7	62	54	63	217	185	2.9	67	50	63	238	193	
22	AN-101	AP-103	83-86	22	3/18	3/18	400	1,125	3,046	21	1.21	2.50	15	5	1.40	2.6	60	40	57	253	183	2.8	65	40	52	622	385	1.46	3.00	30	10	1.92	2.7	62	77	97	181	145	2.9	67	72	98	199	157	
23	AN-101	AP-103	83-86	23	4/19	4/19	400	431	2,636	12	1.21	2.50	15	5	1.40	2.6	60	27	36	299	231	2.8	65	27	39	311	234	1.46	3.00	30	10	1.92	2.7	62	54	63	217	185	2.9	67	50	63	238	193	
24	AN-101	AP-103	83-86	24	6/09	6/09	400	1,114	3,046	21	1.21	2.50	15	5	1.40	2.6	60	40	57	253	183	2.8	65	40	52	622	385	1.46	3.00	30	10	1.92	2.7	62	77	97	181	145	2.9	67	72	98	199	157	
25	AN-101	AP-103	83-86	25	9/05	9/05	400	452	7,028	40	1.21	2.50	15	5	1.40	2.6	60	5	10	608	384	2.8	65	5	12	622	385	1.46	3.00	30	10	1.92	2.7	62	10	18	461	320	2.9	67	10	18	484	325	
26	AN-101	AP-103	83-86	26	8/08	1/14	400	442	7,028	40	1.21	2.50	15	5	1.40	2.6	60	73	88	182	152	2.8	65	73	88	182	155	1.46	3.00	30	10	1.92	2.7	62	141	155	2.9	67	131	152	141	408	273		
27	AN-101	AP-103	83-86	27	2/10	4/12	400	269	421	0	1.21	2.50	15	5	1.40	2.6	60	4	8	658	432	2.8	65	4	9	673	434	1.46	3.00	30	10	1.92	2.7	62	10	14	502	359	2.9	67	9	15	536	366	
28	AN-101	AP-103	83																																										

Case 1b	Group 1 Properties										Group 2 Properties										Notes																							
	Liquid					Solids					Critical					Critical																												
Transfer	From Tank	To Tank	Tank Farm Built (Yr)	Transfer Dates (M/Yr)	3-in. Schedule-40 Pipe (3.068 in. inside dia.)	Carbon Steel Length (ft)	Stainless Steel Length (ft)	Total Change in Elevation (ft)	K	L	M	N	O	P	Q	R	S	T	U	V	W	X	Y	Z	AA	AB	AC	AD	AE	AF	AG	AH	AI	AJ	AK	AL	AM	AN	AO	AP	AQ	AR	AS	
																																												ρ _L Density (kg/L)
1	AN-101	AN-103	83-86	1	3/03	3/03	400	306	2,774	12	1.10	2.50	15	5	1.31	2.9	67	29	38	314	247	3.2	74	29	42	324	249	1.20	3.00	30	10	1.74	3.7	84	58	76	261	211	92	58	83	274	215	New = new or like new pipeline with maximum expected pipe roughness of 2 mils (0.002 inches).
2	AN-101	AN-103	80-81	2	3/03	3/03	400	442	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	540	357	3.2	74	8	17	540	357	1.20	3.00	30	10	1.74	3.7	84	19	37	438	284	92	19	42	455	286	
3	AN-101	AN-103	80-81	3	9/05	8/12	400	557	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	540	357	3.2	74	8	17	540	357	1.20	3.00	30	10	1.74	3.7	84	19	37	438	284	92	19	42	455	286	
4	AN-101	AN-103	80-81	4	10/08	12/14	400	641	3	40	1.10	2.50	15	5	1.31	2.9	67	8	17	540	357	3.2	74	8	17	540	357	1.20	3.00	30	10	1.74	3.7	84	19	37	438	284	92	19	42	455	286	
5	AN-101	AN-103	80-81	1	8/06	8/06	400	319	626	0	1.10	2.50	15	5	1.31	2.9	67	6	12	584	394	3.2	74	6	12	584	394	1.20	3.00	30	10	1.74	3.7	84	15	26	528	337	92	15	30	547	349	
6	AN-101	AN-103	80-81	1	10/08	10/08	400	405	626	0	1.10	2.50	15	5	1.31	2.9	67	6	12	584	394	3.2	74	6	12	584	394	1.20	3.00	30	10	1.74	3.7	84	15	26	528	337	92	15	30	547	349	
7	AN-101	AN-103	80-81	1	12/12	12/12	400	329	626	0	1.10	2.50	15	5	1.31	2.9	67	7	12	581	389	3.2	74	7	12	581	389	1.20	3.00	30	10	1.74	3.7	84	15	26	528	337	92	15	30	547	349	
8	AN-101	AN-103	80-81	1	10/11	10/11	400	300	626	0	1.10	2.50	15	5	1.31	2.9	67	7	12	590	400	3.2	74	7	12	590	400	1.20	3.00	30	10	1.74	3.7	84	15	26	528	337	92	15	30	547	349	
9	AN-101	AN-103	80-81	1	5/13	5/13	400	376	626	0	1.10	2.50	15	5	1.31	2.9	67	7	12	590	400	3.2	74	7	12	590	400	1.20	3.00	30	10	1.74	3.7	84	15	26	528	337	92	15	30	547	349	
10	AN-101	AN-103	80-81	1	9/05	5/15	400	526	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
11	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
12	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
13	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
14	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
15	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
16	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
17	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
18	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
19	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
20	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
21	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
22	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
23	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
24	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
25	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
26	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
27	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
28	AN-101	AN-103	80-81	1	6/09	6/09	400	626	626	0	1.10	2.50	15	5	1.31	2.9	67	8	17	527	335	3.2	74	8	17	527	335	1.20	3.00	30	10	1.74	3.7	84	18	35	444	290	92	18	40	461	292	
29	AN-101	AN-103																																										

CASE 394 Transfer List - Tank-to-Tank		Group 1 Properties										Group 2 Properties										Temperature = 60 °C										Temperature = 10 °C										Temperature = 60 °C										Temperature = 10 °C										Temperature = 60 °C										Temperature = 10 °C										Temperature = 60 °C																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																					
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466	467	468	469	470	471	472	473	474	475	476	477	478	479	480	481	482	483	484	485	486	487	488	489	490	491	492	493	494	495	496	497	498	499	500	501	502	503	504	505	506	507	508	509	510	511	512	513	514	515	516	517	518	519	520	521	522	523	524	525	526	527	528	529	530	531	532	533	534	535	536	537	538	539	540	541	542	543	544	545	546	547	548	549	550	551	552	553	554	555	556	557	558	559	560	561	562	563	564	565	566	567	568	569	570	571	572	573	574	575	576	577	578	579	580	581	582	583	584	585	586	587	588	589	590	591	592	593	594	595	596	597	598	599	600	601	602	603	604	605	606	607	608	609	610	611	612	613	614	615	616	617	618	619	620	621	622	623	624	625	626	627	628	629	630	631	632	633	634	635	636	637	638	639	640	641	642	643	644	645	646	647	648	649	650	651	652	653	654	655	656	657	658	659	660	661	662	663	664	665	666	667	668	669	670	671	672	673	674	675	676	677	678	679	680	681	682	683	684	685	686	687	688	689	690	691	692	693	694	695	696	697	698	699	700	701	702	703	704	705	706	707	708	709	710	711	712	713	714	715	716	717	718	719	720	721	722	723	724	725	726	727	728	729	730	731	732	733	734	735	736	737	738	739	740	741	742	743	744	745	746	747	748	749	750	751	752	753	754	755	756	757	758	759	760	761	762	763	764	765	766	767	768	769	770	771	772	773	774	775	776	777	778	779	780	781	782	783	784	785	786	787	788	789	790	791	792	793	794	795	796	797	798	799	800	801	802	803	804	805	806	807	808	809	810	811	812	813	814	815	816	817	818	819	820	821	822	823	824	825	826	827	828	829	830	831	832	833	834	835	836	837	838	839	840	841	842	843	844	845	846	847	848	849	850	851	852	853	854	855	856	857	858	859	860	861	862	863	864	865	866	867	868	869	870	871	872	873	874	875	876	877	878	879	880	881	882	883	884	885	886	887	888	889	890	891	892	893	894	895	896	897	898	899	900	901	902	903	904	905	906	907	908	909	910	911	912	913	914	915	916	917	918	919	920	921	922	923	924	925	926	927	928	929	930	931	932	933	934	935	936	937	938	939	940	941	942	943	944	945	946	947	948	949	950	951	952	953	954	955	956	957	958	959	960	961	962	963	964	965	966	967	968	969	970	971	972	973	974	975	976	977	978	979	980	981	982	983	984	985	986	987	988	989	990	991	992	993	994	995	996	997	998	999	1000

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Table D-15. 3S6 Case Summary.

Case	Group 1 Properties						Group 2 Properties						Results			Table
	Liquid			Solids			Liquid			Solids			Critical Velocity (ft/s)	Temperature (°C)		
	P _L Density (kg/L)	P _s Density (kg/L)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	C _s Volume (%)	P _L Density (kg/L)	P _s Density (kg/L)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	C _s Volume (%)	Critical Velocity (ft/s)	Temperature (°C)				
													10	60		
1a	1.21			1.4	15	1.46		1.92		2.7	2.8	10	2.7	2.9	D-16	
1b	1.1	2.5	5	1.31		1.2		1.74		3.2	3.2	10	3.7	4	D-17	
1c	1.21			1.21-1.29	1.5 x HTWOS	1.46		1.46-1.66	3	0.3-2.5	0.1-2.7		1.0-2.6	0.4-2.8	D-18	
1d	1.1			1.1-1.24		1.2		1.2-1.43		0.2-2.8	0.1-3.0		0.3-3.5	0.1-3.8	D-19	
2a	1.21			1.4	15	1.46		1.92		3.7	4		4	4.3	D-20	
2b	1.1	2.5	40	1.31		1.2		1.74		4.1	4.5		5.4	5.9	D-21	
2c	1.21			1.21-1.33	1.5 x HTWOS	1.46		1.46-1.66	3	0.3-3.5	0.1-3.8		1.0-3.8	0.4-4.1	D-22	
2d	1.1			1.1-1.24		1.2		1.2-1.43		0.2-3.9	0.1-4.3		0.3-5.1	0.1-5.6	D-23	
3a	1.21			1.4	15	1.46		1.92		4.8	5.3		5	5.4	D-24	
3b	1.1	2.5	200	1.31		1.2		1.74		5.4	5.9		6.8	7.4	D-25	
3c	1.21			1.21-1.33	1.5 x HTWOS	1.46		1.46-1.66	3	0.3-4.6	0.1-5.0		1.0-4.8	0.4-5.1	D-26	
3d	1.1			1.1-1.24		1.2		1.2-1.43		0.2-5.2	0.1-5.6		0.3-6.4	0.1-7.0	D-27	
4	1.2	3	400	1.38	10	1.2		1.47		6.3	6.8		6.5	7.1	D-28	
4-st															D-29	
		3S6 HTWOS	0.0-6.45							1 ft/s = 0.3048 m/s						
	Case 4-st assumes a pump lift station in AP Farms located at the AP "new" valve pit. Selected long intermediate tank-to-tank transfers and all transfers to BNFL would go through the pump lift station.															

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Case 2d	Group 1 Properties										Group 2 Properties										Temperature = 10 °C										Temperature = 60 °C										Temperature = 80 °C										Notes
	Liquid	P _s Density (kg/L)	C _v Volume (%)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	Velocity (ft/s)	Flow Rate (gpm)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe (gpm)	Design Pressure (400 lb/ft ²)	Flow Rate at a Pipe (gpm)	P _s Density (kg/L)	C _v Volume (%)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	Velocity (ft/s)	Flow Rate (gpm)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe (gpm)	Design Pressure (400 lb/ft ²)	Flow Rate at a Pipe (gpm)	P _s Density (kg/L)	C _v Volume (%)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	Velocity (ft/s)	Flow Rate (gpm)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe (gpm)	Design Pressure (400 lb/ft ²)	Flow Rate at a Pipe (gpm)	P _s Density (kg/L)	C _v Volume (%)	d _p Median Dia. (µm)	P _m Bulk Density (kg/L)	Velocity (ft/s)	Flow Rate (gpm)	Total Pressure Drop (lb/ft ²)	Flow Rate at a Pipe (gpm)	Design Pressure (400 lb/ft ²)	Flow Rate at a Pipe (gpm)										
1	AN-101	AN-106	80-81	12/00	12/00	442	676	0	1.10	2.50	0.00	40	1.10	1.10	0.2	6	0	605	389	0.1	2	0	0	614	390	1.20	3.00	0.00	100	1.20	0.3	7	0	578	372	2	0	587	373	New = new or like new											
2	AP-107	83-86	1	7/03	7/03	400	2774	12	1.10	2.50	0.00	40	1.10	1.10	0.2	6	6	348	273	0.1	2	6	6	350	262	1.20	3.00	0.00	100	1.20	0.3	7	7	324	248	2	6	340	261	pipe line with maximum expected											
3	BNFL	83-86	7	2/11	12/19	400	0	6.413	4.0	1.10	2.50	0.00	40	1.10	2.6	60	49	57	233	197	2.8	65	51	62	240	199	1.20	3.00	0.67	100	1.21	3.4	78	77	92	220	187	pipe roughness of 2													
4	AN-102	83-86	1	3/10	3/10	828	3184	12	1.10	2.50	0.06	40	1.10	1.9	43	16	22	303	221	2.1	47	16	24	312	223	1.20	3.00	0.08	100	1.20	2.4	56	25	36	288	211	pipe (0.002 inches).														
5	BNFL	83-86	5	12/08	4/16	408	6823	40	1.10	2.50	0.82	40	1.10	2.8	64	58	72	218	177	3.1	70	60	60	226	179	1.20	3.00	1.00	100	1.22	3.0	84	94	122	208	168	EOI = pipeline at end of life (50 years for carbon steel line, 40 years for stainless steel line) with pipe roughness due to corrosion estimated at 150 mils for carbon steel and 10 mils for stainless steel.														
6	AN-103	80-81	1	4/12	4/12	253	421	0	1.10	2.50	0.23	40	1.10	2.3	53	3	5	764	498	2.5	58	3	6	774	500	1.20	3.00	0.31	100	1.21	3.0	70	5	9	727	476															
7	AN-104	80-81	3	6/05	2/16	376	6823	40	1.10	2.50	0.82	40	1.10	2.8	64	58	72	218	177	3.1	70	60	79	226	180	1.20	3.00	1.10	100	1.22	3.0	84	94	122	208	168															
8	AN-105	80-81	2	4/16	6/16	562	626	0	1.10	2.50	0.50	40	1.10	2.6	60	6	12	571	357	2.8	65	6	14	580	358	1.20	3.00	0.67	100	1.21	3.4	78	10	22	542	341															
9	AN-106	80-81	2	4/14	8/14	410	626	0	1.10	2.50	0.59	40	1.10	2.9	66	6	14	610	388	2.9	67	5	12	613	389	1.20	3.00	0.79	100	1.21	3.5	80	10	22	542	341															
10	AN-107	80-81	5	10/14	11/17	384	421	0	1.10	2.50	4.76	40	1.10	3.6	83	7	16	677	422	3.9	91	8	19	688	424	1.20	3.00	6.34	100	1.31	4.7	108	14	31	637	399															
11	AN-108	80-81	2	8/13	10/13	374	421	0	1.10	2.50	1.65	40	1.10	3.1	71	6	13	649	394	3.4	78	8	16	659	395	1.20	3.00	6.73	100	1.32	4.7	109	14	31	639	402															
12	AN-109	80-81	1	3/19	3/19	489	421	0	1.10	2.50	1.86	40	1.10	3.4	79	31	44	342	260	3.7	86	32	50	351	262	1.20	3.00	4.36	100	1.28	4.5	103	54	80	322	248															
13	AN-110	80-81	2	11/15	11/15	1,012	3,046	21	1.10	2.50	0.63	40	1.10	2.7	62	30	44	289	213	2.9	68	31	50	308	215	1.20	3.00	0.84	100	1.22	3.5	81	48	76	283	203															
14	AN-111	80-81	2	3/15	3/15	994	3,046	21	1.10	2.50	0.83	40	1.10	2.7	62	30	44	289	213	2.9	68	31	50	308	215	1.20	3.00	0.84	100	1.22	3.5	81	48	76	283	203															
15	AN-112	80-81	2	11/15	11/15	1,012	3,046	21	1.10	2.50	0.63	40	1.10	2.7	62	30	44	289	213	2.9	68	31	50	308	215	1.20	3.00	0.84	100	1.22	3.5	81	48	76	283	203															
16	AN-113	80-81	1	1/19	1/19	1,003	3,046	21	1.10	2.50	1.86	40	1.10	3.2	73	39	61	292	206	3.5	79	40	69	301	207	1.20	3.00	2.20	100	1.24	4.1	94	62	99	280	201															
17	AN-114	80-81	1	7/18	7/18	1,125	3,046	21	1.10	2.50	1.86	40	1.10	3.2	73	39	61	292	206	3.5	79	40	69	301	207	1.20	3.00	2.20	100	1.24	4.1	94	62	99	280	201															
18	AN-115	80-81	1	10/10	10/10	562	1,560	5	1.10	2.50	4.24	40	1.10	3.5	82	21	36	412	289	3.9	89	22	41	422	291	1.20	3.00	5.65	100	1.30	4.6	107	39	67	378	273															
19	AN-116	80-81	1	3/10	3/10	329	626	0	1.10	2.50	0.24	40	1.10	2.3	53	4	7	639	426	2.5	58	4	8	649	426	1.20	3.00	0.32	100	1.23	3.9	70	7	13	608	406															
20	AN-117	80-81	3	7/10	7/10	462	7,028	40	1.10	2.50	1.26	40	1.10	3.0	69	65	82	213	173	3.3	75	67	92	221	175	1.20	3.00	1.88	100	1.23	3.9	90	108	141	201	164															
21	AN-118	80-81	6	11/04	1/06	1,242	3,398	21	1.10	2.50	0.00	40	1.10	0.2	6	10	10	280	198	0.1	2	10	10	288	199	1.20	3.00	0.00	100	1.20	0.3	7	11	12	265	189															
22	AN-119	80-81	7	12/18	3/19	442	7,028	40	1.10	2.50	4.83	40	1.10	3.6	83	88	115	208	172	3.9	91	93	131	216	172	1.20	3.00	6.44	100	1.32	4.7	109	154	207	195	159															
23	AN-120	80-81	1	7/11	7/11	557	7,028	40	1.10	2.50	0.34	40	1.10	2.4	56	51	63	213	171	2.7	62	52	69	221	173	1.20	3.00	0.46	100	1.21	3.2	74	80	104	201	163															
24	AN-121	80-81	2	4/08	12/16	419	5,370	28	1.10	2.50	1.14	40	1.10	2.9	68	48	61	246	197	3.2	74	50	68	254	199	1.20	3.00	1.52	100	1.23	3.8	89	79	106	232	187															
25	AN-122	80-81	4	8/19	10/19	1,012	5,780	19	1.10	2.50	0.23	40	1.10	2.3	53	35	43	248	198	2.5	58	36	47	255	200	1.20	3.00	0.30	100	1.21	3.0	69	55	70	234	188															
26	AN-123	80-81	1	1/13	1/13	400	737	421	1.10	2.50	0.00	40	1.10	0.2	6	0	0	580	339	0.1	2	0	0	580	339	1.20	3.00	0.00	100	1.20	0.3	7	0	552	324																
27	AN-124	80-81	3	1/13	10/17	314	5,370	28	1.10	2.50	0.28	40	1.10	2.4	55	36	44	250	205	2.6	60	37	48	258	205	1.20	3.00	0.37	100	1.21	3.1	72	57	71	237	193															
28	AN-125	80-81	1	9/15	9/15	400	616	421	1.10	2.50	0.12	40	1.10	2.1	48	3	9	623	365	1.0	20	6	14	584	348	1.20	3.00	0.17	100	1.20	2.7	63	6	14	584	348															
29	AN-126	80-81	4	3/15	3/15	302	5,370	28	1.10	2.50	0.10	40	1.10	2.1	48	3	9	623	365	1.0	20	6	14	584	348	1.20	3.00	0.17	100	1.20	2.7	63	6	14	584	348															
30	AN-127	80-81	1	7/11	7/11	557	7,028	40	1.10	2.50	0.34	40	1.10	2.4	56	51	63	213	171	2.7	62	52	69	221	173	1.20	3.00	0.46	100	1.21	3.2	74	80	104	201	163															
31	AN-128	80-81	2	4/08	12/16	419	5,370	28	1.10	2.50	1.14	40	1.10	2.9	68	48	61	246	197	3.2	74	50	68	254	199	1.20	3.00	1.52	100	1.23	3.8	89	79	106	232	187															
32	AN-129	80-81	9	7/18	12/18	400	431	5,370	1.10	2.50	0.23	40	1.10	2.3	53	35	43	248	198	2.5	58	36	47	255	200	1.20	3.00	0.30	100	1.21	3.0	69	55	70	234	188															
33	AN-130	80-81	1	1/13	1/13	400	737	421	1.10	2.50	0.00	40	1.10	0.2	6	0	0	580	339	0.1	2	0	0	580	339	1.20	3.00	0.00	100	1.20	0.3	7	0	552	324																
34																																																			

Case No.	CASE 3S6 Transfer List - Tank-to-Tank										Critical velocity by Orskov and Turian (1980) empirical correlation ($\beta_w = 2$). Pressure drop by Weisp et al. (1977) method with lognormal particle size distribution.										Temperature = 60 °C										Notes													
	A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U	V	W	X	Y	Z	AA	AB	AC	AD		AE	AF	AG	AH	AI	AJ	AK	AL	AM	AN	AO	AP	AQ
Transfer	From Tank	To Tank	Tank Farm Bulk (Yr)	Transfer Dates (M/Yr)	3-in. Schedule 40 Pipe (3.069 in. inside dia.)	Carbon Steel Length (ft)	Stainless Steel Length (ft)	Total Change in Elevation (ft)	Group 1 Properties			Group 2 Properties			Temperature = 10 °C			Temperature = 60 °C			Temperature = 130 °C																							
									Liquid	Solids	P _m Bulk Density (kg/L)	P _m Bulk Density (kg/L)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)	Flow Rate at a Pipe Design Pressure of 400 lb/ft ² (gpm)																							
1	AN-101	AN-106	80-81	1	12/00	12/00	0	442	626	1.21	2.50	15	200	1.40	4.8	111	24	48	534	347	400	1.92	5.0	115	44	76	412	292	5.4	124	52	101	452	301	New = new or like new pipeline with maximum expected pipe roughness of 2 mils (0.002 inches).									
2	AP-103	83-86	1	7/03	7/03	400	306	2,774	12	1.21	2.50	15	200	1.40	4.8	111	75	107	307	242	400	1.92	5.0	115	138	176	223	196	5.4	124	160	223	257	202										
3	BNFL	BNFL	BNFL	7	2/11	12/19	400	0	6,413	1.21	2.50	15	200	1.40	4.8	111	166	207	203	173	400	1.92	5.0	115	143	188	228	189	5.4	124	165	238	252	193										
4	AN-102	AP-107	83-86	1	3/10	3/10	400	828	3,184	1.21	2.50	15	200	1.40	4.8	111	96	155	267	197	400	1.92	5.0	115	300	343	147	5.4	124	345	151	151	151											
5	AN-103	AN-102	80-81	1	4/12	4/12	0	253	421	1.21	2.50	15	200	1.40	4.8	111	15	29	675	445	400	1.92	5.0	115	28	47	526	374	5.4	124	33	62	570	368	EOL = pipeline at end of life (50 years for carbon steel line, 40 years for stainless steel line) with pipe roughness due to corrosion estimated at 150 mils for carbon steel and 10 mils for stainless steel.									
6	AN-104	AN-101	80-81	3	6/05	2/16	400	562	626	1.21	2.50	15	200	1.40	4.8	111	184	245	191	156	400	1.92	5.0	115	49	88	390	269	5.4	124	58	118	428	277										
7	AN-103	80-81	2	4/16	6/16	400	439	626	0	1.21	2.50	15	200	1.40	4.8	111	26	56	505	321	400	1.92	5.0	115	44	76	413	292	5.4	124	52	100	452	301										
8	AN-103	80-81	2	4/14	8/14	400	410	626	0	1.21	2.50	15	200	1.40	4.8	111	23	46	542	355	400	1.92	5.0	115	43	73	419	289	5.4	124	50	96	459	308										
9	AN-105	80-81	5	10/14	1/17	400	384	421	0	1.21	2.50	15	200	1.40	4.8	111	18	38	616	389	400	1.92	5.0	115	33	60	479	327	5.4	124	39	80	521	337										
10	AN-107	80-81	2	8/13	10/13	400	374	421	0	1.21	2.50	15	200	1.40	4.8	111	18	38	620	392	400	1.92	5.0	115	33	59	482	330	5.4	124	39	79	524	340										
11	AN-107	80-81	1	3/19	3/19	400	489	421	0	1.21	2.50	15	200	1.40	4.8	111	25	63	598	359	400	1.92	5.0	115	38	71	449	300	5.4	124	44	95	490	308										
12	AN-101	83-86	4	10/13	2/17	400	419	2,636	12	1.21	2.50	15	200	1.40	4.8	111	75	111	308	237	400	1.92	5.0	115	137	181	233	193	5.4	124	158	230	258	197										
13	AP-102	83-86	1	8/13	8/13	400	431	2,636	12	1.21	2.50	15	200	1.40	4.8	111	73	104	314	248	400	1.92	5.0	115	138	176	223	200	5.4	124	159	232	258	196										
14	AP-106	83-86	2	8/16	12/16	400	430	2,636	12	1.21	2.50	15	200	1.40	4.8	111	75	112	307	236	400	1.92	5.0	115	138	182	233	192	5.4	124	159	232	258	196										
15	AP-108	83-86	1	7/18	7/18	400	443	2,636	12	1.21	2.50	15	200	1.40	4.8	111	75	113	307	235	400	1.92	5.0	115	138	183	232	191	5.4	124	160	234	257	195										
16	AW-101	78-80	2	3/15	3/15	400	994	3,046	21	1.21	2.50	15	200	1.40	4.8	111	102	168	264	189	400	1.92	5.0	115	186	268	198	153	5.4	124	213	218	146	171										
17	AW-103	78-80	2	1/15	1/15	400	1,013	3,046	21	1.21	2.50	15	200	1.40	4.8	111	102	168	264	189	400	1.92	5.0	115	186	268	198	153	5.4	124	214	217	145	145										
18	AW-104	78-80	1	1/19	1/19	400	1,125	3,046	21	1.21	2.50	15	200	1.40	4.8	111	105	177	280	184	400	1.92	5.0	115	191	191	195	148	5.4	124	220	220	213	213										
19	AW-105	78-80	1	7/18	7/18	400	1,125	3,046	21	1.21	2.50	15	200	1.40	4.8	111	105	177	280	184	400	1.92	5.0	115	191	191	195	148	5.4	124	220	220	213	213										
20	AZ-101	71	1	10/10	10/10	400	562	1,560	5	1.21	2.50	15	200	1.40	4.8	111	86	374	285	151	400	1.92	5.0	115	137	181	233	193	5.4	124	158	230	258	197										
21	BNFL	BNFL	BNFL	1	9/10	9/10	400	0	6,275	40	1.21	2.50	15	200	1.40	4.8	111	163	203	206	173	400	1.92	5.0	115	295	337	149	5.4	124	338	154	154	154										
22	AN-105	80-81	1	3/10	3/10	400	329	626	0	1.21	2.50	15	200	1.40	4.8	111	21	40	565	390	400	1.92	5.0	115	40	64	437	319	5.4	124	46	85	478	329										
23	BNFL	BNFL	BNFL	3	8/12	5/17	400	452	7,028	40	1.21	2.50	15	200	1.40	4.8	111	190	256	187	151	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
24	AW-102	78-80	6	11/04	1/06	400	1,242	3,389	21	1.21	2.50	15	200	1.40	4.8	111	115	194	246	174	400	1.92	5.0	115	265	327	159	5.4	124	305	169	169	169											
25	BNFL	BNFL	BNFL	7	12/18	3/19	400	447	7,028	40	1.21	2.50	15	200	1.40	4.8	111	190	255	187	152	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
26	BNFL	BNFL	BNFL	1	7/11	7/11	400	557	7,028	40	1.21	2.50	15	200	1.40	4.8	111	192	263	185	149	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
27	AP-101	BNFL	BNFL	2	4/06	12/16	400	419	5,370	28	1.21	2.50	15	200	1.40	4.8	111	145	200	217	174	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
28	AP-102	BNFL	BNFL	9	7/18	12/18	400	431	5,370	28	1.21	2.50	15	200	1.40	4.8	111	145	200	217	174	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
29	AP-103	83-86	1	1/13	1/13	400	443	5,370	28	1.21	2.50	15	200	1.40	4.8	111	146	200	217	174	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169											
30	AP-104	BNFL	BNFL	3	1/13	10/17	400	314	5,370	28	1.21	2.50	15	200	1.40	4.8	111	143	192	219	179	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
31	AP-105	AP-104	83-86	1	9/15	9/15	400	616	421	1.21	2.50	15	200	1.40	4.8	111	143	192	219	179	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169											
32	AP-106	AN-102	80-81	1	8/12	8/12	400	1,446	3,798	21	1.21	2.50	15	200	1.40	4.8	111	161	244	201	153	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
33	BNFL	BNFL	BNFL	9	3/15	8/19	400	894	5,780	19	1.21	2.50	15	200	1.40	4.8	111	161	244	201	153	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
34	AW-103	AW-104	78-80	1	10/15	1/15	400	2,015	1,241	0	1.21	2.50	15	200	1.40	4.8	111	72	173	301	189	400	1.92	5.0	115	264	326	159	5.4	124	304	169	169	169										
35	AW-102	88-70	1	12/13	12/1																																							

1	2	CASE 3S6 Transfer List - Tank-to-Tank										Critical velocity by Orskov and Turian (1980) empirical correlation ($\beta_w = 1.2$). Pressure drop by Weisp et al. (1977) method with lognormal particle size distribution.										Temperature = 60 °C										Temperature = 80 °C										Notes
		Transfer		Tank		Transfer		Tank		Transfer		Tank		Transfer		Tank		Transfer		Tank		Transfer		Tank		Transfer		Tank		Transfer		Tank		Transfer		Tank		Transfer		Tank		
		From Tank	To Tank	Farm Bulk (Yr)	Dates (M/Yr)	Start	End	3-in. Schedule 40 Pipe (3.068 in. inside dia.)	Carbon Steel Length (ft)	Stainless Steel Length (ft)	Total Change in Elevation (ft)	Liquid	Solids	P _m Bulk Density (kg/L)	d _p Median Dia. (µm)	C _s Volume (%)	P _s Density (kg/L)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)	Flow Rate (gpm)	Velocity (ft/s)	Total Pressure Drop (lb/ft ²)				
6	AN-101	AN-106	80-81	1	12/00	12/00	442	626	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	91	556	357	572	356	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	New = new or like new	
7	AN-101	AP-103	83-86	1	7/03	7/03	400	1,914	2	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	92	107	409	341	342	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	pipe line with maximum expected roughness of 2 mils (0.002 inches).	
8	AN-101	AP-103	83-86	1	7/03	7/03	400	1,914	2	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	92	107	409	341	342	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	EOI = pipeline at end of life (50 years for carbon steel line, 40 years for stainless steel line) with pipe roughness due to corrosion estimated at 150 mils for carbon steel and 10 mils for stainless steel.	
9	AN-101	AP-107	83-86	1	6/05	6/05	400	423	1,914	2	0	1.20	3.00	10	400	1.38	1.44	6.3	144	82	107	409	341	342	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347		
10	AN-101	AP-107	83-86	1	6/05	6/05	400	423	1,914	2	0	1.20	3.00	10	400	1.38	1.44	6.3	144	82	107	409	341	342	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347		
11	AN-101	AP-107	83-86	1	6/05	6/05	400	423	1,914	2	0	1.20	3.00	10	400	1.38	1.44	6.3	144	82	107	409	341	342	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347		
12	AN-101	AP-107	83-86	1	6/05	6/05	400	423	1,914	2	0	1.20	3.00	10	400	1.38	1.44	6.3	144	82	107	409	341	342	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347		
13	AN-102	AP-107	83-86	1	3/10	3/10	400	405	2,324	10	0	1.20	3.00	10	400	1.38	1.44	6.3	144	81	114	467	320	319	319	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
14	AN-102	AP-107	83-86	1	3/10	3/10	400	405	2,324	10	0	1.20	3.00	10	400	1.38	1.44	6.3	144	81	114	467	320	319	319	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
15	AN-102	AP-107	83-86	1	3/10	3/10	400	405	2,324	10	0	1.20	3.00	10	400	1.38	1.44	6.3	144	81	114	467	320	319	319	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
16	AN-102	AP-107	83-86	1	3/10	3/10	400	405	2,324	10	0	1.20	3.00	10	400	1.38	1.44	6.3	144	81	114	467	320	319	319	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
17	AN-103	AN-102	80-81	1	4/12	4/12	253	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	27	55	702	459	461	461	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
18	AN-103	AN-102	80-81	1	4/12	4/12	253	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	27	55	702	459	461	461	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
19	AN-103	AN-102	80-81	1	4/12	4/12	253	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	27	55	702	459	461	461	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
20	AN-104	AN-101	80-81	3	6/05	2/16	562	626	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	47	107	527	327	327	327	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
21	AN-104	AN-102	80-81	2	4/16	6/16	439	626	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	42	91	557	357	357	357	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
22	AN-104	AN-102	80-81	2	4/14	6/14	410	626	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	41	87	565	356	356	356	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
23	AN-104	AN-102	80-81	2	8/13	10/13	384	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
24	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
25	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
26	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
27	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
28	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
29	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
30	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
31	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
32	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
33	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	
34	AN-104	AN-106	80-81	2	8/13	10/13	374	421	0	0	0	1.20	3.00	10	400	1.38	1.44	6.3	144	32	75	842	369	369	369	1.20	3.00	15	400	1.47	6.5	151	110	546	349	7.1	164	65	151	565	347	

APPENDIX E

**SENSITIVITY ANALYSIS OF THE TRANSFER ROUTE
FROM TANK 241-AN-107 TO BNFL INC.**

Calculations contained herein were produced in Mathcad 2000 Professional (Mathcad is a registered trademark of MathSoft, Inc. of Cambridge, Massachusetts).

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CONVERSIONS

ft

feet = 0.3048 meter (m)

in.

inch = 2.54 centimeters (cm)

lbf/in²

pounds per square inch = 0.06895 megapascals (MPa)

mils

0.001 in. = 0.0254 millimeter (mm)

gpm

0.0631 liter/second (L/s)

cP

centipose = 0.001 kilogram/meter-second (kg/m-s)

CH2MHILL Hanford Group, Inc.

EVALUATION ANALYSIS

Calc. No. RPP-LJJ-002Revision: 0Page No. 1 of 38

Client: Numatec Hanford Corporation
 Subject: Critical Velocity and Pressure Drop Study in Support of
Waste Tank Feed Transfer System
 Location: 200 Area - Hanford Site, Richland, Washington

WO/Job No. 110299 & 110300/BA10
 Date: 03/08/2000 By: L. J. Julyk
 Checked: 03/10/2000 By: T. C. Oton
 Revised: _____ By: _____

SENSITIVITY ANALYSIS - AN-107-to-BNFL Transfer Route

The AN-107-to-BNFL transfer route was identified as one of the longest transfer routes in the Case 3S6 Transfer List. Hence, this transfer route was chosen for a sensitivity analysis on the pipeline pressure drop for the range of waste properties being considered for waste feed delivery to BNFL. The assumed particle size distribution by volume is based on a Rosin-Rammler cumulative and probability density function (Shook and Roco 1991) curve fit to the particle size distribution measurements obtain from Tank 241-SY-101 waste samples (O'Rourke 1999). The Tank 241-SY-101 particle size distribution had a median particle size (d_{50}) of 184 μm (see Appendix F). The resulting curve-fit Rosin-Rammler cumulative and probability density function was then shifted to target median particle sizes for the sensitivity analysis.

The following waste and pipe parameter were considered in the sensitivity analysis:

Internal pipe diameter (nominal)

$$D_{\text{pipe}} = 3.068 \text{ in}$$

Pipe roughness estimates (Anantatmula and Divine 1999, new and end-of-life (EOL) pipe conditions indicated by dashed and solid lines, respectively in following plots)

$$\epsilon_{\text{new}} = 2 \text{ mil} \quad \text{new commercial pipe}$$

$$\epsilon_{\text{EOL_SS}} = 10 \text{ mil} \quad \text{for stainless steel pipe at end of 40-year service life}$$

$$\epsilon_{\text{EOL_CS}} = 150 \text{ mil} \quad \text{for carbon steel pipe at end of 50-year service life}$$

Waste parameters (Estey 2000)

$$\rho_L = 1.2 \text{ and } 1.46 \text{ kg/L} \quad \text{liquid "carrier" density}$$

$$\rho_s = 2.5 \text{ and } 3 \text{ kg/L} \quad \text{solids particle density}$$

$$T = 10 \text{ and } 60 \text{ }^\circ\text{C} \quad \text{temperature of waste feed}$$

$$d_p = 400, 200, 100, 40, \text{ and } 5 \text{ } \mu\text{m} \quad \text{median particle size } (d_{50}) \text{ of shifted particle size distribution}$$

$$C_s = 5, 10, \text{ and } 15\% \quad \text{volume percent concentration of solids.}$$

A pipeline design pressure of 400 lb/in^2 is assumed for reference (horizontal dashed line). Critical velocity and pressure drop estimates are based on Oroskar and Turian (1980) and Wasp et al. (1977) empirical correlations, respectively (see Appendix F). Results are given in Figures E-1 through E-5 for $d_p = 400, 200, 100, 40, \text{ and } 5 \text{ } \mu\text{m}$, respectively for the above waste and pipe parameters. In Figure E-6 the Oroskar and Turian (1980) critical velocity correlation is compared to a critical velocity prediction based on the Wasp "vehicle" concept (see Appendix F). Figures E-7 through 10 shows the pressure drop at the critical velocity versus the median particle size for selected waste transfer parameters and pipe roughness values.

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Wasp, E. J., J. P. Kenny, and R. L. Gandhi, 1977, *Solid-Liquid Flow -- Slurry Pipeline Transportation*, Trans. Tech. Publ., Rockport, MA.

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EVALUATION ANALYSIS

Calc. No. RPP-LJJ-002

Revision: 0

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Client: Numatec Hanford Corporation

WO/Job No. 110299 & 110300/BA10

Subject: Critical Velocity and Pressure Drop Study in Support of Waste Tank Feed Transfer System

Date: 03/08/2000

By: L. J. Julyk

Location: 200 Area - Hanford Site, Richland, Washington

Checked: 03/10/2000

By: T. C. Oter

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Figure E-1a. $d_p=400 \mu\text{m}$ Case - Cumulative and Probability Density Particle Size Distribution by Volume.

$d_{50} := 400 \mu\text{m}$

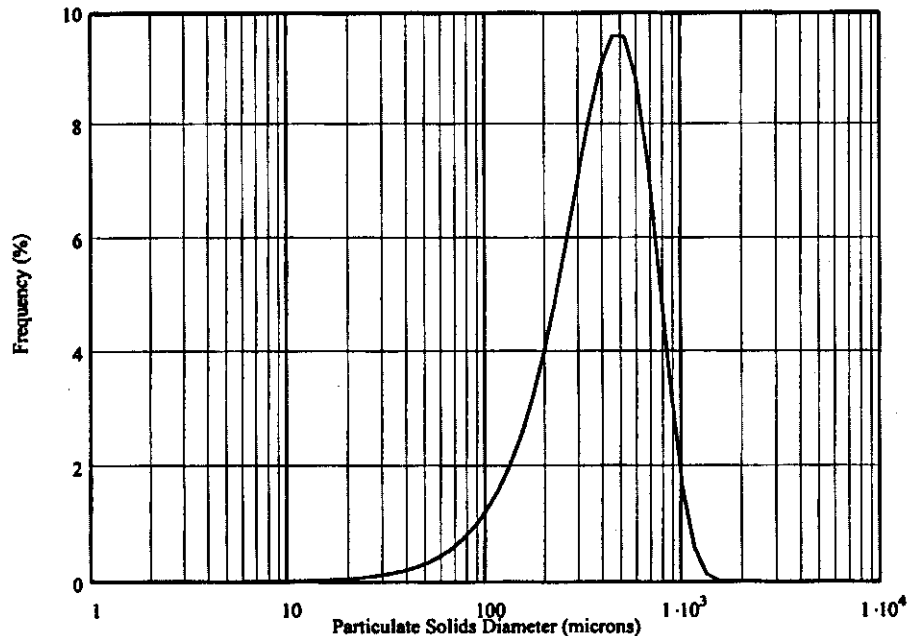
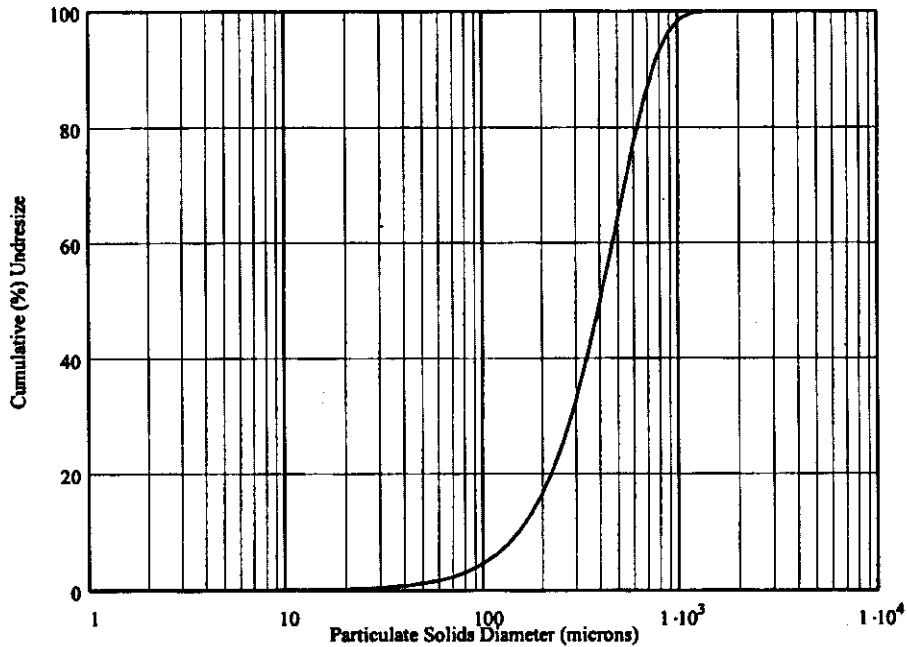
$(d_{pRR} \phi_{RR}) := \text{dist}_{RR}(d_{pR}, d_{50})$

$d_{pRR} := d_{pRR} \mu\text{m}$ reset units

$\phi_{RRcum} := \text{cum}(\phi_{RR})$

$D_{pRR} := D_{\beta}(d_{pRR}, \phi_{RRcum}, 50\%)$

$D_{pRR} = 373 \mu\text{m}$



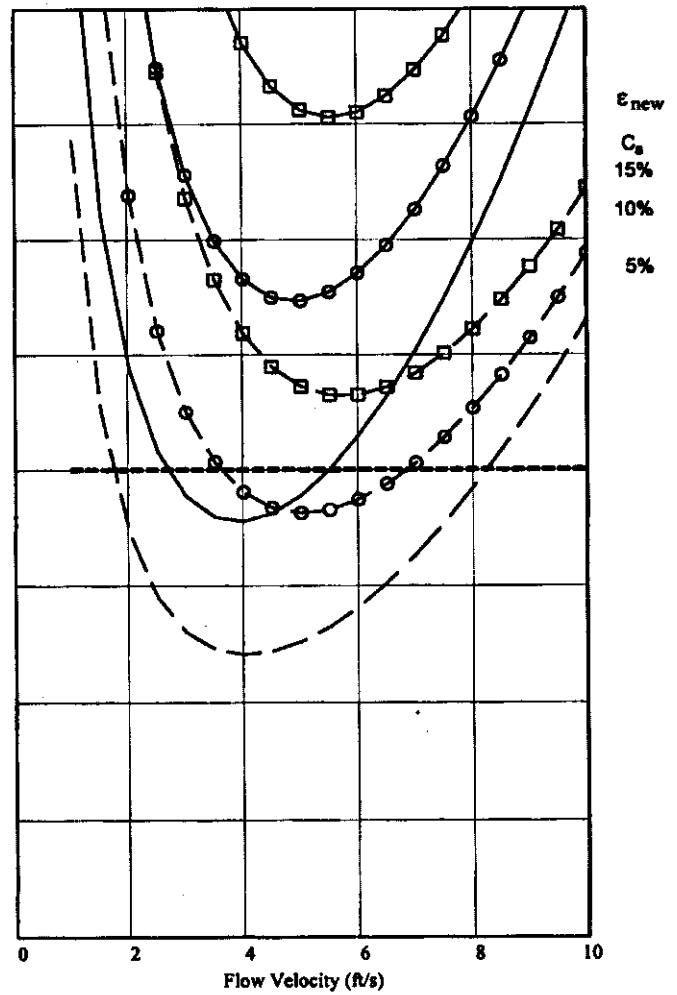
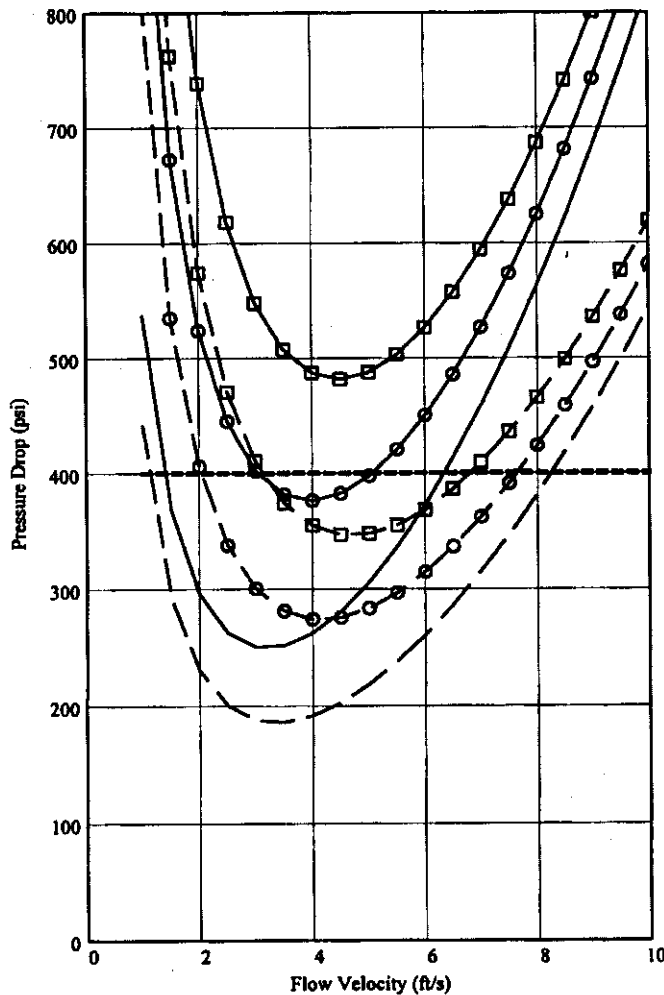
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Figure E-1b. Pressure Drop for AN-107 to BNFL - $d_p=400 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} := 1.2$	$L_{CS} := 557\text{-ft}$	$L_{SS} := 7028\text{-ft}$	$H := 40\text{-ft}$	$d_{50} = 400 \mu\text{m}$	$\epsilon_{\text{new}} = 2 \text{ mil}$
$\kappa_{cv} := 1.30$					$\epsilon_{\text{EOL}_{SS}} = 10 \text{ mil}$
$\kappa_f := 1$					$\epsilon_{\text{EOL}_{CS}} = 150 \text{ mil}$
$T := 10 \text{ }^\circ\text{C}$	$\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$	$\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$	$\rho_s := 3 \frac{\text{kg}}{\text{liter}}$	$T := 60 \text{ }^\circ\text{C}$	$\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5.7 \frac{\text{ft}}{\text{sec}}$				$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 6.3 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 6.3 \frac{\text{ft}}{\text{sec}}$				$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 6.8 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 6.5 \frac{\text{ft}}{\text{sec}}$				$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 7.1 \frac{\text{ft}}{\text{sec}}$	15%
					$\epsilon_{\text{EOL}} \quad C_s = 15\%, 10\%, 5\%$



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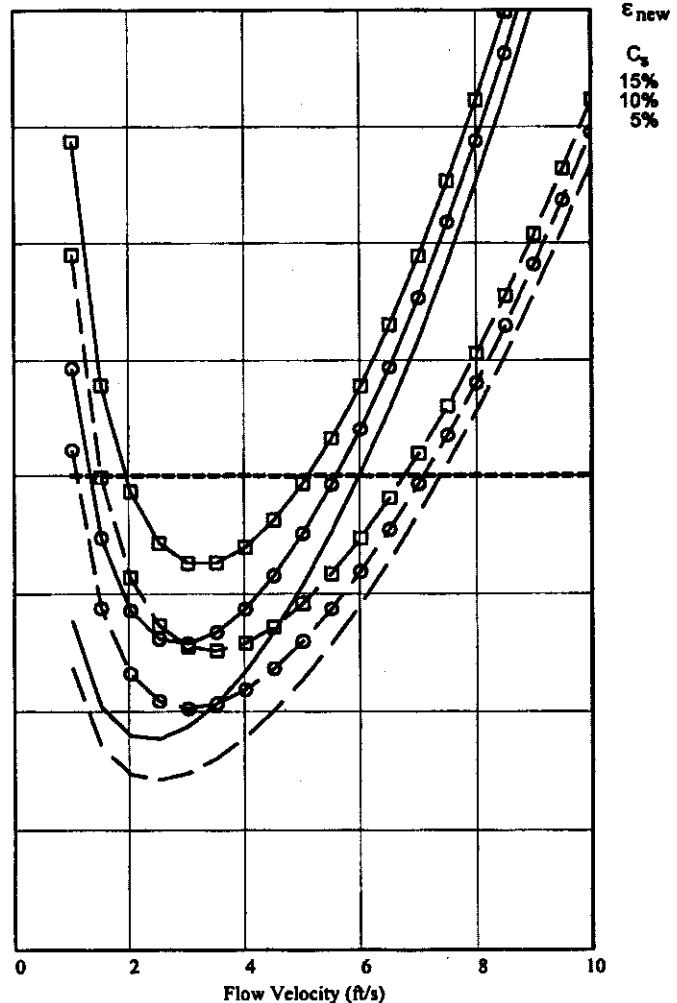
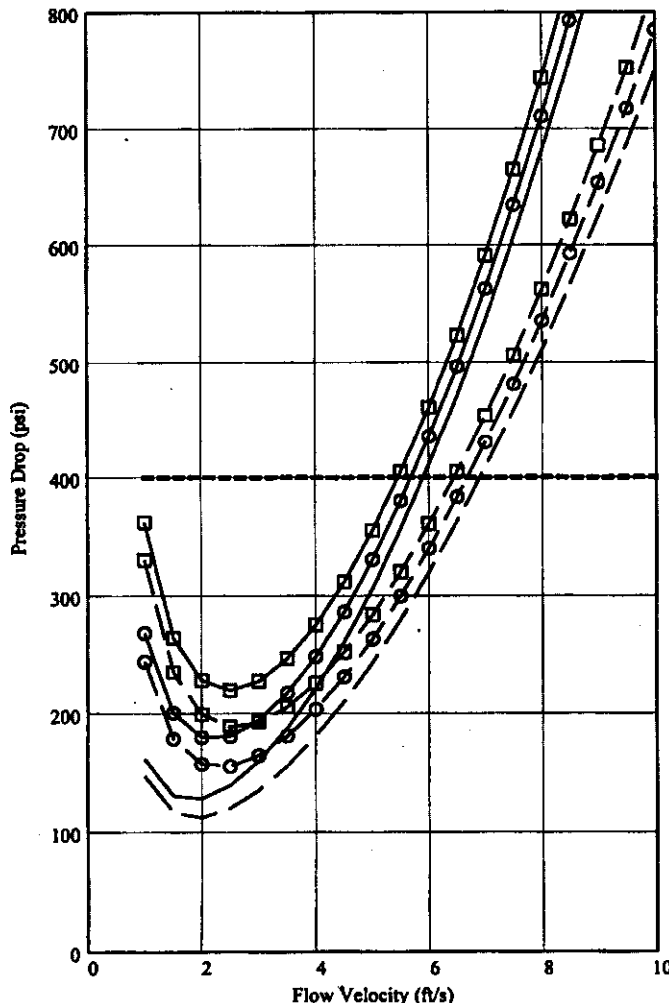
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Figure E-1c. Pressure Drop for AN-107 to BNFL - $d_p=400 \mu\text{m}$, $\rho_L=1.48 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 400 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.2 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.6 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.6 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.8 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.2 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{EOL} \quad C_s = 15\%, 10\%, 5\%$



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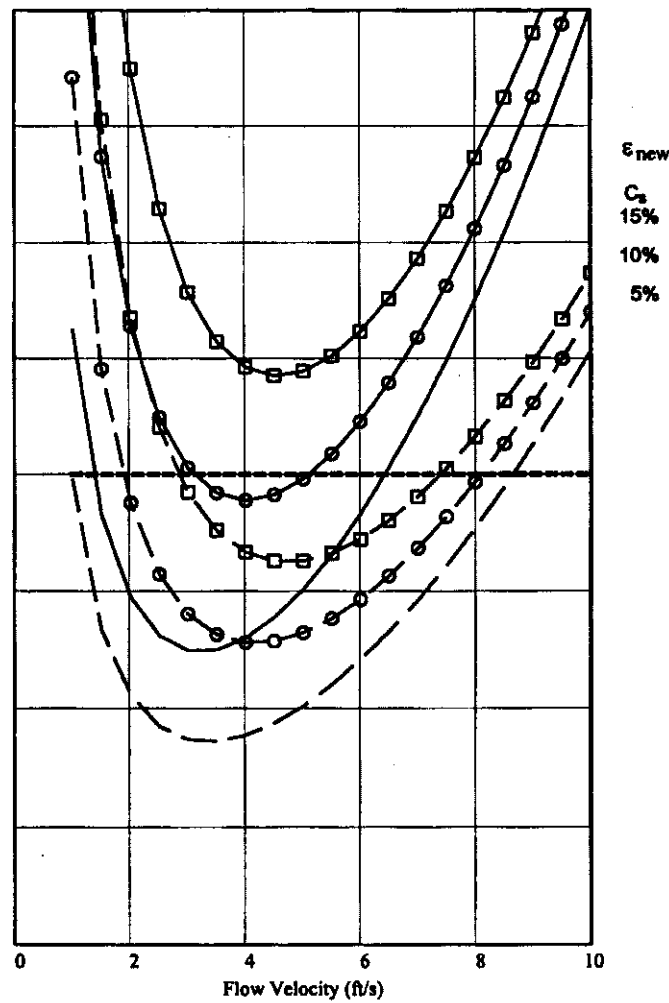
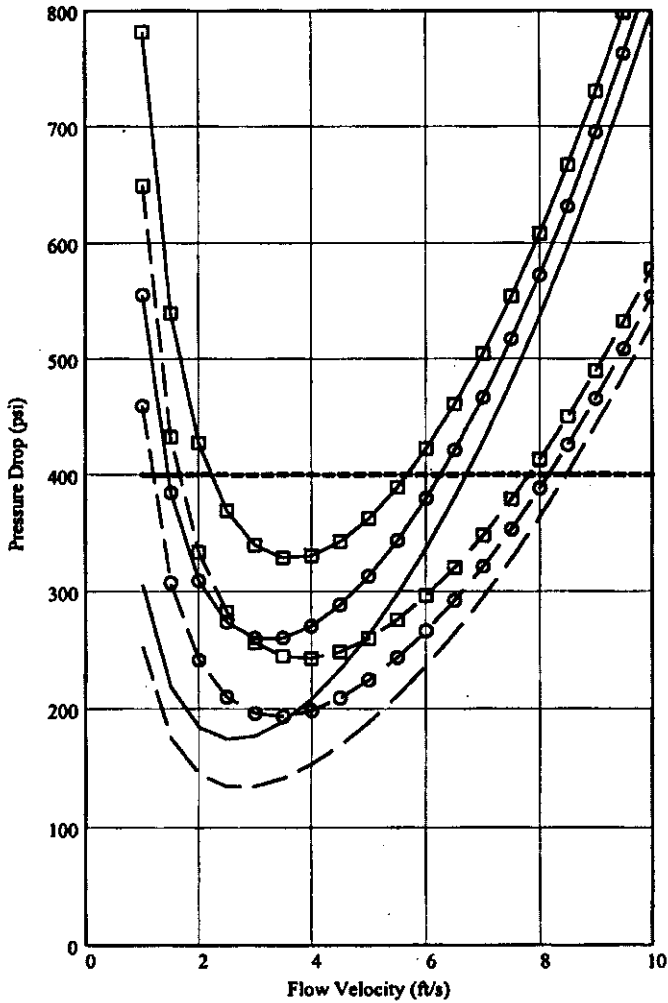
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Figure E-1d. Pressure Drop for AN-107 to BNFL - $d_p=400 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 400 \mu\text{m}$ $\epsilon_{\text{new}} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{\text{EOL}_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{\text{EOL}_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.8 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.2 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5.2 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.7 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5.5 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 6 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{\text{EOL}} \quad C_s = 15\%, 10\%, 5\%$



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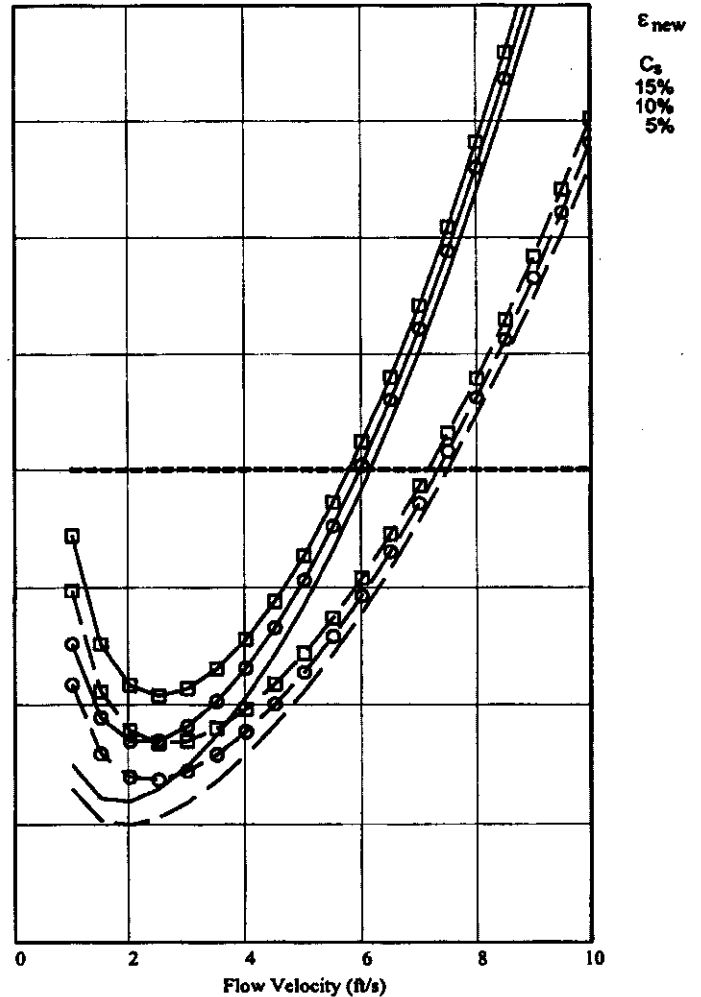
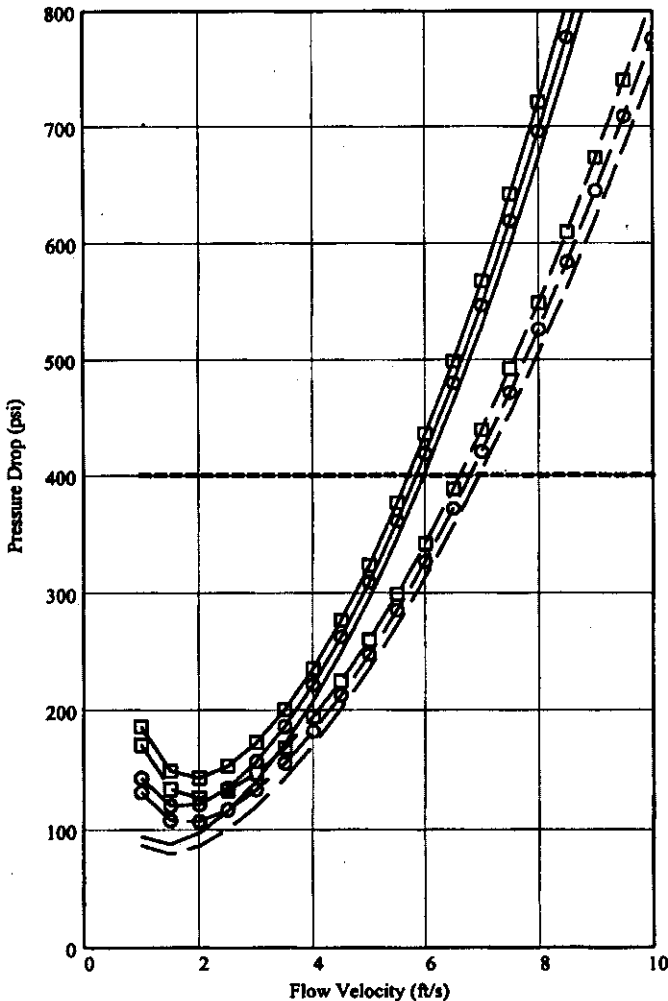
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Figure E-1e. Pressure Drop for AN-107 to BNFL - $d_p=400 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 400 \mu\text{m}$ $\epsilon_{\text{new}} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{\text{EOL}_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{\text{EOL}_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.4 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.7 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.7 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.9 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.2 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{\text{EOL}} \quad C_s = 15\%, 10\%, 5\%$



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By: L. J. Julyk

Waste Tank Feed Transfer System

Checked: 03/10/2000

By: T. C. Oten

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Figure E-2a. $d_p=200 \mu\text{m}$ Case - Cumulative and Probability Density Particle Size Distribution by Volume.

$d_{50} := 200 \cdot \mu\text{m}$

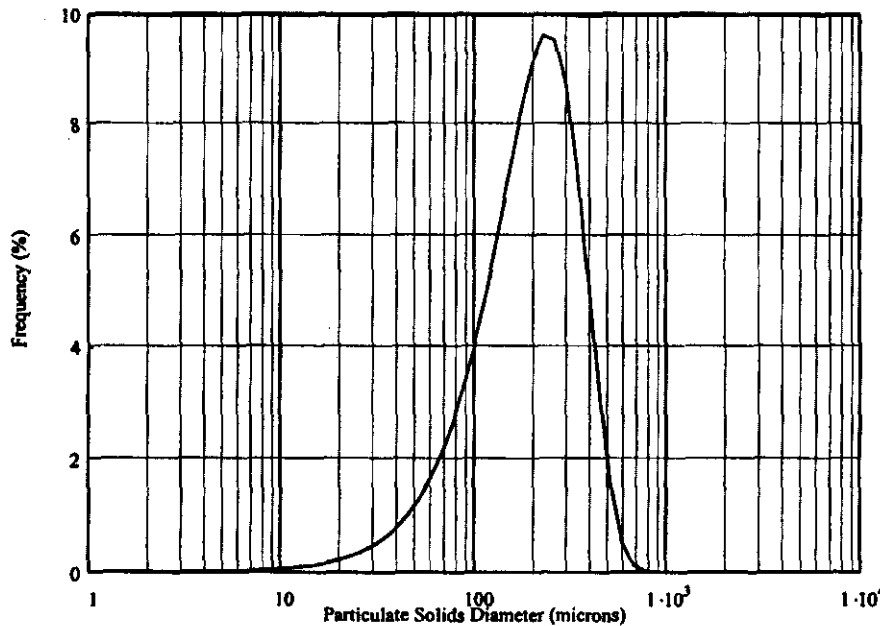
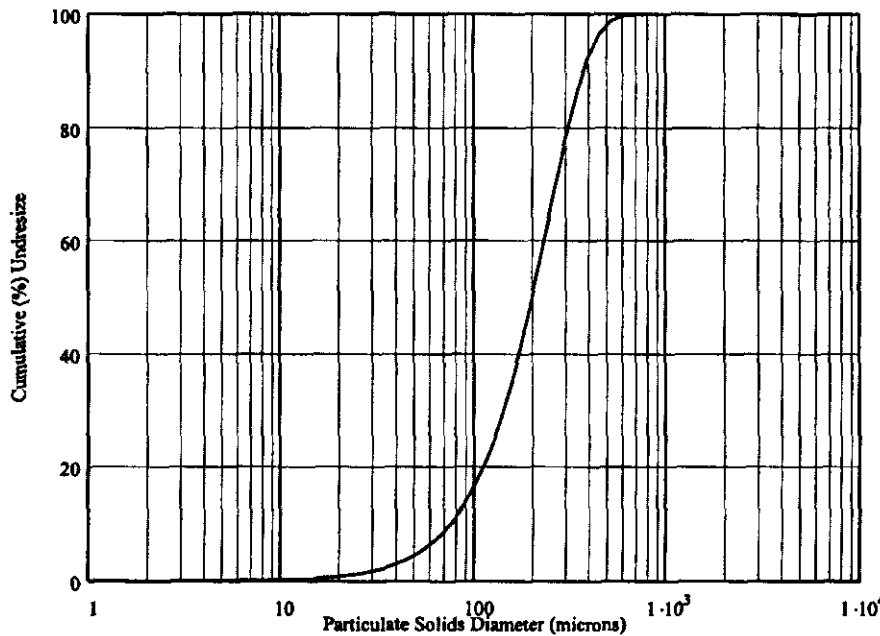
$(d_{pRR} \ \phi_{RR}) := \text{dist}_{RR}(d_{pR}, d_{50})$

$d_{pRR} := d_{pRR} \cdot \mu\text{m}$ reset units

$\phi_{RRcum} := \text{cum}(\phi_{RR})$

$D_{pRR} := D_{\beta}(d_{pRR}, \phi_{RRcum}, 50\%)$

$D_{pRR} = 187 \mu\text{m}$



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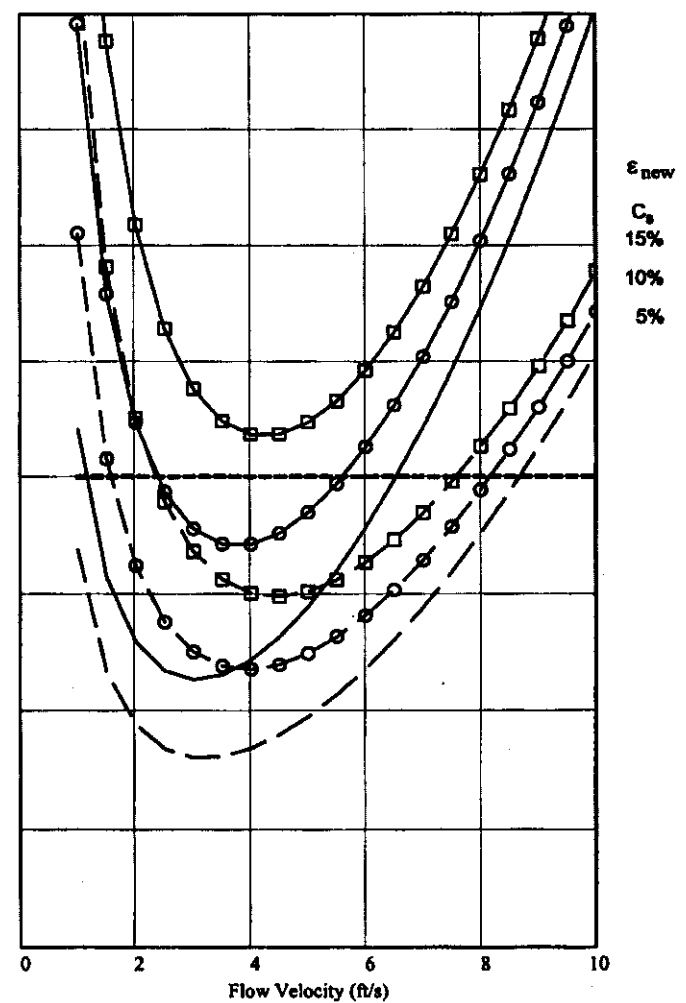
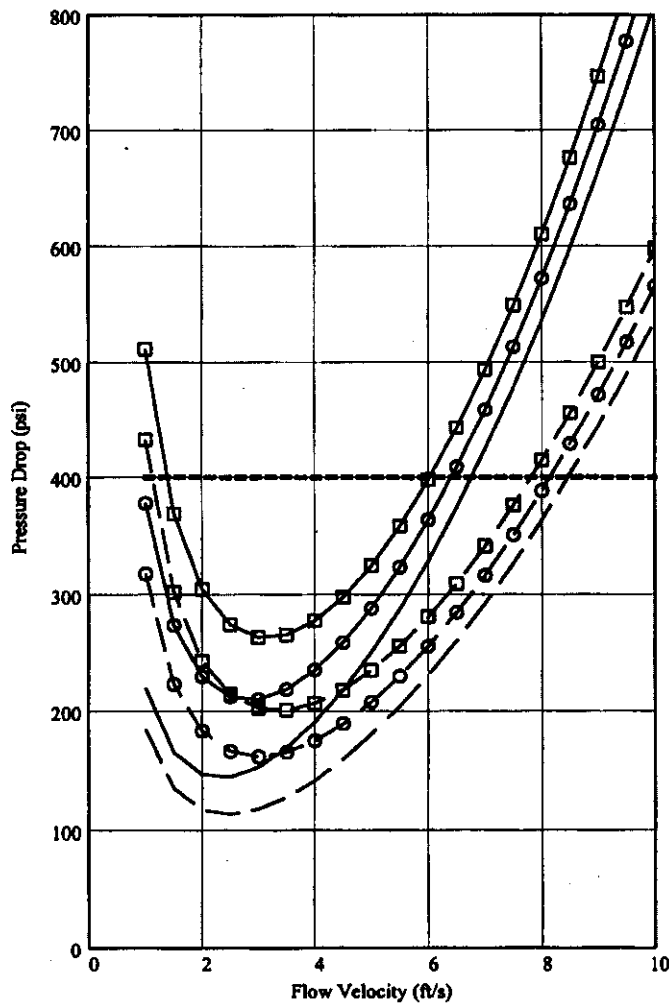
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Figure E-2b. Pressure Drop for AN-107 to BNFL - $d_p=200 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 200 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5.1 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.6 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5.6 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 6.1 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5.8 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 6.3 \frac{\text{ft}}{\text{sec}}$	15%

ϵ_{EOL} $C_s = 15\%, 10\%, 5\%$



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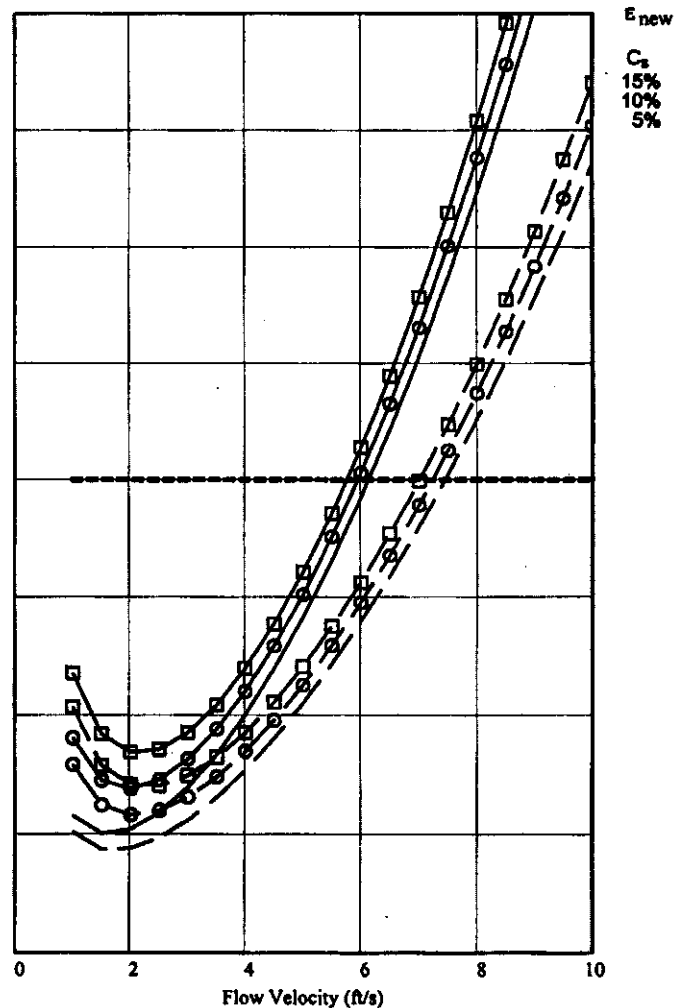
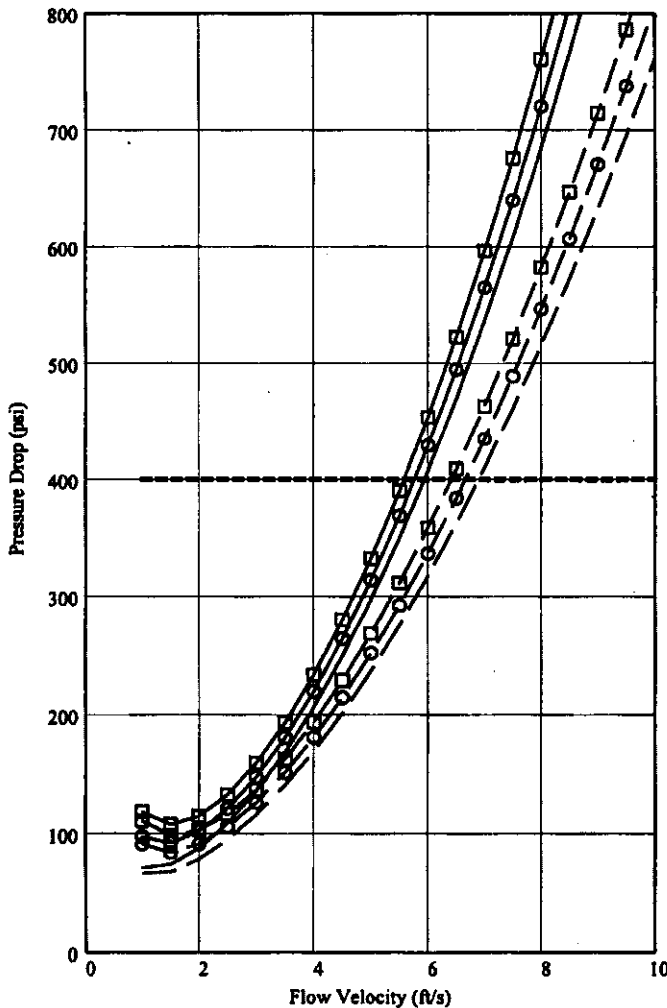
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Figure E-2c. Pressure Drop for AN-107 to BNFL - $d_p=200 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 200 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \cdot \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \cdot \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.8 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.1 \frac{\text{ft}}{\text{sec}}$ C_s
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.1 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.4 \frac{\text{ft}}{\text{sec}}$ 5%
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.3 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.6 \frac{\text{ft}}{\text{sec}}$ 10%
 15%

$\epsilon_{EOL} C_s = 15\%, 10\%, 5\%$



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 Subject: Critical Velocity and Pressure Drop Study in Support of Waste Tank Feed Transfer System
 Location: 200 Area - Hanford Site, Richland, Washington

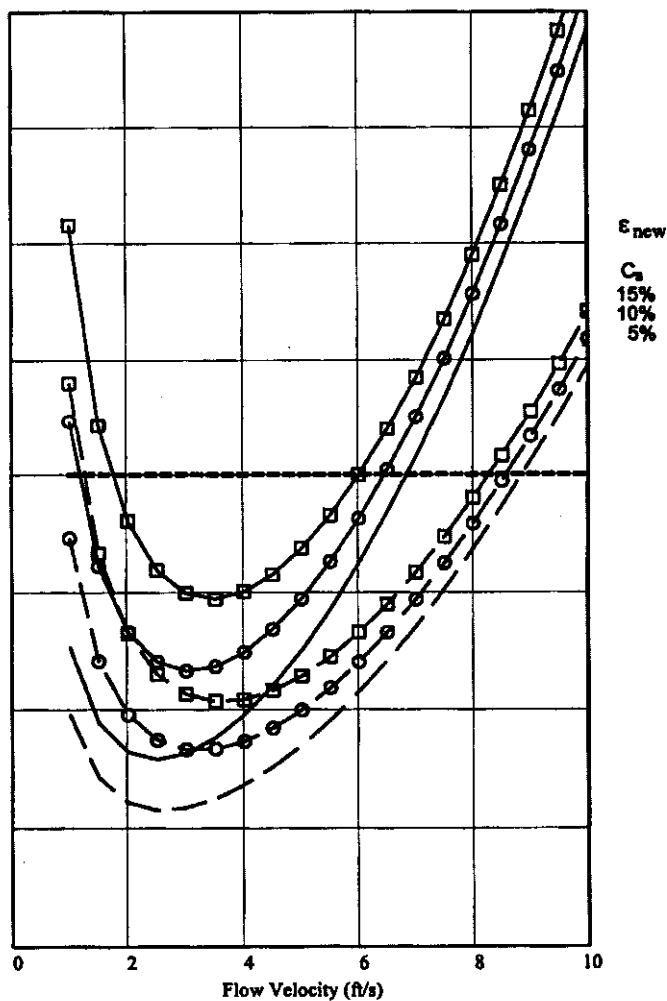
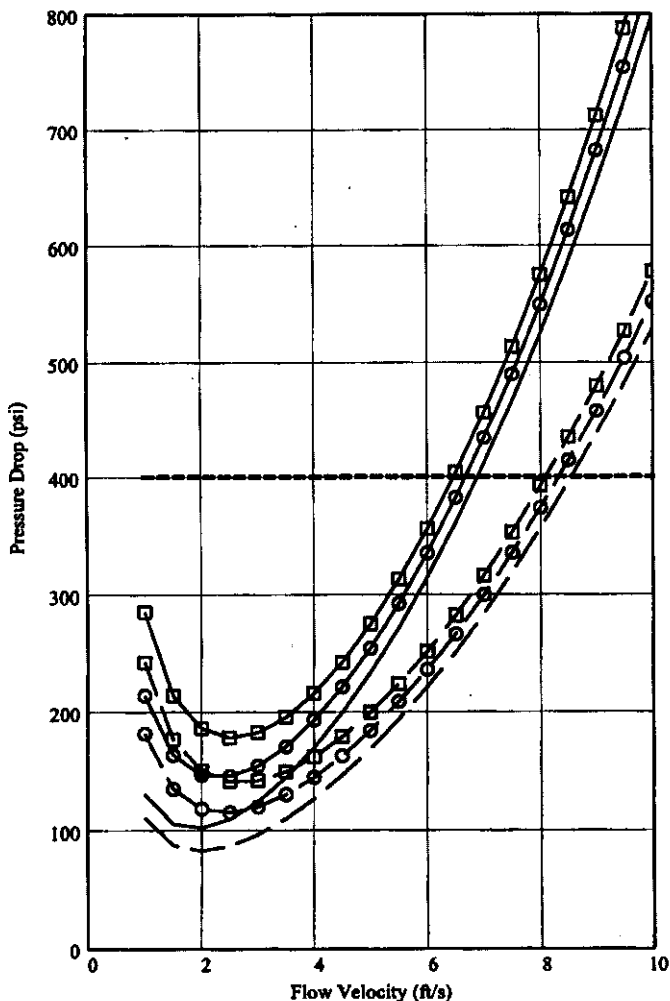
WO/Job No. 110299 & 110300/BA10
 Date: 03/08/2000 By: L. J. Julyk
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 Revised: _____ By: _____

Figure E-2d. Pressure Drop for AN-107 to BNFL - $d_p=200 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 200 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.3 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.7 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.7 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.1 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.9 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.3 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{EOL} C_s = 15\%, 10\%, 5\%$



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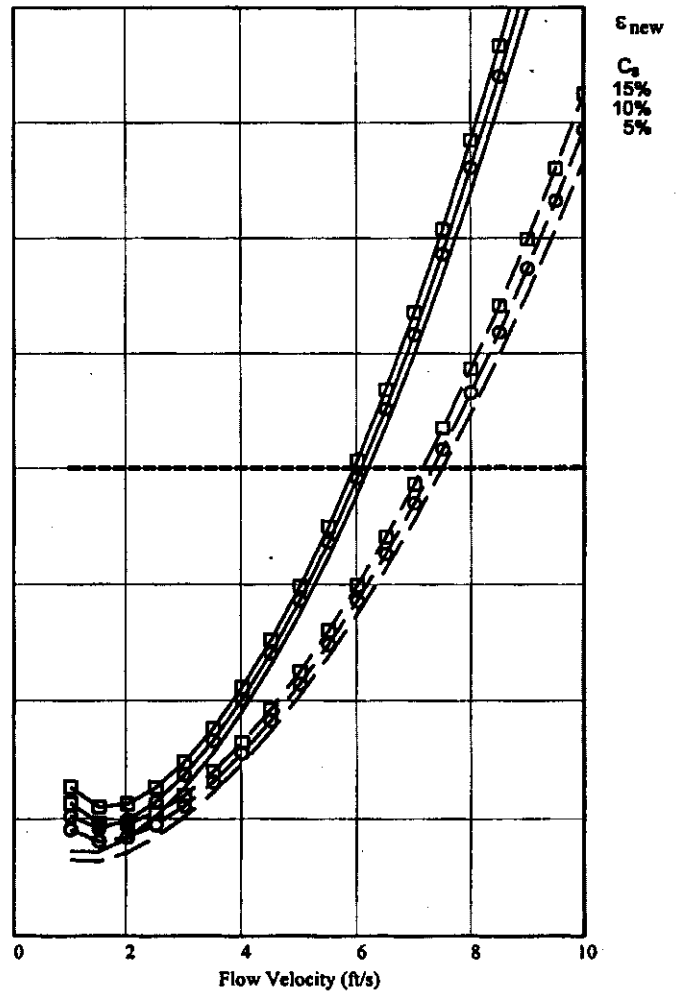
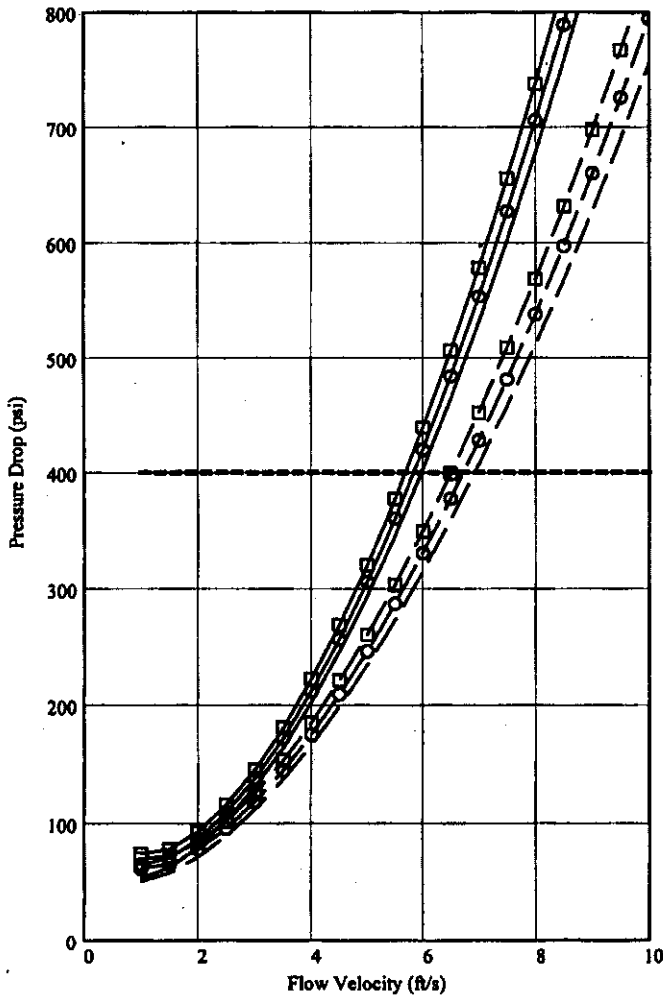
WO/Job No. 110299 & 110300/BA10
 Date: 03/08/2000 By: L. J. Julyk
 Checked: 03/10/2000 By: T. C. Oter
 Revised: _____ By: _____

Figure E-2e. Pressure Drop for AN-107 to BNFL - $d_p=200 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 200 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.1 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.3 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.3 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.6 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.5 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.7 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{EOL} C_s = 15\%, 10\%, 5\%$



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Figure E-3a. $d_p=100 \mu\text{m}$ Case - Cumulative and Probability Density Particle Size Distribution by Volume.

$d_{50} := 100 \cdot \mu\text{m}$

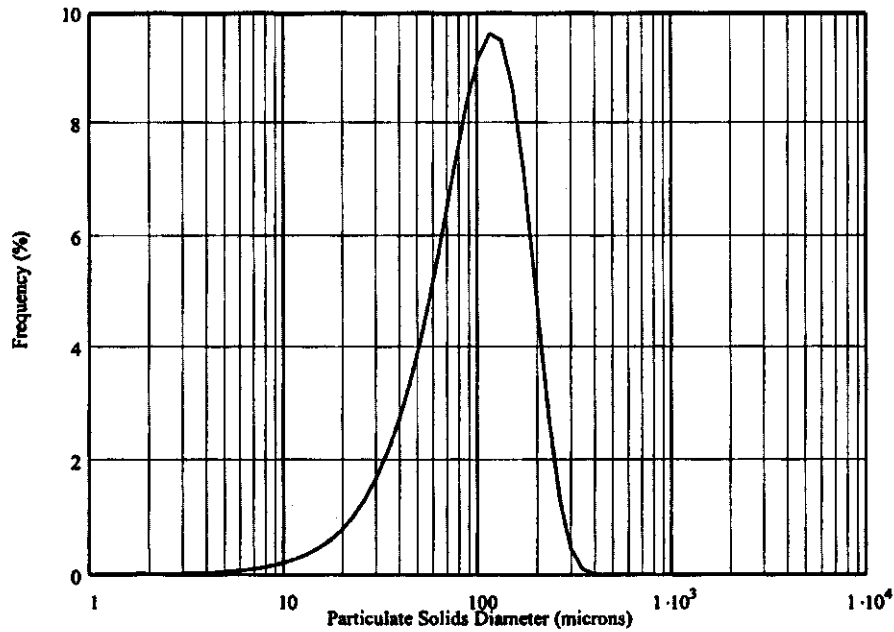
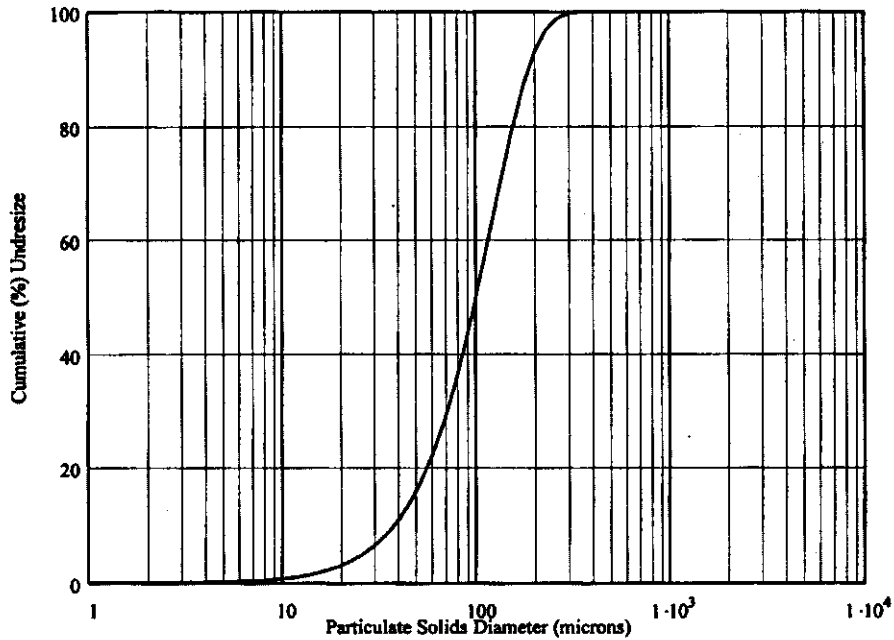
$(d_{pRR} \phi_{RR}) := \text{dist}_{RR}(d_{pR}, d_{50})$

$d_{pRR} := d_{pRR} \cdot \mu\text{m}$ reset units

$\phi_{RRcum} := \text{cum}(\phi_{RR})$

$D_{pRR} := D_{\beta}(d_{pRR}, \phi_{RRcum}, 50\%)$

$D_{pRR} = 93 \mu\text{m}$



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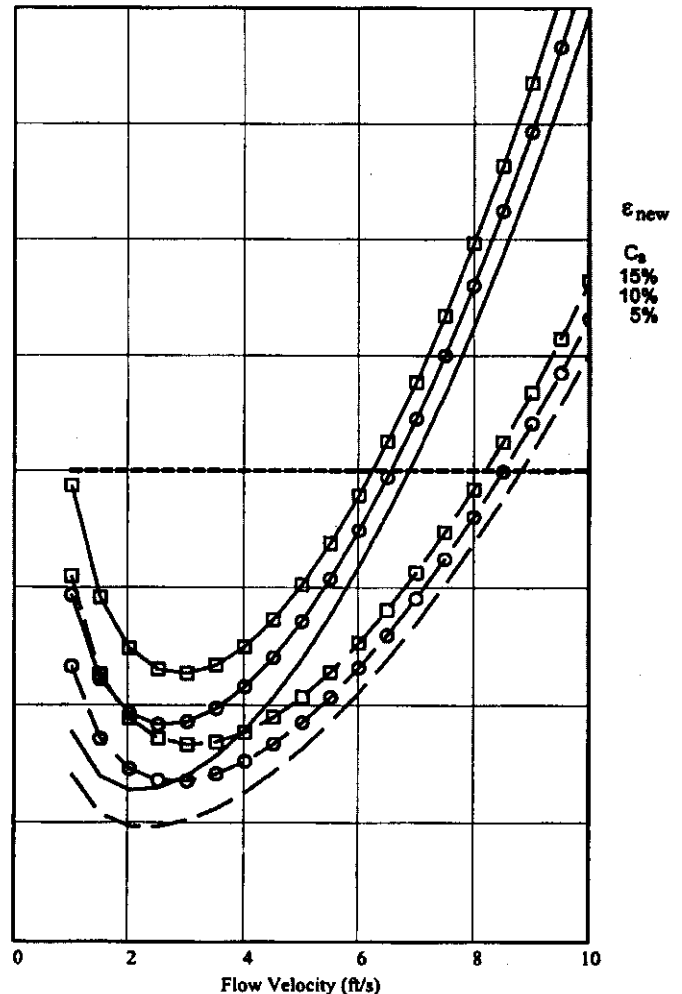
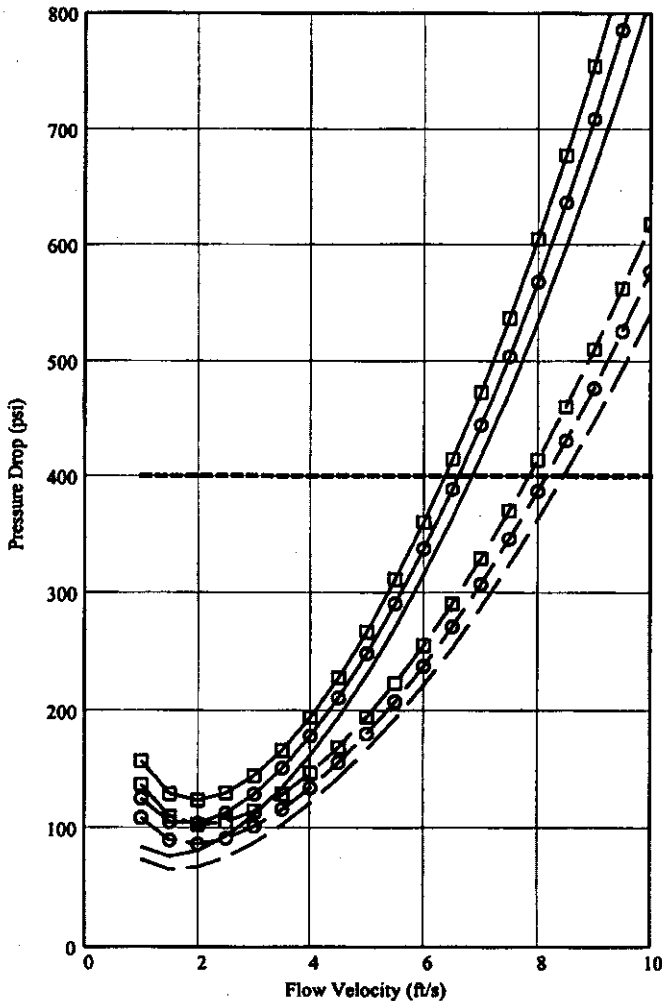
WO/Job No. 110299 & 110300/BA10
 Date: 03/08/2000 By: L. J. Julyk
 Checked: 03/10/2000 By: T. C. Oten
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Figure E-3b. Pressure Drop for AN-107 to BNFL - $d_p=100 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 100 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.6 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.4 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 5.2 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 5.7 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{EOL} \quad C_s = 15\%, 10\%, 5\%$



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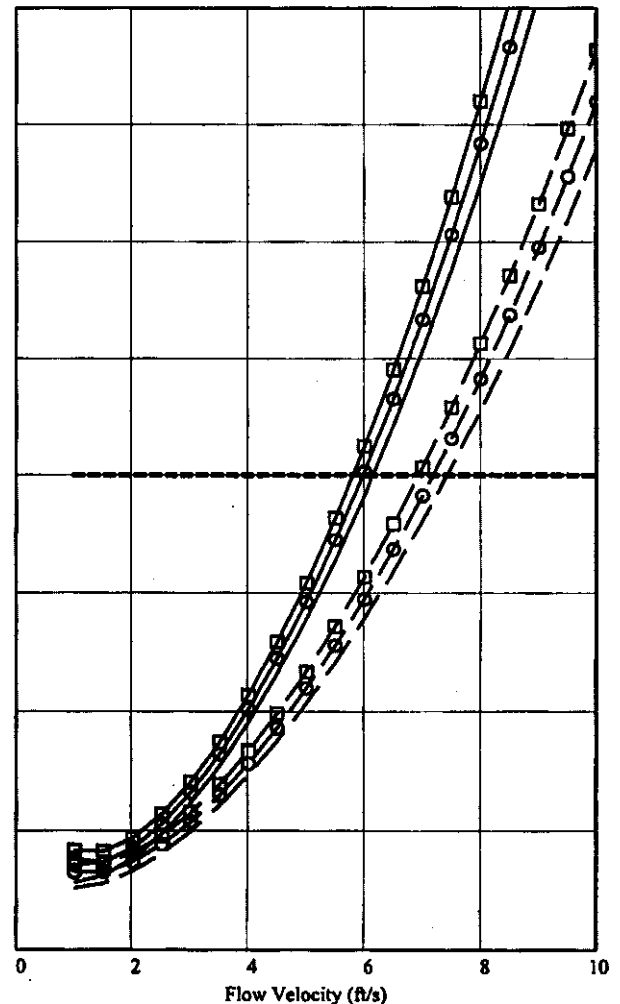
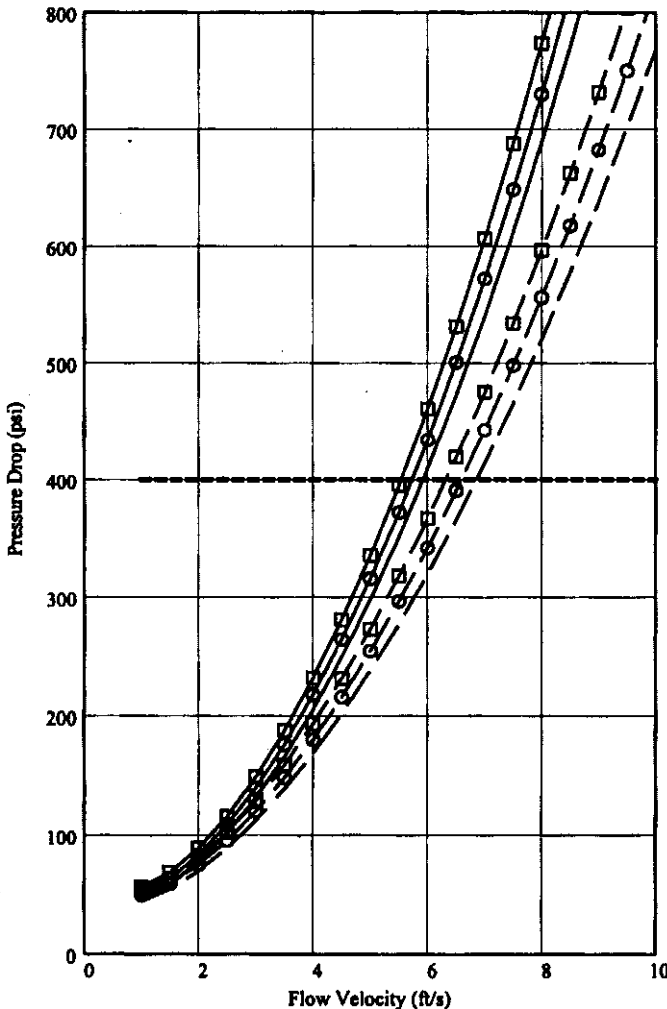
WO/Job No. 110299 & 110300/BA10
 Date: 03/08/2000 By: L. J. Julyk
 Checked: 03/10/2000 By: T. C. Oten
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Figure E-3c. Pressure Drop for AN-107 to BNFL - $d_p=100 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 100 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.4 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.6 \frac{\text{ft}}{\text{sec}}$ C_s
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.7 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4 \frac{\text{ft}}{\text{sec}}$ 5%
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.8 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.1 \frac{\text{ft}}{\text{sec}}$ 10%
 15%

ϵ_{EOL} $C_s = 15\%, 10\%, 5\%$ ϵ_{new}



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WO/Job No. 110299 & 110300/BA10

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By: L. J. Julyk

Checked: 03/10/2000

By: T. C. Oten

Revised:

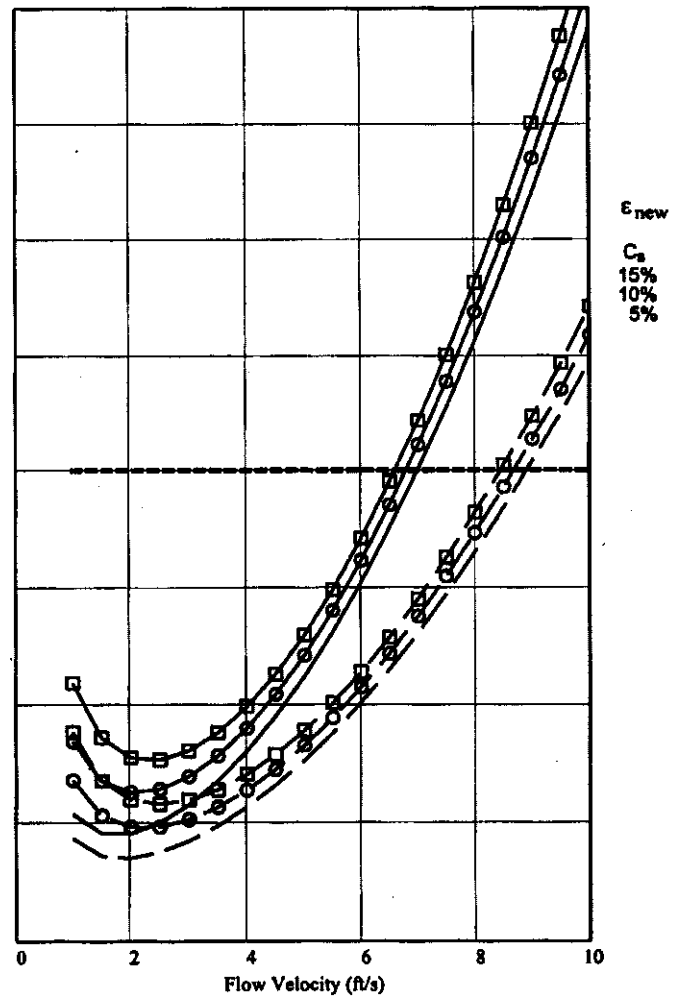
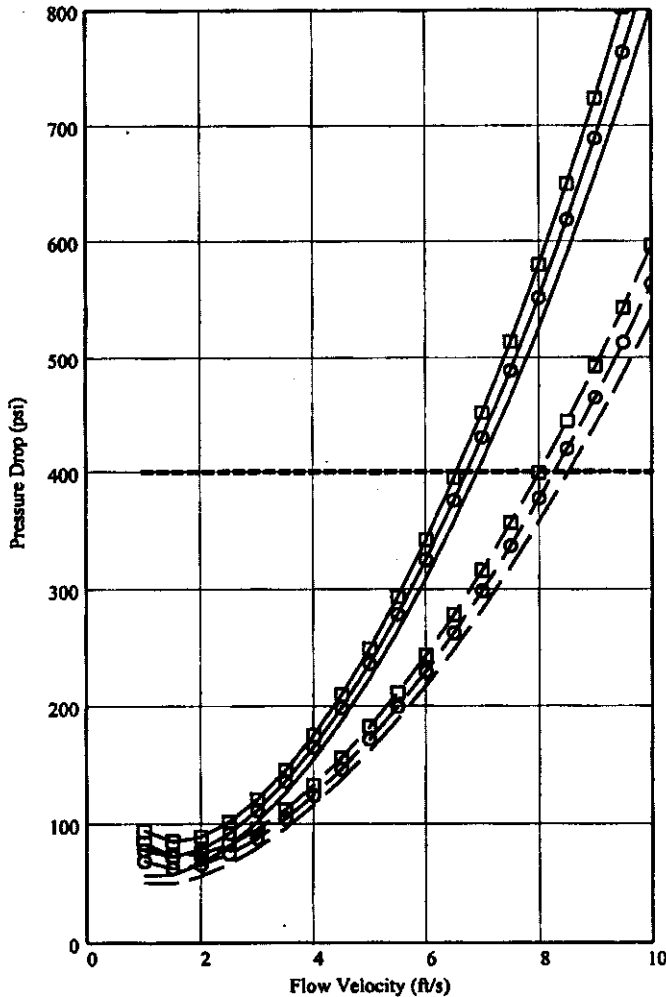
By:

Figure E-3d. Pressure Drop for AN-107 to BNFL - $d_p=100 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 100 \mu\text{m}$ $\epsilon_{\text{new}} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{\text{EOL}_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{\text{EOL}_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.8 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.2 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.2 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.5 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.3 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.7 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{\text{EOL}} \quad C_s = 15\%, 10\%, 5\%$



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By: L. J. Julyk

Checked: 03/10/2000

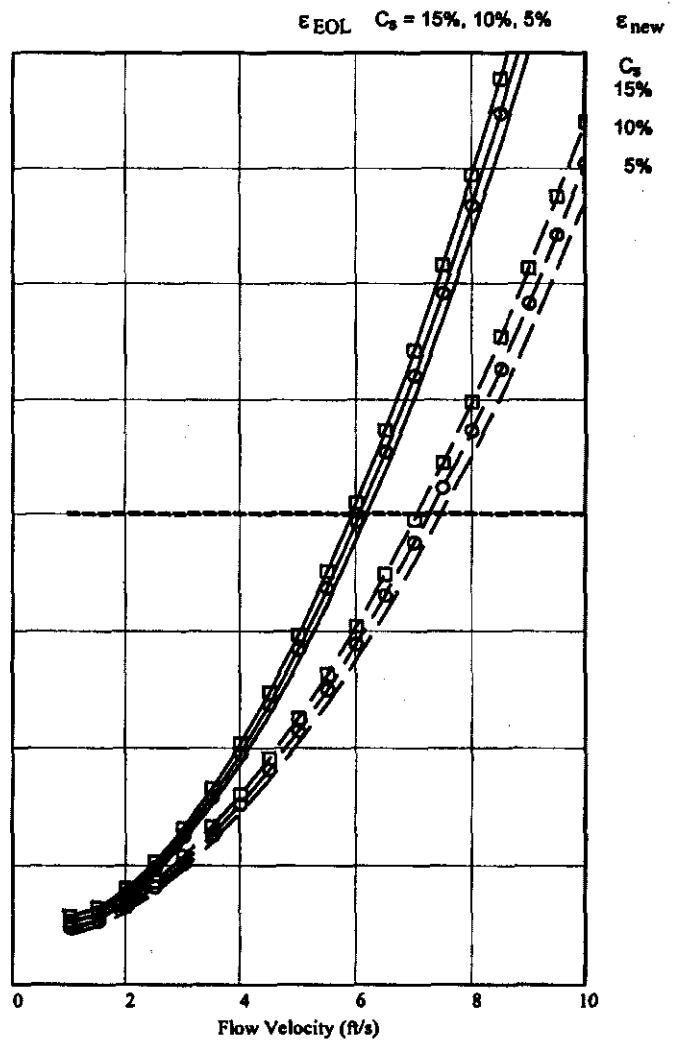
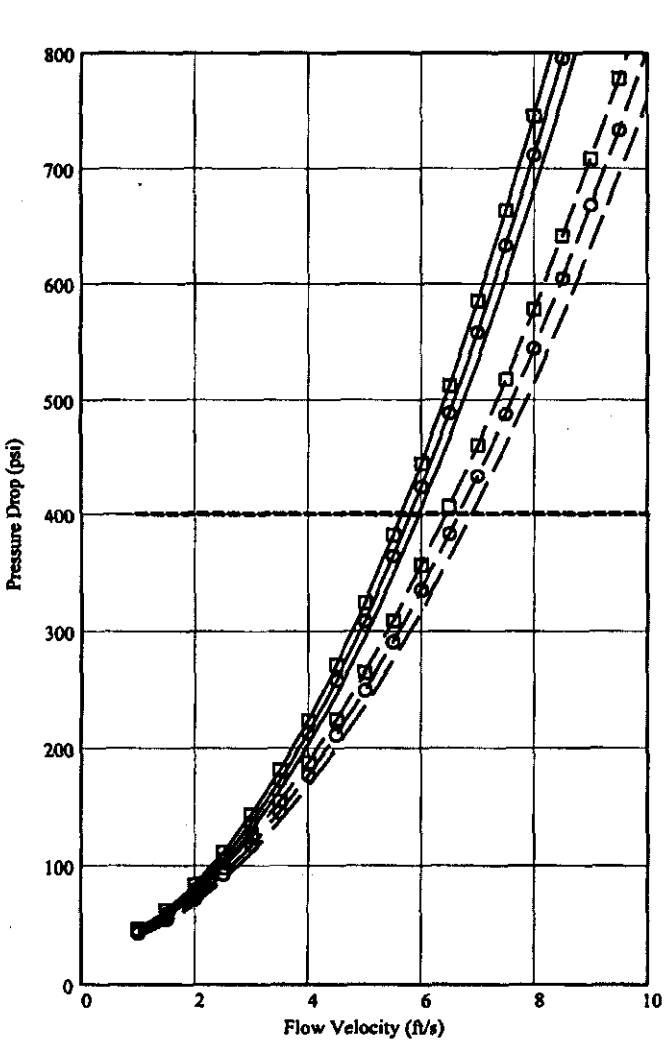
By: T. C. Oten

Revised:

By:

Figure E-3e. Pressure Drop for AN-107 to BNFL - $d_p=100 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$	$L_{CS} = 557 \text{ ft}$	$L_{SS} = 7028 \text{ ft}$	$H = 40 \text{ ft}$	$d_{50} = 100 \mu\text{m}$	$\epsilon_{new} = 2 \text{ mil}$
$\kappa_{cv} = 1.3$	$\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$	$\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$			$\epsilon_{EOL_{SS}} = 10 \text{ mil}$
$\kappa_f = 1$	$T := 10 \text{ }^\circ\text{C}$	$\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$	$T := 60 \text{ }^\circ\text{C}$	$\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$	$\epsilon_{EOL_{CS}} = 150 \text{ mil}$
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.7 \frac{\text{ft}}{\text{sec}}$			$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2.9 \frac{\text{ft}}{\text{sec}}$	C_s	
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3 \frac{\text{ft}}{\text{sec}}$			$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.2 \frac{\text{ft}}{\text{sec}}$	5%	
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.1 \frac{\text{ft}}{\text{sec}}$			$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.3 \frac{\text{ft}}{\text{sec}}$	10%	
				15%	



EVALUATION ANALYSIS

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WO/Job No. 110299 & 110300/BA10
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Figure E-4a. $d_p=40 \mu\text{m}$ Case - Cumulative and Probability Density Particle Size Distribution by Volume.

$d_{50} := 40 \mu\text{m}$

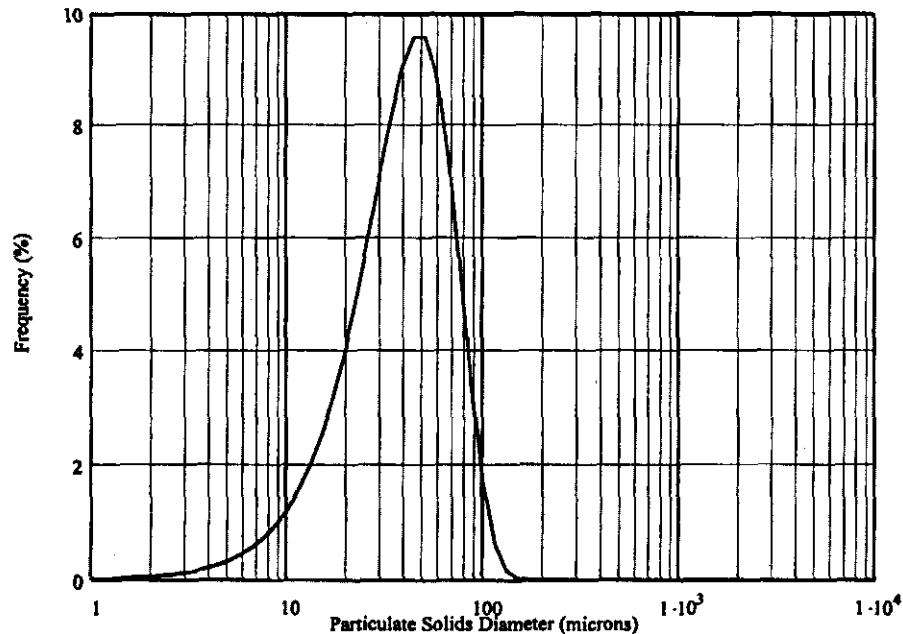
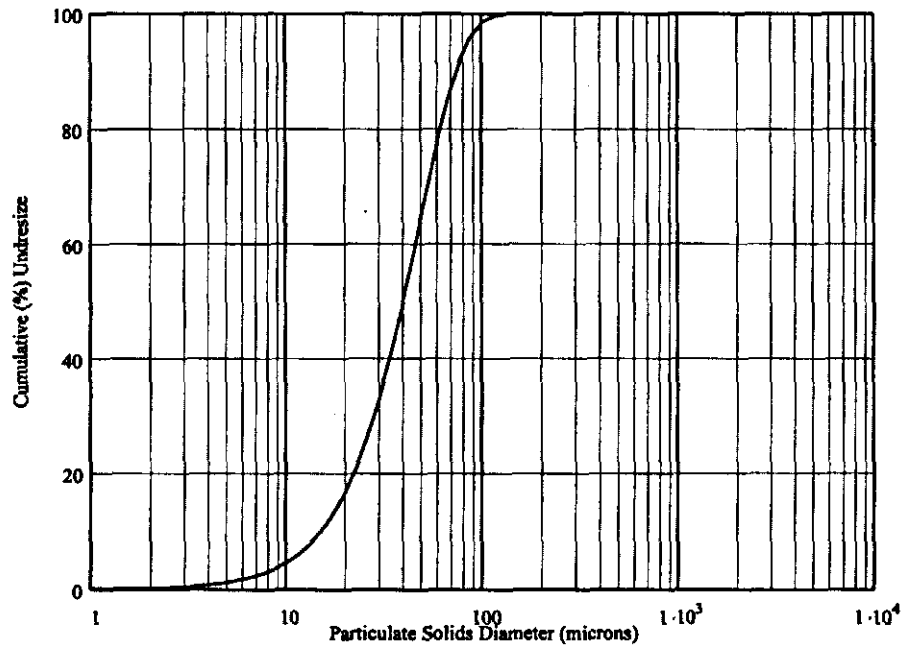
$(d_{pRR} \phi_{RR}) := \text{dist}_{RR}(d_{pR}, d_{50})$

$d_{pRR} := d_{pRR} \cdot \mu\text{m}$ reset units

$\phi_{RRcum} := \text{cum}(\phi_{RR})$

$D_{pRR} := D_p(d_{pRR}, \phi_{RRcum}, 50\%)$

$D_{pRR} = 37 \mu\text{m}$



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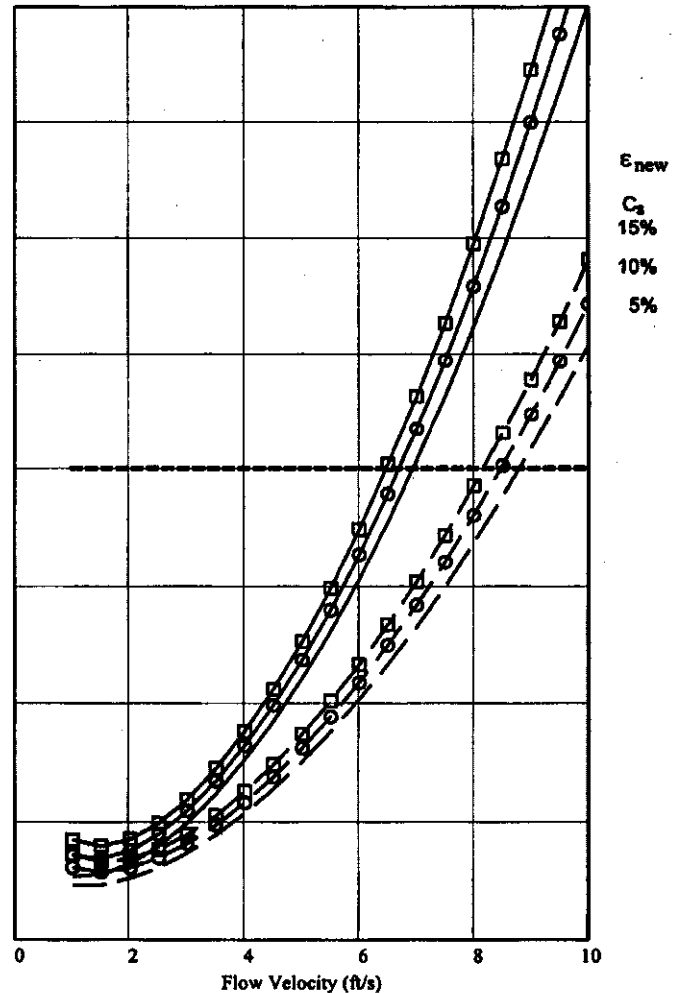
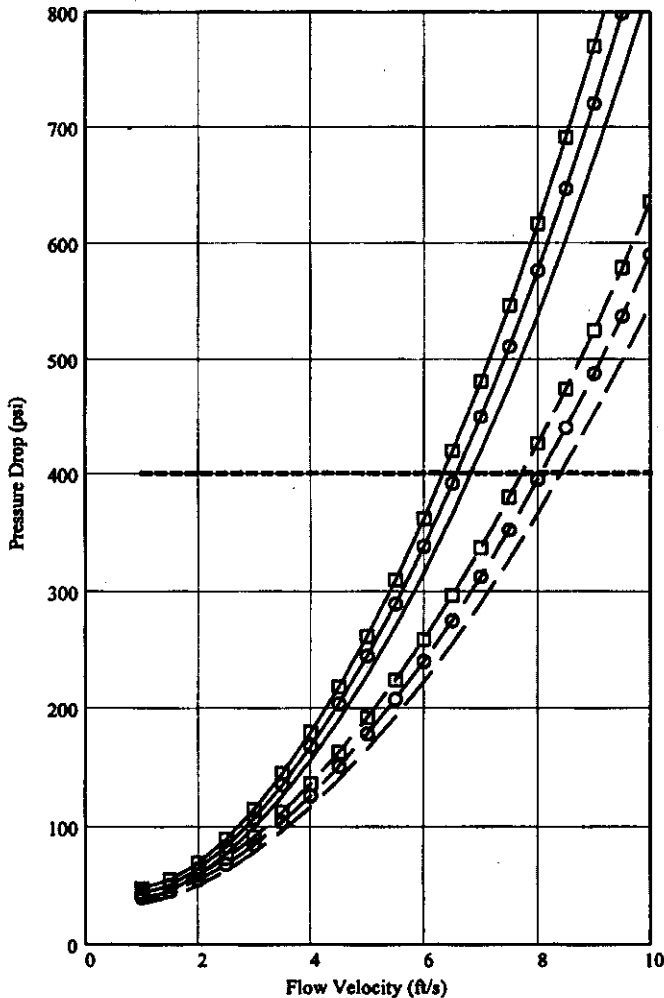
WO/Job No. 110299 & 110300/BA10
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Figure E-4b. Pressure Drop for AN-107 to BNFL - $d_p=40 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 40 \mu\text{m}$ $\epsilon_{\text{new}} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{\text{EOL}_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{\text{EOL}_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.9 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.3 \frac{\text{ft}}{\text{sec}}$ C_s 5%
 $v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.3 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.7 \frac{\text{ft}}{\text{sec}}$ 10%
 $v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 4.4 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.9 \frac{\text{ft}}{\text{sec}}$ 15%

$\epsilon_{\text{EOL}} C_s = 15\%, 10\%, 5\%$



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 Location: 200 Area - Hanford Site, Richland, Washington

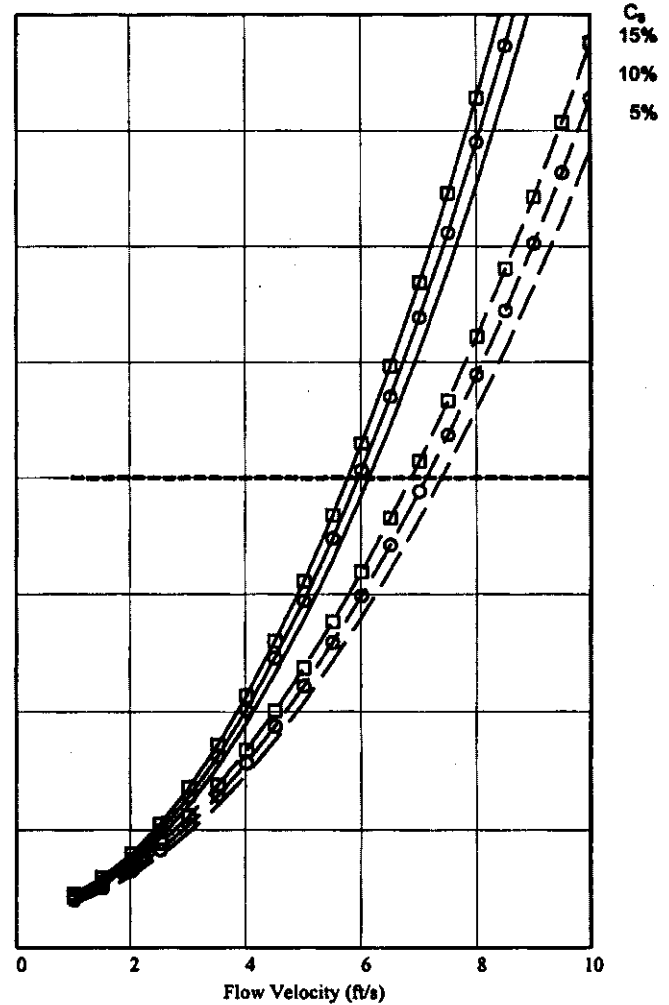
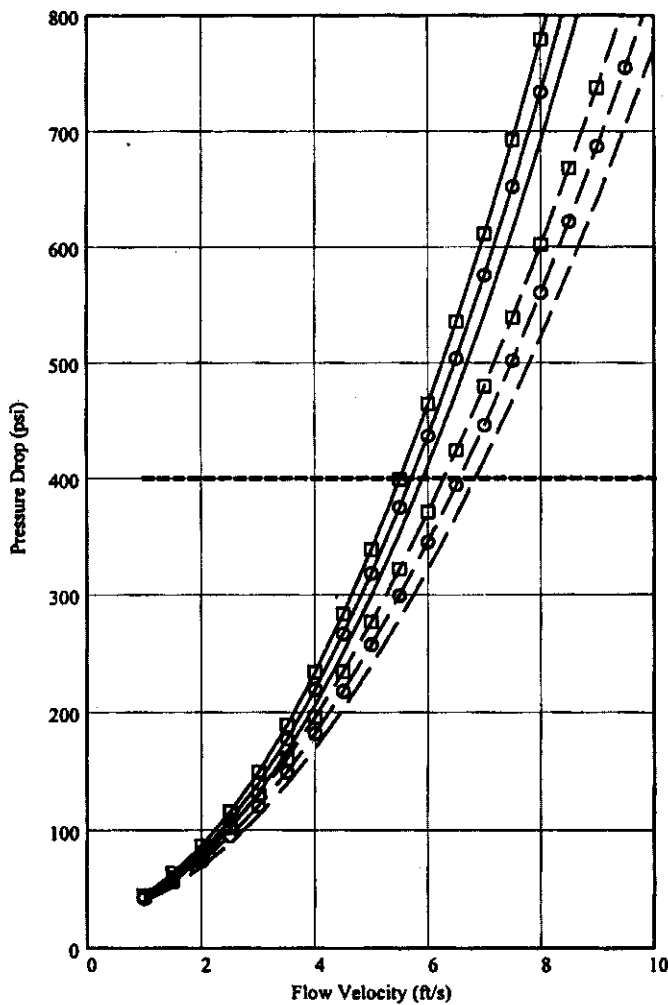
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Figure E-4c. Pressure Drop for AN-107 to BNFL - $d_p=40 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 40 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.9 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.1 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.2 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.4 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.3 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.5 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{EOL} \quad C_s = 15\%, 10\%, 5\% \quad \epsilon_{new}$



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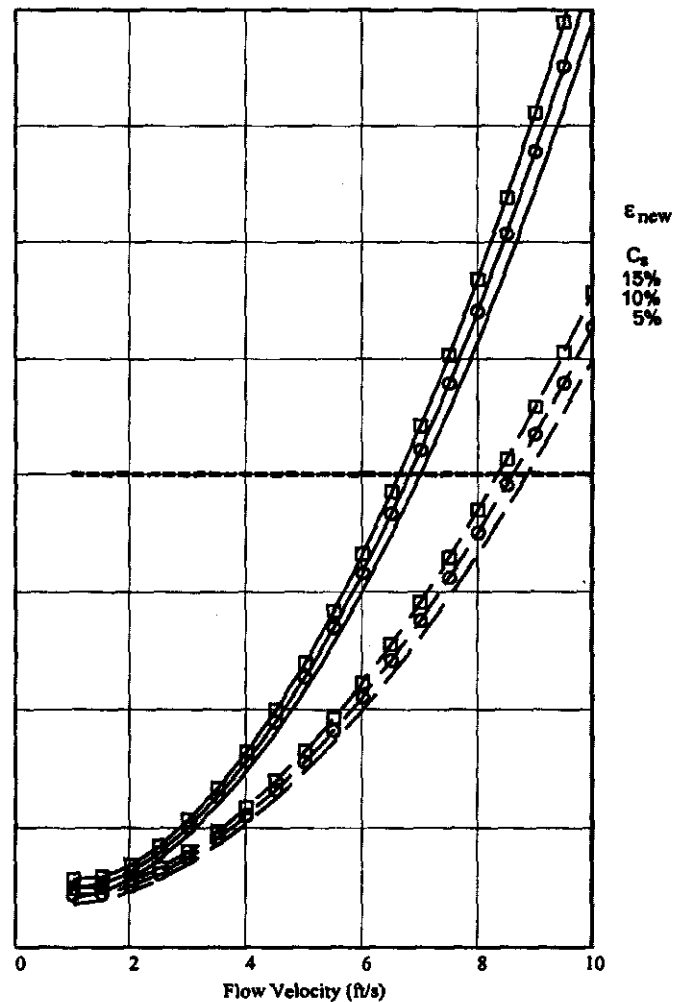
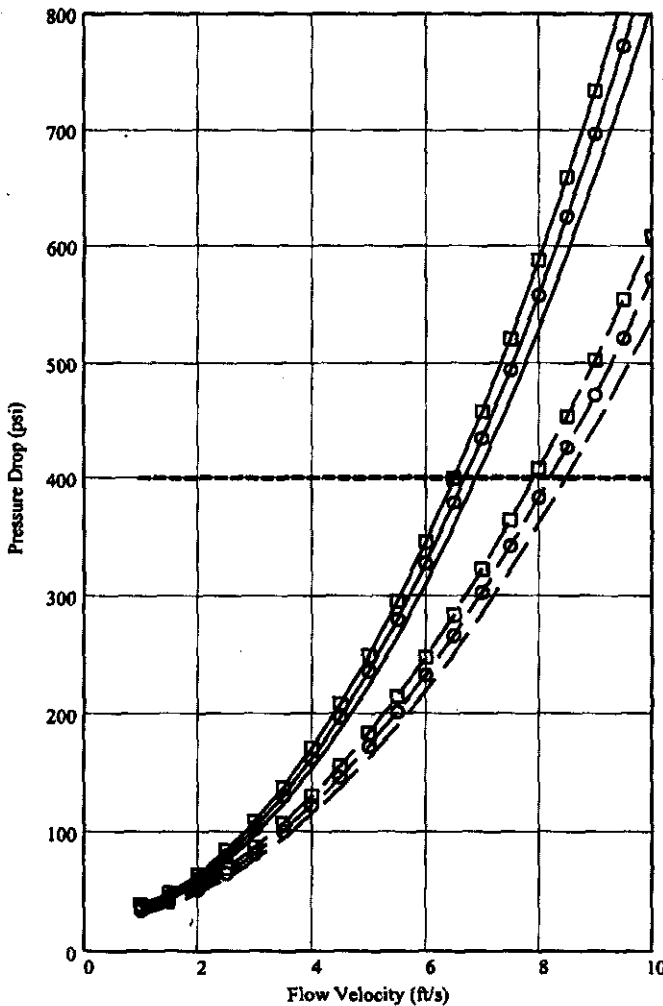
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Figure E-4d. Pressure Drop for AN-107 to BNFL - $d_p=40 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 40 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.3 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.6 \frac{\text{ft}}{\text{sec}}$ C_s
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.6 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.9 \frac{\text{ft}}{\text{sec}}$ 5%
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.7 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 4.1 \frac{\text{ft}}{\text{sec}}$ 10%
 15%

$\epsilon_{EOL} C_s = 15\%, 10\%, 5\%$



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Figure E-4e. Pressure Drop for AN-107 to BNFL - $d_p=40 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$

$L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 40 \mu\text{m}$

$\epsilon_{new} = 2 \text{ mil}$

$\kappa_{cv} = 1.3$

$\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$

$\epsilon_{EOL_{SS}} = 10 \text{ mil}$

$\kappa_f = 1$

$T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$

$T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$

$\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.3 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2.5 \frac{\text{ft}}{\text{sec}}$ $C_s \text{ 5\%}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.5 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2.7 \frac{\text{ft}}{\text{sec}}$ $C_s \text{ 10\%}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.7 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2.9 \frac{\text{ft}}{\text{sec}}$ $C_s \text{ 15\%}$

$\epsilon_{EOL} \quad C_s = 15\%, 10\%, 5\%$

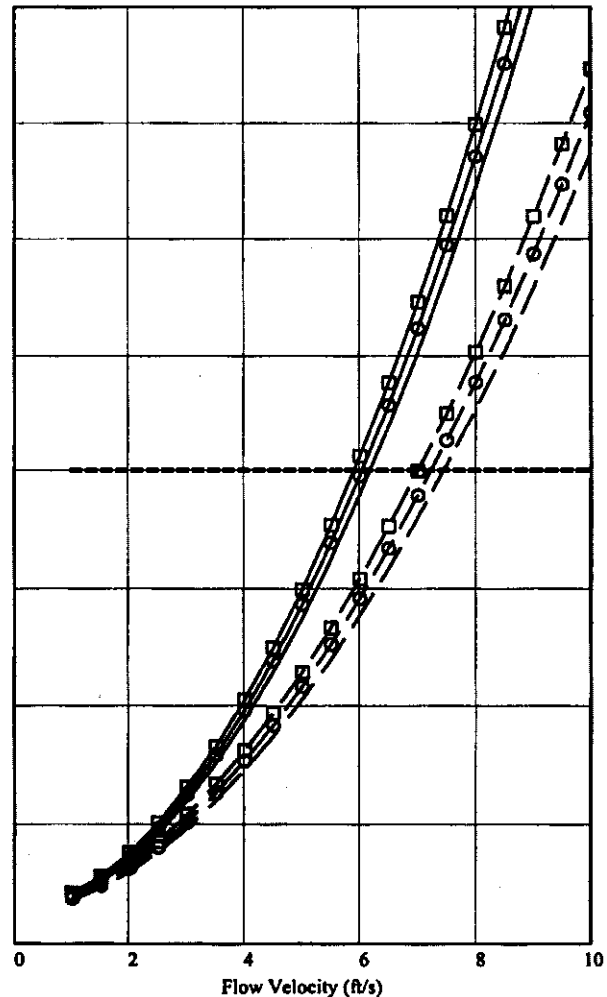
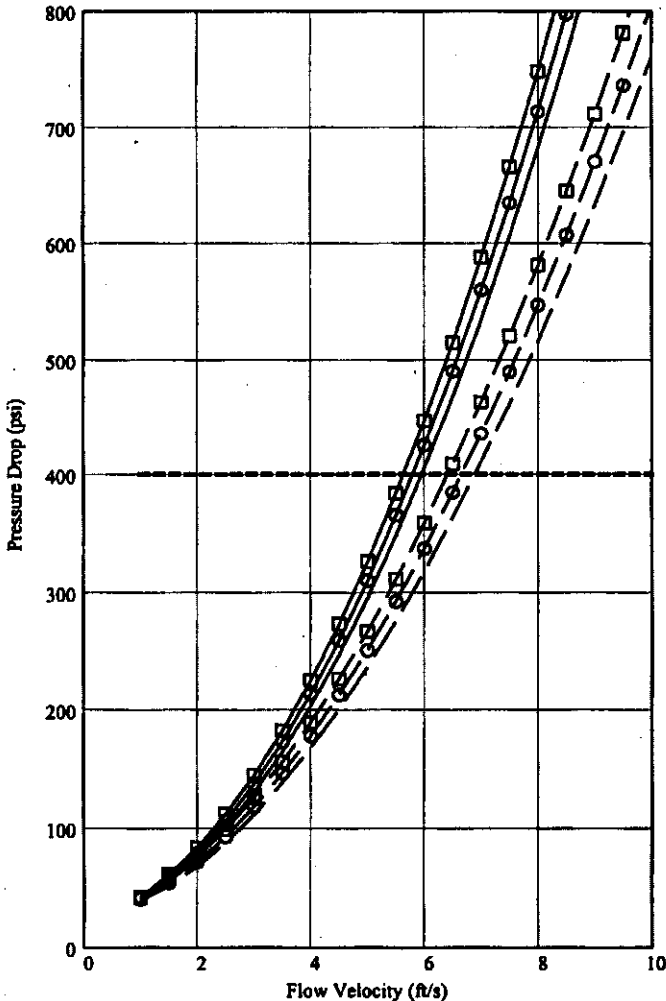
ϵ_{new}

C_s

15%

10%

5%



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Figure E-5a. $d_p=5 \mu\text{m}$ Case - Cumulative and Probability Density Particle Size Distribution by Volume.

$d_{50} := 5 \mu\text{m}$

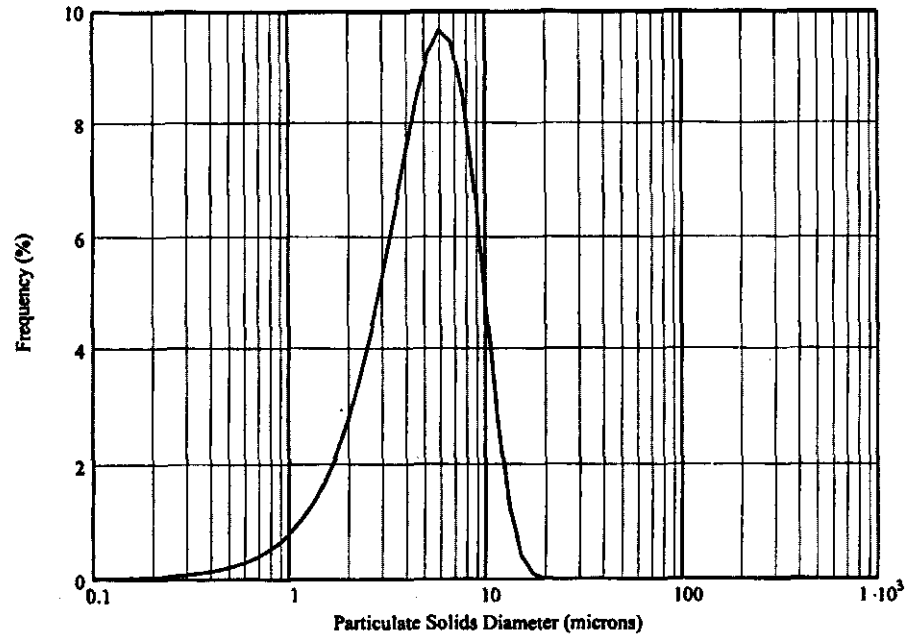
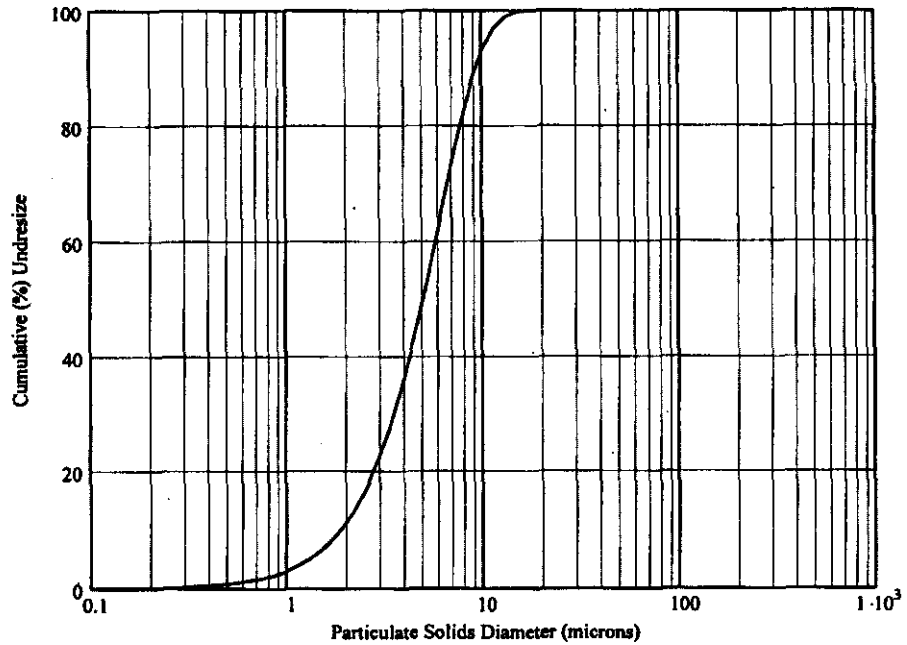
$(d_{pRR} \ \phi_{RR}) := \text{dist}_{RR}(d_{pR}, d_{50})$

$d_{pRR} := d_{pRR} \cdot \mu\text{m}$ reset units

$\phi_{RRcum} := \text{cum}(\phi_{RR})$

$D_{pRR} := D_{\beta}(d_{pRR}, \phi_{RRcum}, 50\%)$

$D_{pRR} = 5 \mu\text{m}$



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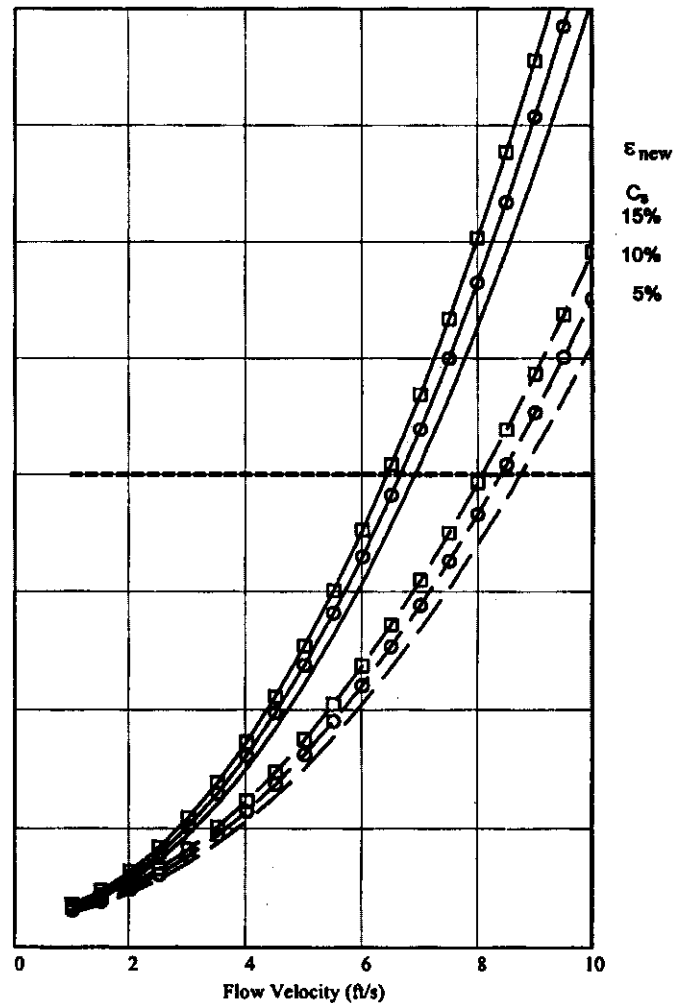
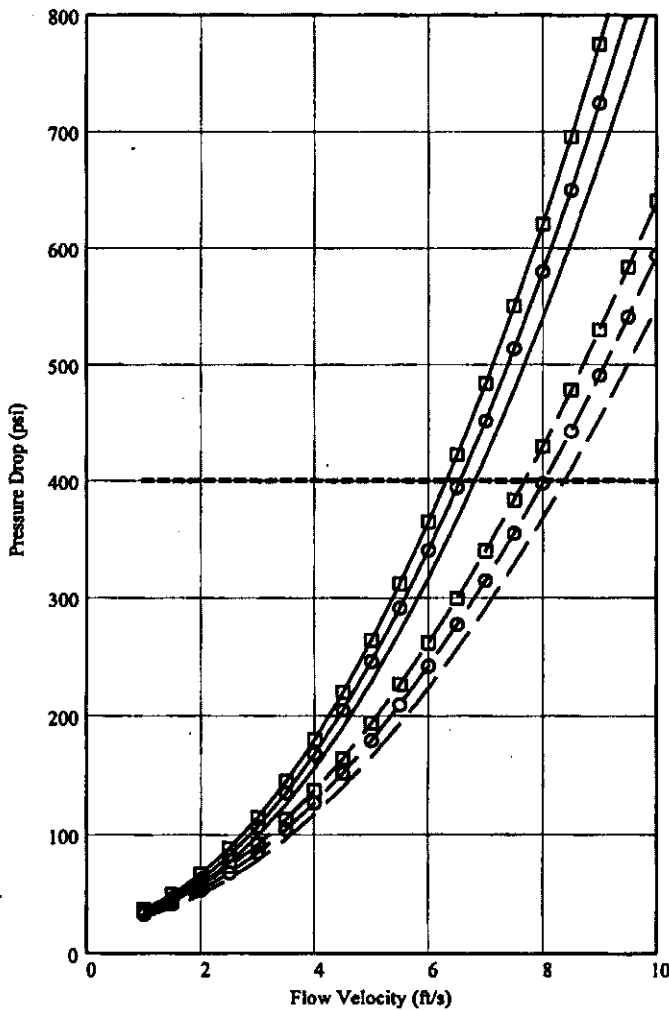
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Figure E-5b. Pressure Drop for AN-107 to BNFL - $d_p=5 \mu\text{m}$, $\rho_L=1.2 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 5 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 2 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 0.8 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.8 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3 \frac{\text{ft}}{\text{sec}}$ C_s
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.3 \frac{\text{ft}}{\text{sec}}$ 5%
 $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 3.1 \frac{\text{ft}}{\text{sec}}$ $v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 3.4 \frac{\text{ft}}{\text{sec}}$ 10%
 15%

$\epsilon_{EOL} C_s = 15\%, 10\%, 5\%$



EVALUATION ANALYSIS

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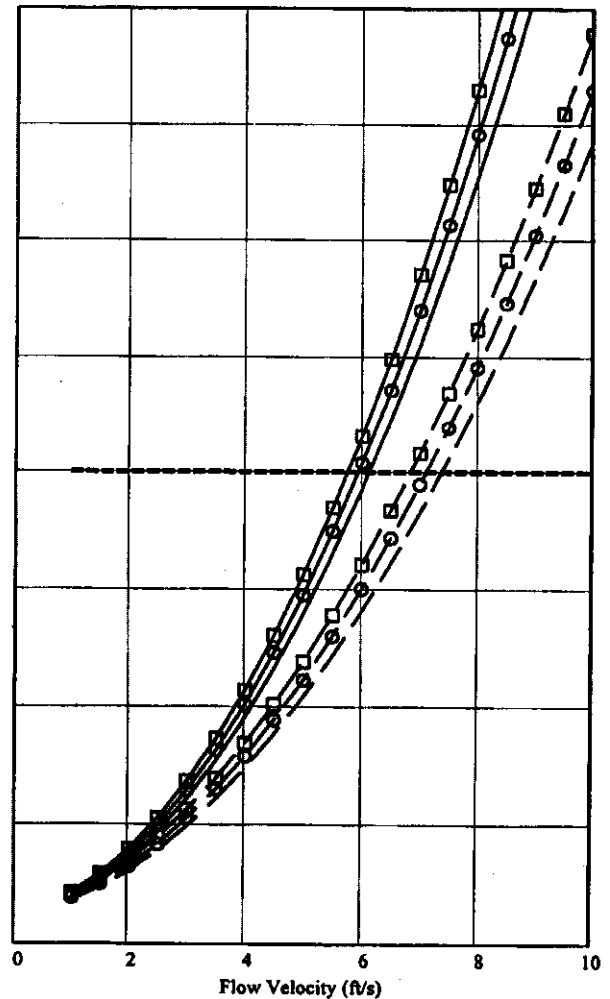
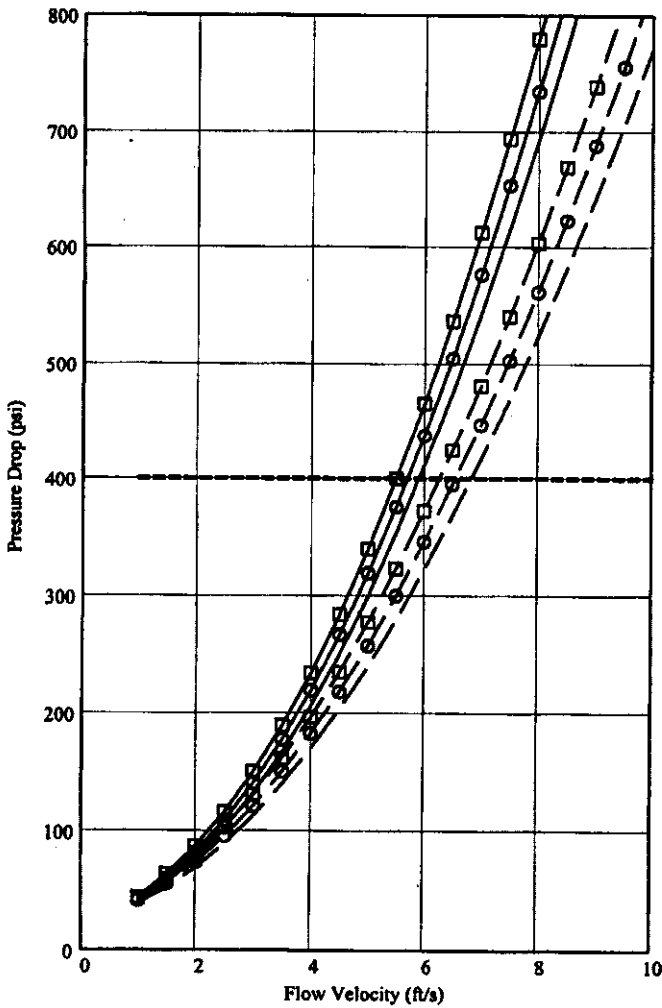
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Figure E-5c. Pressure Drop for AN-107 to BNFL - $d_p=5 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 5 \mu\text{m}$ $\epsilon_{\text{new}} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{\text{EOL}_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{\text{EOL}_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2.2 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.2 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2.4 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 2.3 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{\text{pipe}}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2.5 \frac{\text{ft}}{\text{sec}}$	15%

ϵ_{EOL} $C_s = 15\%, 10\%, 5\%$ ϵ_{new}



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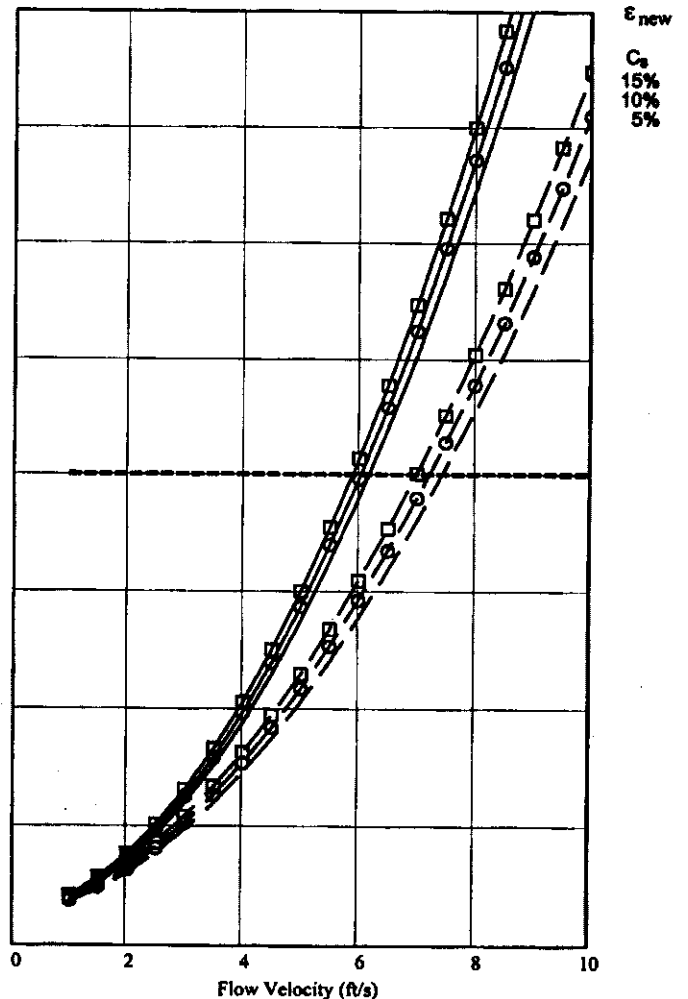
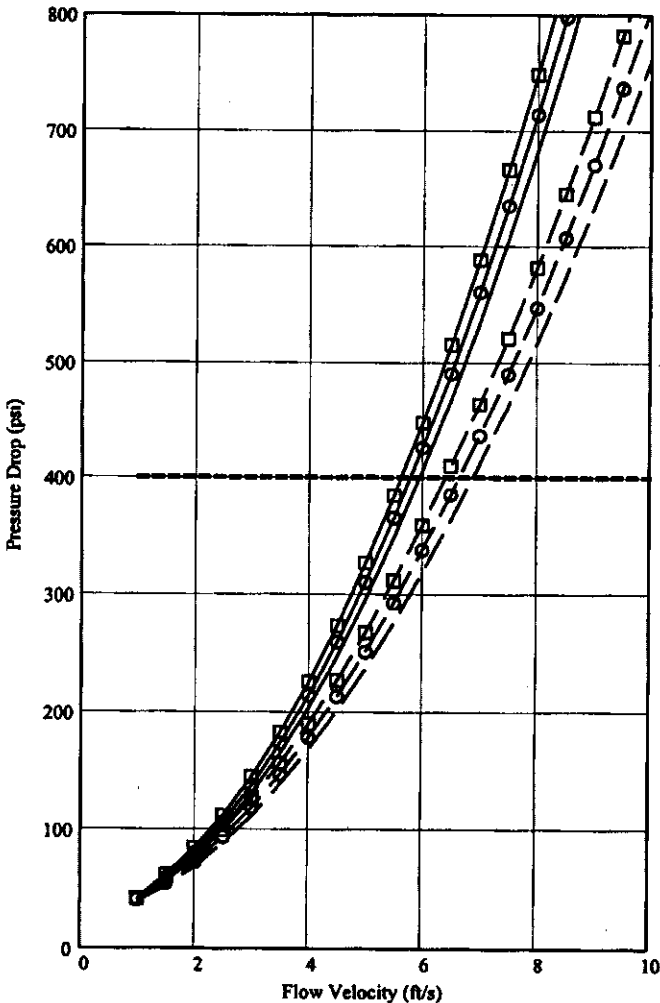
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Figure E-5e. Pressure Drop for AN-107 to BNFL - $d_p=5 \mu\text{m}$, $\rho_L=1.46 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$ Case, $T = 10$ and $60 \text{ }^\circ\text{C}$.

$\beta_{cv} = 1.2$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $d_{50} = 5 \mu\text{m}$ $\epsilon_{new} = 2 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\rho_L := 1.46 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$
 $\kappa_f = 1$ $T := 10 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 10) = 8.4 \text{ cP}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_{cL}(\rho_L, 60) = 3.7 \text{ cP}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 1.6 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 5\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 1.8 \frac{\text{ft}}{\text{sec}}$	C_s 5%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 1.8 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 10\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 1.9 \frac{\text{ft}}{\text{sec}}$	10%
$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 10), \kappa_{cv}, \beta_{cv}) = 1.9 \frac{\text{ft}}{\text{sec}}$	$v_{cr}(D_{pipe}, d_{50}, \rho_s, 15\%, \rho_L, \mu_{cL}(\rho_L, 60), \kappa_{cv}, \beta_{cv}) = 2 \frac{\text{ft}}{\text{sec}}$	15%

$\epsilon_{EOL} \quad C_s = 15\%, 10\%, 5\%$



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Figure E-6a. Critical Velocity Comparison - New Pipe.

$D_{pipe} := 3.068 \text{ in}$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $T := 60 \text{ }^\circ\text{C}$ $\mu_L := \mu_{cL}(\rho_L, T)$ $\mu_L = 0.77 \text{ cP}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\epsilon := \epsilon_{new}$
 $\kappa_f = 1$ $\kappa_{cv} := 1$ best-estimate $d_{50} := 400 \text{ }\mu\text{m}$ $C_s := 15\%$ $TOL = 1 \times 10^{-5}$ $\epsilon = 2 \text{ mil}$

$(d_{pRR} \phi_{RR}) := \text{dist}_{RR}(d_{pR}, d_{50})$ $d_{pRR} := d_{pRR} \text{ }\mu\text{m}$ reset units
 $\phi_{RRcum} := \text{cum}(\phi_{RR})$ $\text{length}(\phi_{RR}) - 1 = 57$

Settling velocity

$v_{\infty}(d_{50}, \rho_s, \rho_L, \mu_L) = 0.22 \frac{\text{ft}}{\text{sec}}$

$v_{\infty}(d_{50}, \rho_s, \rho_c(\rho_s, \rho_L, C_s), \mu_c(\mu_L, C_s)) = 0.15 \frac{\text{ft}}{\text{sec}}$

- 1 Oroskar and Turian (1980) maximum of semi-theoretical and empirical equation
- 1.1 Oroskar and Turian (1980) semi-theoretical equation
- 1.2 Oroskar and Turian (1980) empirical equation
- $\beta_{cv} = 2$ Wasp et al. (1977)
- 3 Zandi and Govatos (1967)
- 4 Shook (1969)
- 5 Gogus and Kokpinar (1999)
- 6 Robinson and Graf (1972)

Critical velocity using β_{cv} selected correlations

$v_{cr}(D_{pipe}, d_{50}, \rho_s, C_s, \rho_L, \mu_L, \kappa_{cv}, 1.2) = 5.48 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, C_s, \rho_L, \mu_L, \kappa_{cv}, 2) = 4.52 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, C_s, \rho_L, \mu_L, \kappa_{cv}, 3) = 7.48 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, C_s, \rho_L, \mu_L, \kappa_{cv}, 4) = 5.58 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, C_s, \rho_L, \mu_L, \kappa_{cv}, 5) = 6.23 \frac{\text{ft}}{\text{sec}}$

$v_{cr}(D_{pipe}, d_{50}, \rho_s, C_s, \rho_L, \mu_L, \kappa_{cv}, 6) = 3.66 \frac{\text{ft}}{\text{sec}}$

Critical velocity using β_{cv} selected correlations with Wasp vehicle concept

$v_{crD_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, 1.2) = 5.333 \frac{\text{ft}}{\text{sec}}$

$v_{crD_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, 2) = 4.37 \frac{\text{ft}}{\text{sec}}$

$v_{crD_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, 3) = 6.99 \frac{\text{ft}}{\text{sec}}$

$v_{crD_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, 4) = 5.29 \frac{\text{ft}}{\text{sec}}$

$v_{crD_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, 5) = 6.05 \frac{\text{ft}}{\text{sec}}$

$v_{crD_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, 6) = 3.56 \frac{\text{ft}}{\text{sec}}$

$v := 3 \frac{\text{ft}}{\text{sec}}$ $f_W := f_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_{pRR}, \rho_s, C_s \phi_{RR}, \kappa_f, \theta)$ $f_W = 0.28277$ $\beta_{cv} := 1.2$

$\Phi_v := \Phi_c(v, \rho_L, \mu_L, d_{pRR}, \rho_s, C_s \phi_{RR}, f_W)$

$\Phi_v = 1.539\%$

$\rho_c(\rho_s, \rho_L, \Phi_v) = 1.228 \frac{\text{kg}}{\text{liter}}$

$\mu_c(\mu_L, \Phi_v) = 0.805 \text{ cP}$

$v_{cr_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, \beta_{cv}) = 5.333 \frac{\text{ft}}{\text{sec}}$

$v_{crT_Wasp}(D_{pipe}, d_{pRR}, \rho_s, C_s \phi_{RR}, \rho_L, \mu_L, \epsilon, \kappa_{cv}) = 0.123 \frac{\text{ft}}{\text{sec}}$

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Figure E-6b. Pressure Drop at Critical Velocity Comparison for New Pipe at Selected Median Particle Sizes - AN-107-to-BNFL Transfer Route.

$\epsilon_{CS} := \epsilon_{new}$ $\epsilon_{SS} := \epsilon_{new}$

$D_{pipe} = 3.068 \text{ in}$ $L_{CS} := 557 \text{ ft}$ $L_{SS} := 7028 \text{ ft}$ $H := 40 \text{ ft}$ $\theta := \text{atan}\left(\frac{H}{L_{CS} + L_{SS}}\right)$

$C_s = 15\%$ $\rho_s = 3 \frac{\text{kg}}{\text{liter}}$ $T = 60 \text{ }^\circ\text{C}$ $\rho_L = 1.2 \frac{\text{kg}}{\text{liter}}$ $\mu_L = 0.771 \text{ cP}$

$i := 0..12$

$\kappa_f = 1$

$d_{50_i} :=$

$\kappa_{cv} = 1$

Best-estimate

$\beta_{cv} = 1.2$

Critical velocity - Oroskar and Turian (1980) empirical equation

5
10
40
60
100
150
200
250
300
350
400
450
500

$d_{50} := d_{50} \mu\text{m}$

$d_{pRR} := 0$ $\phi_{RR} := 0$

$(d_{pRR}^{(i)} \phi_{RR}^{(i)}) := \text{dist}_{RR}(d_{pR}, d_{50_i})$ $d_{pRR} := d_{pRR} \mu\text{m}$ reset units

$v_{cm_i} := v_{cr}(D_{pipe}, d_{50_i}, \rho_s, C_s, \rho_L, \mu_L, \kappa_{cv}, \beta_{cv})$ Critical velocity -Oroskar and Turian (1980) empirical equation

$\Delta p_{cm_i} := \Delta p_{T_Wasp}(D_{pipe}, v_{cm_i}, \rho_L, \mu_L, \epsilon_{CS}, \epsilon_{SS}, d_{pRR}^{(i)}, \rho_s, C_s, \phi_{RR}^{(i)}, L_{CS}, L_{SS}, \kappa_f, \theta)$

$v_{cm_Wasp_i} := v_{cr_Wasp}(D_{pipe}, d_{pRR}^{(i)}, \rho_s, C_s, \phi_{RR}^{(i)}, \rho_L, \mu_L, \epsilon, \kappa_{cv}, \beta_{cv})$ Critical velocity using Wasp vehicle concept

$\Delta p_{cm_Wasp_i} := \Delta p_{T_Wasp}(D_{pipe}, v_{cm_Wasp_i}, \rho_L, \mu_L, \epsilon_{CS}, \epsilon_{SS}, d_{pRR}^{(i)}, \rho_s, C_s, \phi_{RR}^{(i)}, L_{CS}, L_{SS}, \kappa_f, \theta)$

Critical velocity -Oroskar and Turian (1980) empirical equation

Critical velocity using Wasp vehicle concept

$\frac{d_{50}}{\mu\text{m}} =$	$\frac{v_{cm}}{\frac{\text{ft}}{\text{sec}}} =$	$\frac{\Delta p_{cm}}{\frac{\text{lb}_f}{\text{in}^2}} =$	$\frac{v_{cm_Wasp}}{\frac{\text{ft}}{\text{sec}}} =$	$\frac{\Delta p_{cm_Wasp}}{\frac{\text{lb}_f}{\text{in}^2}} =$
5	2.64	70	1.24	37
10	2.96	81	1.63	44
40	3.73	114	2.84	85
60	3.99	136	3.29	115
100	4.35	185	3.87	174
150	4.65	246	4.31	241
200	4.88	301	4.62	299
250	5.07	349	4.85	349
300	5.23	392	5.04	393
350	5.36	431	5.2	432
400	5.48	465	5.33	467
450	5.59	496	5.45	498
500	5.69	524	5.56	525

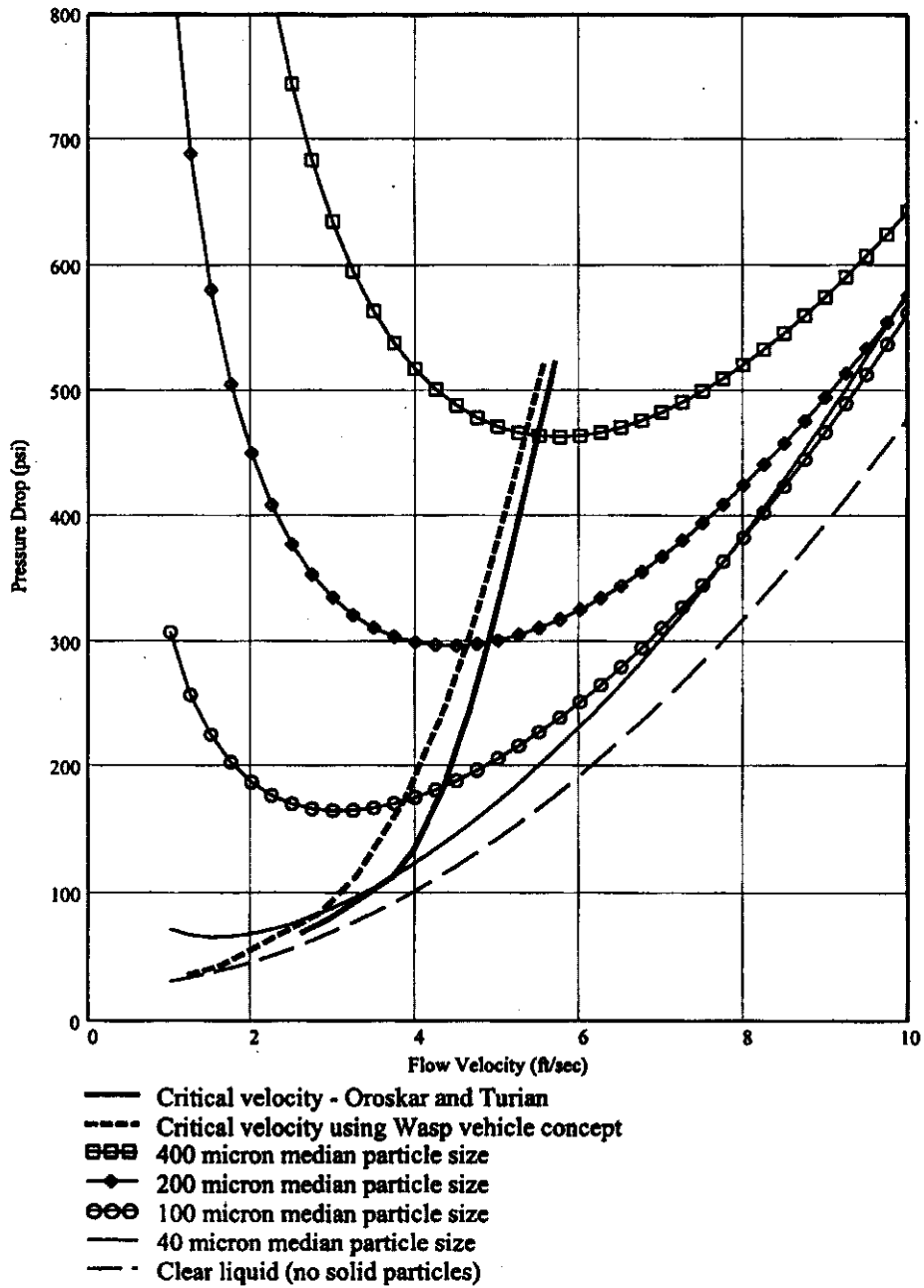
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Figure E-6c. Pressure Drop and Critical Velocity Comparison for New Pipe at Selected Median Particle Sizes - AN-107-to-BNFL Transfer Route.

$D_{pipe} = 3.068 \text{ in}$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $\epsilon_{CS} = 2 \text{ mil}$ $\epsilon_{SS} = 2 \text{ mil}$

$C_s = 15\%$ $\rho_s = 3 \frac{\text{kg}}{\text{liter}}$ $T = 60 \text{ }^\circ\text{C}$ $\rho_L = 1.2 \frac{\text{kg}}{\text{liter}}$ $\mu_L = 0.771 \text{ cP}$ $\kappa_f = 1$ $\kappa_{cv} = 1$ $\beta_{cv} = 1.2$



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$\kappa_f := 1$ $\beta_f := 1$

$\kappa_{cv} := 1.3$ $\beta_{cv} := 1.2$

Pressure Drop (Wasp et al. 1977) at Critical Velocity (Oroskar and Turian 1980)

$TOL = 1 \times 10^{-5}$

$$\Delta p_{cr}(D_{pipe}, \rho_L, T, \epsilon_{CS}, \epsilon_{SS}, d_{50}, \rho_s, C_s, L_{CS}, L_{SS}, H) := \left\{ \begin{array}{l} \theta \leftarrow \text{atan}\left(\frac{H}{L_{CS} + L_{SS}}\right) \\ \mu \leftarrow \mu_{cl}(\rho_L, T) \\ v_{crm} \leftarrow v_{cr}(D_{pipe}, d_{50}, \rho_s, C_s, \rho_L, \mu, \kappa_{cv}, \beta_{cv}) \\ J \leftarrow \text{length}(d_{pR}) - 1 \\ \Delta d \leftarrow \Delta(d_{pR}) \\ \text{for } j \in 0..J \\ \phi_j \leftarrow \text{PDF}_{RR}(d_{pR}, d_{50}, PRR) \cdot \Delta d_j \\ d_{pRR} \leftarrow \text{pack}(d_{pR}, \phi) \\ \phi_{RR} \leftarrow \text{pack}(\phi, \phi) \\ \Delta P_{T_Wasp}(D_{pipe}, v_{crm}, \rho_L, \mu, \epsilon_{CS}, \epsilon_{SS}, d_{pRR}, \rho_s, C_s, \phi_{RR}, L_{CS}, L_{SS}, \kappa_f, \theta) \end{array} \right.$$

$d_{50} :=$

5
10
20
40
80
100
150
200
250
300
350
400

Shifted Tank 241-SY-101 particle size distribution by volume (O'Rourke 1999) for specified median particle size (d_{50})

$d_{50} := d_{50} \cdot \mu\text{m}$

AN-107-to-BNFL transfer route pipe parameters

$D_{pipe} := 3.068\text{-in}$ $L_{CS} := 557\text{-ft}$ $L_{SS} := 7028\text{-ft}$ $H := 40\text{-ft}$

Temperature of slurry at transfer

$T_1 := 10 \text{ }^\circ\text{C}$ $T_2 := 60 \text{ }^\circ\text{C}$

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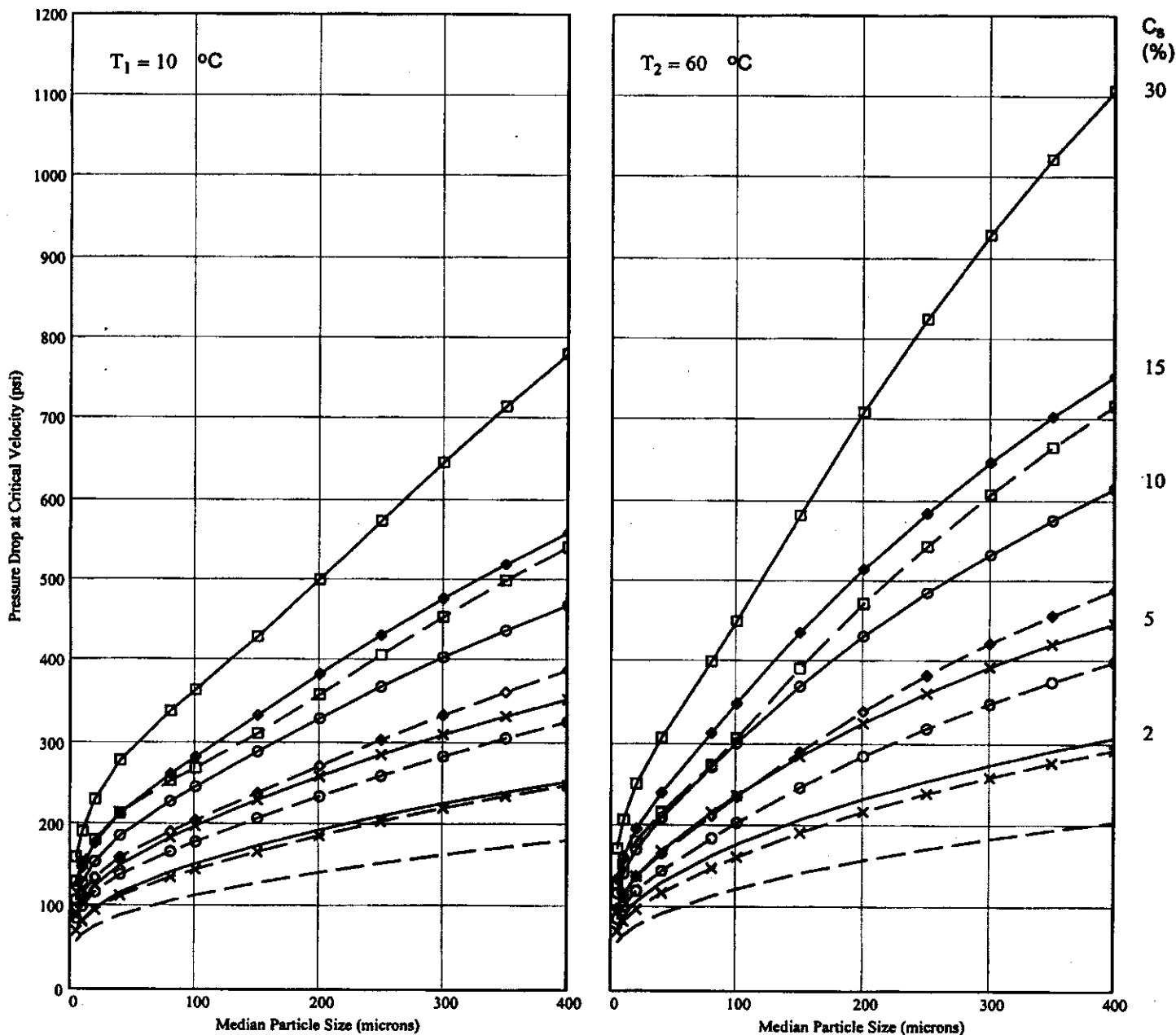
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By: _____

$\kappa_f = 1$ $\beta_f = 1$ $D_{pipe} = 3.068 \text{ in}$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $\epsilon_{new} = 2 \text{ mil}$ $\epsilon_{EOL_CS} = 150 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\mu_{cL}(\rho_L, T_1) = 2.021 \text{ cP}$ $\mu_{cL}(\rho_L, T_2) = 0.771 \text{ cP}$ $\epsilon_{EOL_SS} = 10 \text{ mil}$

Figure E-7a. Pressure Drop at Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.2 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$, $T = 10$ and 60°C , for New (dashed lines) and EOL (solid lines) Pipe Roughness Estimates.



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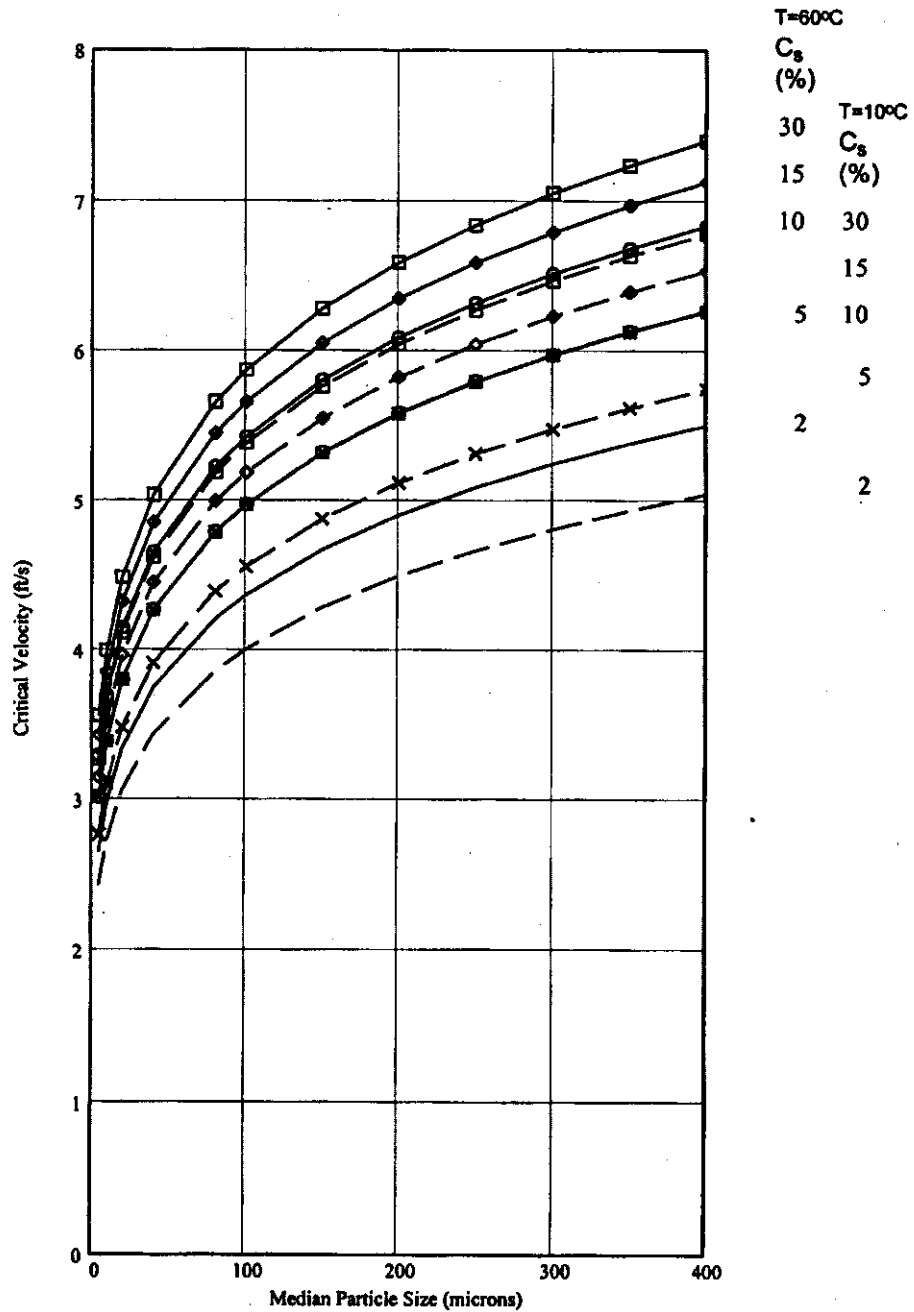
By: T. C. Oten

Revised:

By:

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 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L = 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s = 3 \frac{\text{kg}}{\text{liter}}$ $\mu_{cL}(\rho_L, T_1) = 2.021 \text{ cP}$ $\mu_{cL}(\rho_L, T_2) = 0.771 \text{ cP}$ $\epsilon_{EOL_SS} = 10 \text{ mil}$

Figure E-7b. Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.2 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$, $T = 10$ (dashed lines) and 60°C (solid lines).



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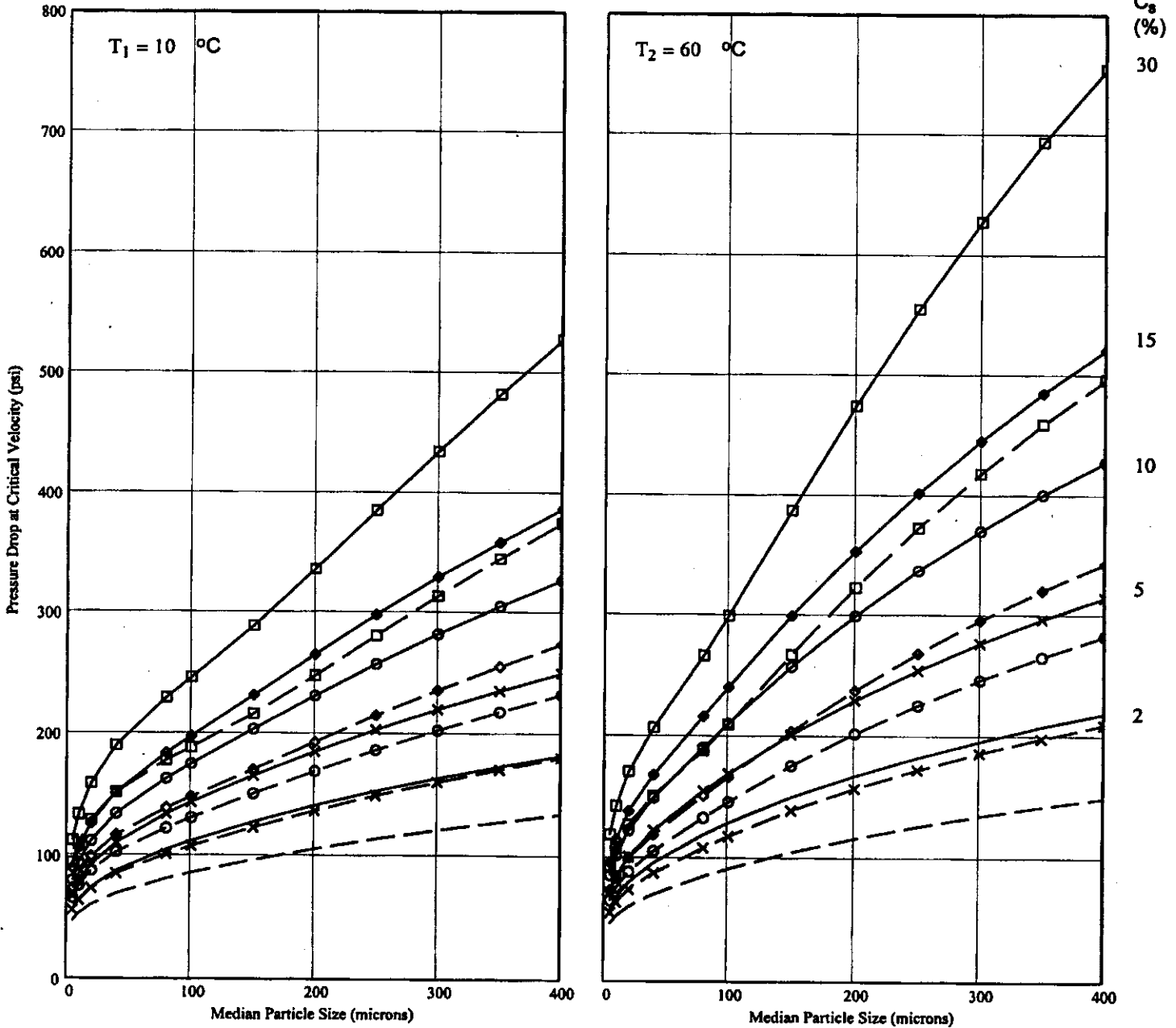
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 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L := 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\mu_{cL}(\rho_L, T_1) = 2.021 \text{ cP}$ $\mu_{cL}(\rho_L, T_2) = 0.771 \text{ cP}$ $\epsilon_{EOL_SS} = 10 \text{ mil}$

Figure E-8a. Pressure Drop at Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.2 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$, $T = 10$ and 60°C , for New (dashed lines) and EOL (solid lines) Pipe Roughness Estimates.



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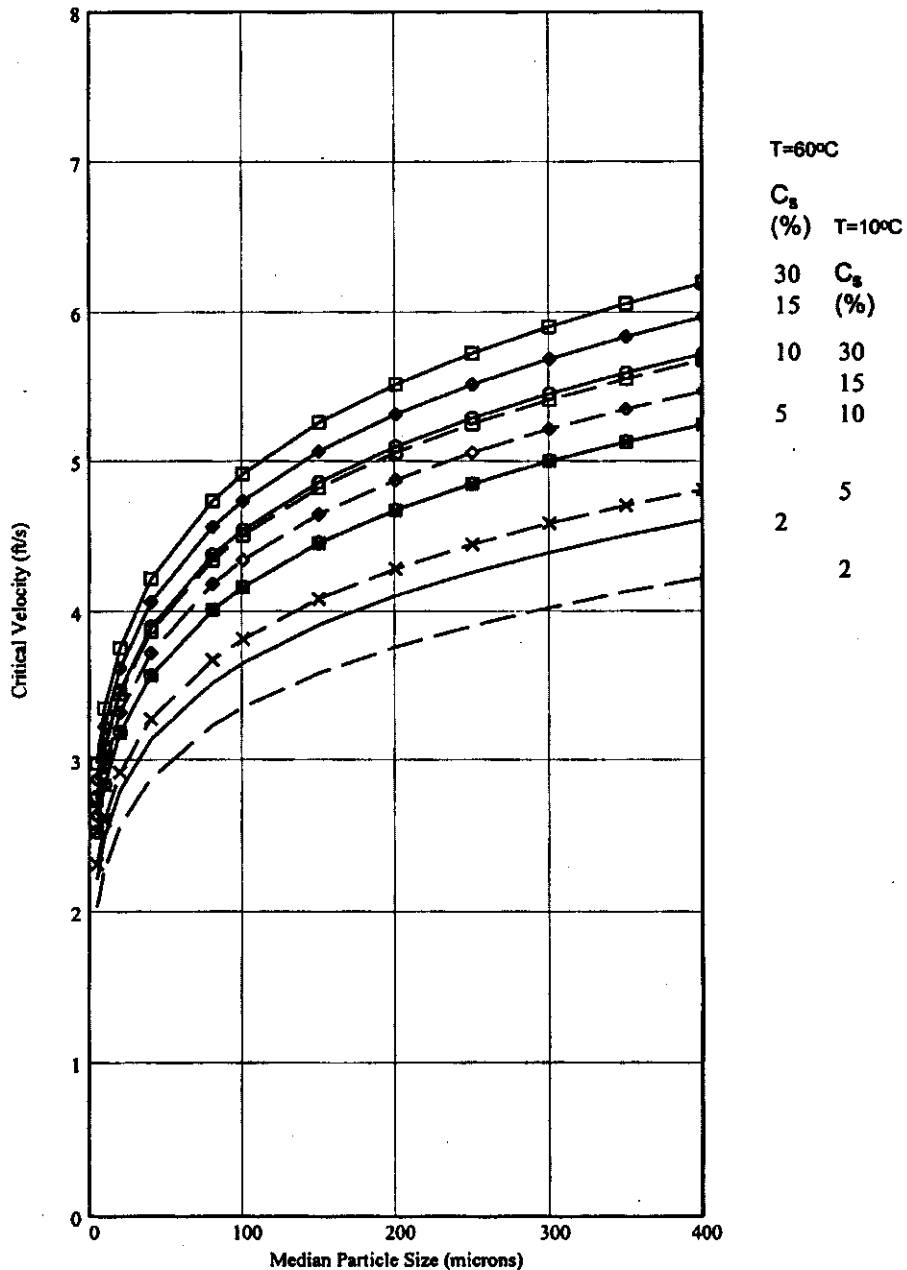
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$\kappa_f = 1$ $\beta_f = 1$ $D_{pipe} = 3.068 \text{ in}$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $\epsilon_{new} = 2 \text{ mil}$ $\epsilon_{EOL_CS} = 150 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L = 1.2 \frac{\text{kg}}{\text{liter}}$ $\rho_s = 2.5 \frac{\text{kg}}{\text{liter}}$ $\mu_{cL}(\rho_L, T_1) = 2.021 \text{ cP}$ $\mu_{cL}(\rho_L, T_2) = 0.771 \text{ cP}$ $\epsilon_{EOL_SS} = 10 \text{ mil}$

Figure E-8b. Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.2 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$, $T= 10$ (dashed lines) and 60°C (solid lines).

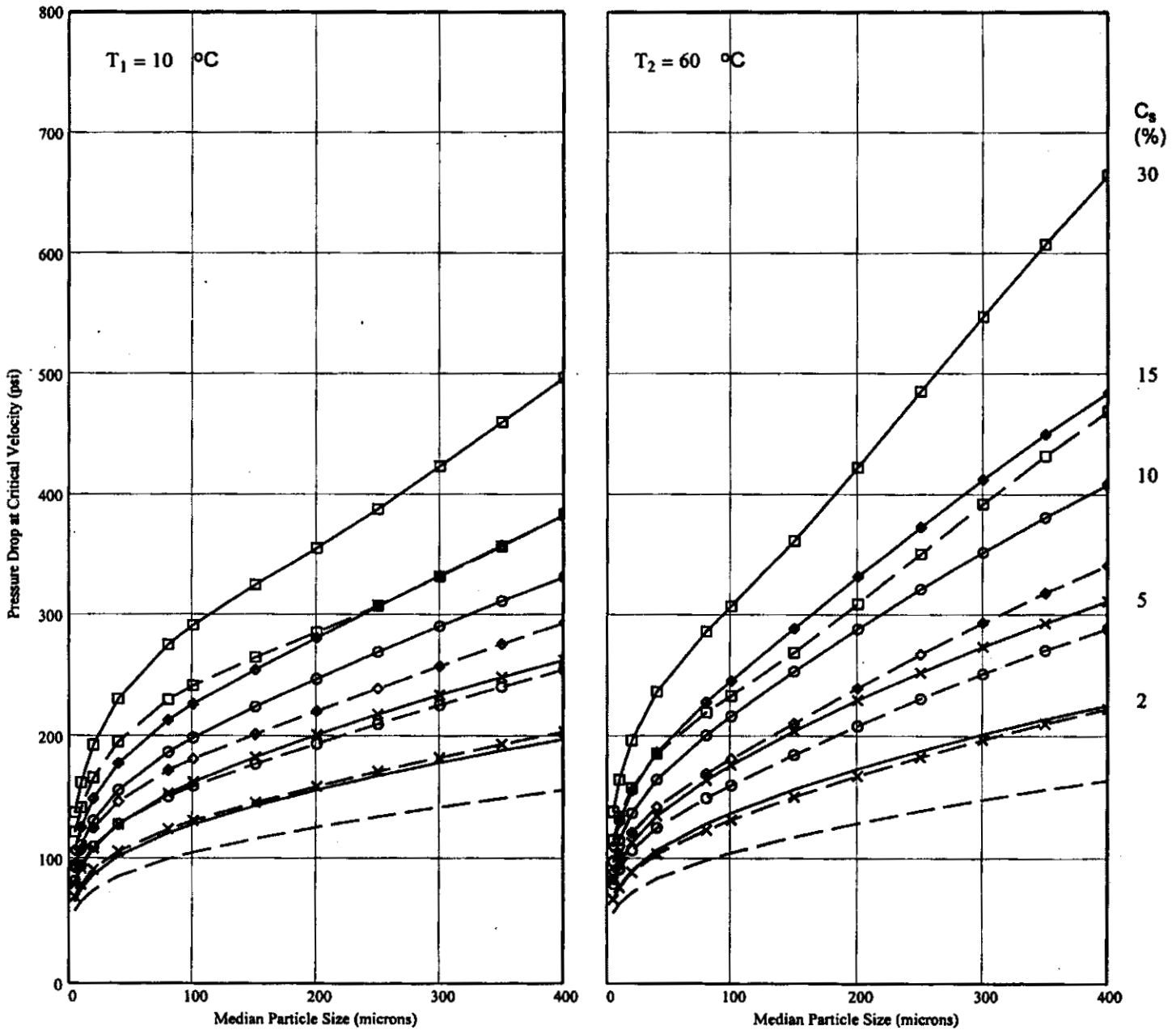


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$\kappa_f = 1$ $\beta_f = 1$ $D_{pipe} = 3.068 \text{ in}$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $\epsilon_{new} = 2 \text{ mil}$ $\epsilon_{EOL_CS} = 150 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L := 1.4 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 3 \frac{\text{kg}}{\text{liter}}$ $\mu_{cL}(\rho_L, T_1) = 5.401 \text{ cP}$ $\mu_{cL}(\rho_L, T_2) = 2.274 \text{ cP}$ $\epsilon_{EOL_SS} = 10 \text{ mil}$

Figure E-9a. Pressure Drop at Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.4 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$, $T = 10$ and 60°C , for New (dashed lines) and EOL (solid lines) Pipe Roughness Estimates.

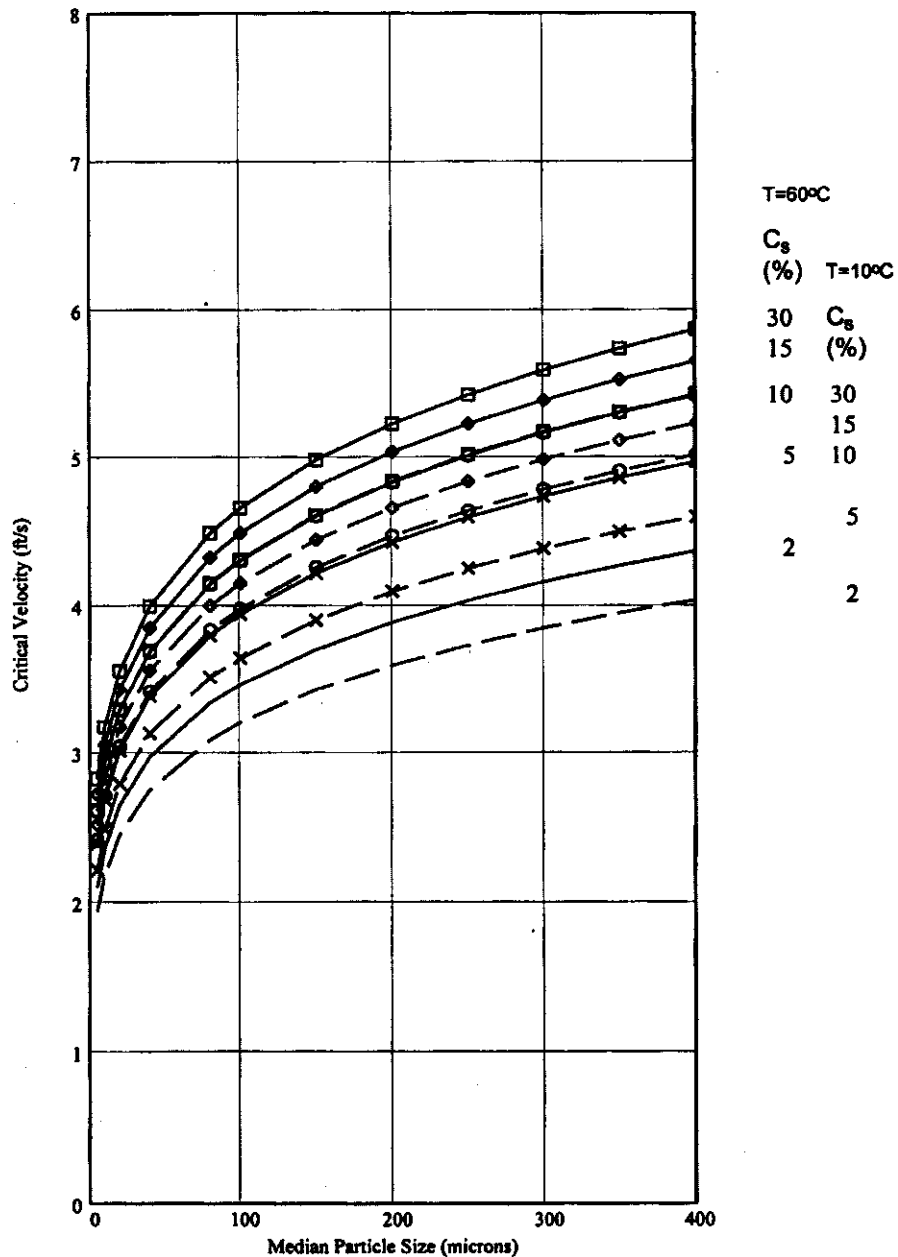


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 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L = 1.4 \frac{\text{kg}}{\text{liter}}$ $\rho_s = 3 \frac{\text{kg}}{\text{liter}}$ $\mu_{cL}(\rho_L, T_1) = 5.401 \text{ cP}$ $\mu_{cL}(\rho_L, T_2) = 2.274 \text{ cP}$ $\epsilon_{EOL_SS} = 10 \text{ mil}$

Figure E-9b. Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.4 \text{ kg/L}$, $\rho_s=3 \text{ kg/L}$, $T = 10$ (dashed lines) and 60°C (solid lines).



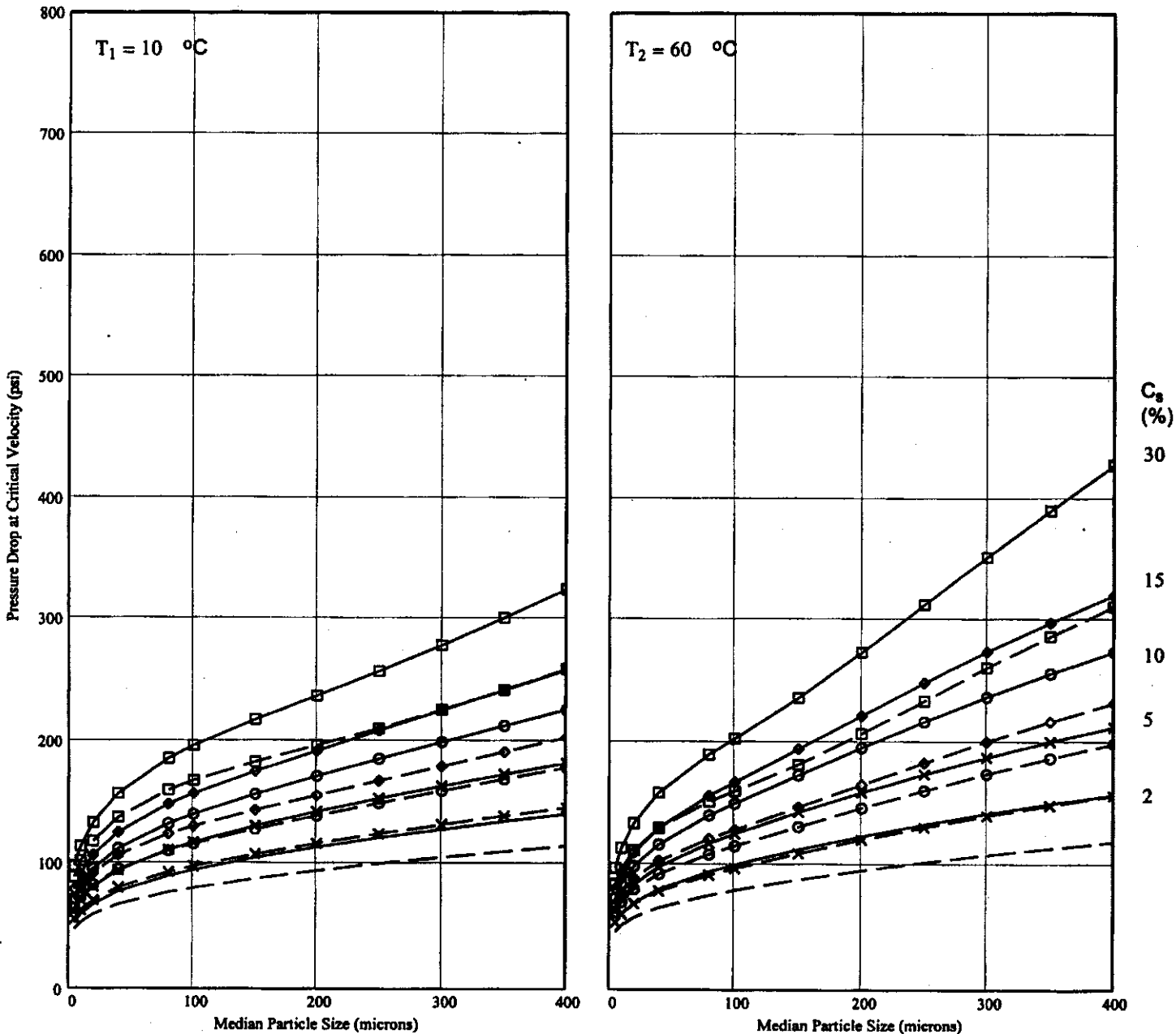
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$\kappa_f = 1$ $\beta_f = 1$ $D_{pipe} = 3.068 \text{ in}$ $L_{CS} = 557 \text{ ft}$ $L_{SS} = 7028 \text{ ft}$ $H = 40 \text{ ft}$ $\epsilon_{new} = 2 \text{ mil}$ $\epsilon_{EOL_{CS}} = 150 \text{ mil}$
 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L := 1.4 \frac{\text{kg}}{\text{liter}}$ $\rho_s := 2.5 \frac{\text{kg}}{\text{liter}}$ $\mu_{cL}(\rho_L, T_1) = 5.401 \text{ cP}$ $\mu_{cL}(\rho_L, T_2) = 2.274 \text{ cP}$ $\epsilon_{EOL_{SS}} = 10 \text{ mil}$

Figure E-10a. Pressure Drop at Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.4 \text{ kg/L}$, $\rho_s=2.5 \text{ kg/L}$, $T = 10$ and 60°C , for New (dashed lines) and EOL (solid lines) Pipe Roughness Estimates.

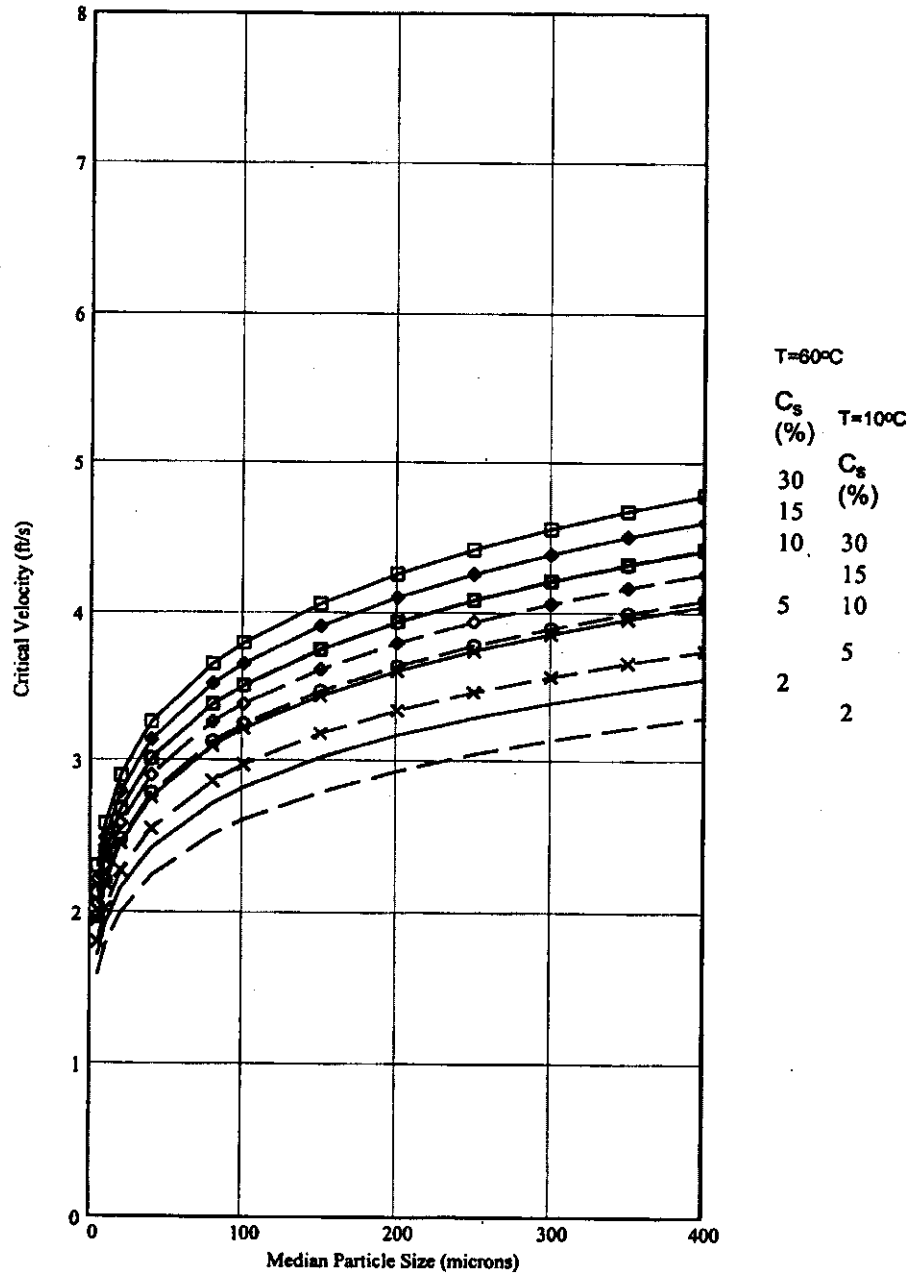


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$\kappa_f = 1$ $\beta_f = 1$ $D_{pipe} = 3.068$ in $L_{CS} = 557$ ft $L_{SS} = 7028$ ft $H = 40$ ft $\epsilon_{new} = 2$ mil $\epsilon_{EOL_{CS}} = 150$ mil
 $\kappa_{cv} = 1.3$ $\beta_{cv} = 1.2$ $\rho_L = 1.4 \frac{kg}{liter}$ $\rho_s = 2.5 \frac{kg}{liter}$ $\mu_{cL}(\rho_L, T_1) = 5.401$ cP $\mu_{cL}(\rho_L, T_2) = 2.274$ cP $\epsilon_{EOL_{SS}} = 10$ mil

Figure E-10b. Critical Velocity for AN-107-to-BNFL Transfer Route vs. Median Particle Size and Selected Solid Volume Concentrations - $\rho_L=1.4$ kg/L, $\rho_s=2.5$ kg/L, T = 10 (dashed lines) and 60°C (solid lines).



APPENDIX F

**CRITICAL VELOCITY AND PRESSURE
DROP CORRELATION EQUATIONS**

Calculations contained herein were produced in Mathcad 2000 Professional (Mathcad is a registered trademark of MathSoft, Inc. of Cambridge, Massachusetts).

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CONVERSIONS

ft	feet = 0.3048 meter (m)
in.	inch = 2.54 centimeters (cm)
lbf/in ²	pounds per square inch = 0.06895 megapascals (MPa)
mils	0.001 in. = 0.0254 millimeter (mm)
gpm	0.0631 liter/second (L/s)
cP	centipose = 0.001 kilogram/meter-second (kg/m-s)

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1.0 INTRODUCTION

Current pressure ratings of existing and proposed additions to the tank-to-tank, 3-inch diameter, Schedule-40, transfer pipelines for the double-shell tanks (DSTs) in the 200 East Area are 275 and 400 lbf/in². The 275 lbf/in² lines are carbon steel pipelines that were installed approximately 20 years ago. All new lines are expected to be stainless steel.

Corrosion of in-place Hanford waste tank transfer pipelines could restrict proposed waste feed activities. Corrosion of the inner pipe surface reduces the effective wall thickness necessary to maintain pipe structural integrity and increases the effective roughness of the pipe causing increased frictional resistance or pressure loss. The increased pressure loss requires greater pump capacity to achieve required flow. The flow rate must be maintained above a minimum flow rate to assure that solid particles in the waste stream do not settle in the pipeline, leading to line plugging. The maximum flow rate is restricted by the design pressure rating of the pipeline system.

Hence, increased corrosion leads to a reduction in the effective operating flow range of the pump and pipe system. An investigation of the proposed transfer conditions, pipeline configurations, and potential corrosion is necessary to evaluate pump requirements and adequacy of existing pipeline pressure ratings, as well as, requirements for any pipe additions.

2.0 APPROACH

Minimum flow rates (critical velocity) to maintain particle suspension and flow rate at the design pipeline pressure rating is estimated based on critical velocity and pipeline pressure drop correlations for slurry flow. Estey and Hu (1998) compared various critical velocity correlations and recommended the correlation developed by Oroskar and Turian (1980).

The pipeline pressure drop correlation for slurry flow developed by Wasp et al. (1977) is applied to obtain pressure drop estimates for each of the proposed transfer routes and waste conditions. New pipe and end-of-life (EOL) pipe roughness values for carbon and stainless steel pipe were applied from estimates given by Anantatmula and Divine (1999) to envelop the pressure drop estimates.

3.0 SUMMARY RESULTS

Although alternate critical velocity and pressure drop correlations were investigated, the Oroskar and Turian (1980) critical velocity and Wasp et al. (1977) pressure drop correlations were selected as being the most appropriate based on current limited knowledge of the waste physical properties during proposed transfers. Hanford waste feed delivery physical property data was estimated from the data summary provided in Estey 2000 for the in situ waste physical properties, as well as, from various waste characterization reports and flowsheets for Cases 3S4 and 3S6 from the Hanford Tank Waste Operation Simulator (HTWOS) model (Kirkbride et al. 1999). The range of waste and transfer pipe physical properties of interest are identified in Section 5.0 of this appendix. The assumed particle size distribution for the solid particles is based on the measured particle size distribution by volume for Tank 241-SY-101 (O'Rourke 1999). The measured distribution was curve fit to a Rosin-Rammler distribution function (Shook and Roco 1991) and then shifted to obtain a distribution with the target median particle size of interest. Appendix D provides a tabular listing of the critical velocity, pressure drop at critical velocity, and flow rate at a reference pipeline design pressure of 400 lbf/in² for each of the 200-East Area transfer routes identified in Cases 3S4 and 3S6 from the HTWOS model for a range of waste transfer properties. An alternate solution to reduce the predicted pressure drops for worst-case conditions was also considered. This alternate solution assumed a pump lift station in AP Farms located at the AP "new" valve pit. Selected long intermediate tank-to-tank transfers and all transfers to BNFL (Case 4-st) would go through the pump lift station. Appendix E provides the results of a "sensitivity" analysis on the longest transfer route (AN-107 to BNFL) for the range of waste transfer properties considered appropriate. See main body of this report for a discussion of the detail results. Appendix G provides results of Wasp model verification.

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5.0 HANFORD WASTE AND TRANSFER PIPE PHYSICAL PROPERTIES

TOL = 1×10^{-5}

Development of the expressions for the critical velocity and pressure drop correlations require knowledge of the following transfer pipe and waste physical property parameters.

Transfer pipeline parameters

D_{pipe} inside diameter of transfer pipeline $D_{pipe} := 3.068\text{-in}$
 P_{pipe_design} pipe design pressure $P_{pipe_design} := 400 \cdot \frac{\text{lbf}}{\text{in}^2}$ or $275 \cdot \frac{\text{lbf}}{\text{in}^2}$

The following roughness values are assumed for the 3-in. transfer pipe lines (Anantatmula and Divine 1999)

new pipe upper bound roughness	assumed corroded upper bound roughness	mil := $10^{-3} \cdot \text{in}$
$\epsilon_{new} := 2\text{-mil}$	$\epsilon_{EOL_CS} := 150\text{-mil}$	for carbon steel pipe at end of 50-year service life
$\epsilon_{new} = 2\text{ mil}$	$\epsilon_{EOL_SS} := 10\text{-mil}$	for stainless steel pipe at end of 40-year service life
$\frac{\epsilon_{new}}{D_{pipe}} = 0.00065$	$\frac{\epsilon_{EOL_CS}}{D_{pipe}} = 0.049$	$\frac{\epsilon_{EOL_SS}}{D_{pipe}} = 0.00326$
		corresponding relative roughness values for D_{pipe}

Waste transfer parameters

α_d or C_s	particulate solids volume fraction	$0 < \alpha_d \leq 0.30$	$\alpha_d < 0.10$ (typical)
$\alpha_c(\alpha_d) := (1 - \alpha_d)$	carrier liquid volume fraction		
D_d or d_p	particulate solid phase effective median diameter (μm)	$1 \leq D_d \leq 400$	$\mu\text{m} = 10^{-6} \cdot \text{m}$
ρ_d or ρ_s	particulate solid phase density (kg/L)	$1.4 \leq \rho_d \leq 4$	
ρ_c or ρ_L	carrier liquid phase density (kg/L)	$\rho_{water} \leq \rho_c \leq 1.5$	
μ_c or μ_L	carrier liquid phase viscosity (cP)	$\mu_{water}(t_c) \leq \mu_c \leq 10$	$\text{cP} = 10^{-2} \cdot \text{poise}$

The density and viscosity of water as a function of temperature ($^{\circ}\text{C}$) were obtained from Shook and Roco (1991) and Estey and Hu (1998) as

$$\rho_{water}(t_c) := 999.7 - 0.10512 \cdot (t_c - 10) - 0.005121 \cdot (t_c - 10)^2 + 0.00001329 \cdot (t_c - 10)^3 \cdot \frac{\text{kg}}{\text{m}^3} \quad \text{mass density of water} \quad \text{(F-1)}$$

$(5^{\circ}\text{C} < t_c < 100^{\circ}\text{C})$

viscosity of water as a function of temperature ($^{\circ}\text{C}$)

$$\mu_{water}(t_c) := \begin{cases} 100 \cdot \exp \left[\ln(10) \cdot \left[\frac{1301}{998.333 + 8.1855 \cdot (t_c - 20) + 0.00585 \cdot (t_c - 20)^2} - 3.30233 \right] \right] \cdot \text{cP} & \text{if } t_c \leq 20 \\ (1.002) \cdot \exp \left[\ln(10) \cdot \left[\frac{1.3272 \cdot (20 - t_c) - 0.001053 \cdot (t_c - 20)^2}{t_c + 105} \right] \right] \cdot \text{cP} & \text{otherwise} \end{cases} \quad \text{(F-2)}$$

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The above properties of water can be assumed for the pipeline flush condition as a function of the temperature of the flush water. These properties can vary if the flush water is treated.

The viscosity of water based on the above correlation is plotted in Figure F-1a as a function of temperature.

The viscosity of the carrier liquid is given in Estey 2000 by the following relation:

ρ_L density of carrier liquid with dissolved solids
 $x_{\text{salt}} := 0.9$ fraction of dissolved solids composed of sodium and other salts
 $x_{\text{caustic}} := 0.1$ fraction of dissolved solids composed of sodium hydroxide

$$\mu_{cL}(\rho_L, t_C) := \mu_{\text{water}}(t_C) \left[x_{\text{salt}} \left[1 + 1.071 \cdot \left(\frac{\rho_L}{\rho_{\text{water}}(t_C)} - 1 \right) \right] + x_{\text{caustic}} \exp \left[7.143 \cdot \left(\frac{\rho_L}{\rho_{\text{water}}(t_C)} - 1 \right)^{1.15} \right] \right] \quad (\text{F-3})$$

This relation is shown in Figure F-1b vs. the density of the carrier liquid at selected temperatures.

The following temperature conversion functions are defined to convert between Fahrenheit and Celsius temperature scales.

$$T_F(t_C) := \frac{9}{5} \cdot t_C + 32 \quad (\text{F-4})$$

$$T_C(t_F) := \frac{5}{9} \cdot (t_F - 32)$$

6.0 CRITICAL VELOCITY AND PRESSURE DROP CORRELATIONS

Empirical correlations for critical velocity (minimum slurry transport flow velocity to maintain suspension of solid particles) and pipeline friction pressure drop loss are available in the literature for various slurry types (homogeneous, heterogeneous, or compound homogeneous-heterogeneous) based on solid particle size and density, concentration of solids by weight or volume relative to total mass or volume of material being transported, carrier fluid density and viscosity, and pipe diameter and roughness. The slurry may exhibit Newtonian or non-Newtonian behavior. Non-Newtonian behavior is more likely at high concentrations (approaching 30% by volume) of fine particle solids (particle diameter less than approximately 100 microns). The slurry may be considered homogeneous if the solid particles do not settle out independent of the flow velocity. In the following only heterogeneous and compound homogeneous-heterogeneous Newtonian flow conditions are addressed. This restriction appears to be appropriate for the Hanford waste feed transfers being considered based on the limited sample data available at this time.

6.1 HETEROGENEOUS FLOW

6.1.1 Critical Velocity

Estey and Hu (1998) recommended that the Oroskar and Turian (1980) critical velocity empirical correlation be used for Hanford waste transfer conditions. The flow should be maintained above the critical velocity to prevent particles from settling in the pipe which could lead to pipe plugging. This correlation as well as alternate correlation's from the open literature are provided below for comparison. These alternate correlation's expand on

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the comparison given in Estey and Hu (1998). However, the Oroskar and Turian (1980) correlation is still selected for application to the Hanford waste transfer conditions as it is generally more conservative when compared to the alternate correlation's without direct experimental data for the Hanford waste.

The Oroskar and Turian (1980) critical velocity and alternate correlation's are given below.

The Reynolds Number for Newtonian flow is defined as

$$Re(D, v, \rho, \mu) := D \cdot v \cdot \frac{\rho}{\mu} \tag{F-5}$$

where D is a characteristic length, v is the flow velocity, ρ is the fluid density, and μ is the fluid viscosity.

The terminal settling velocity (v_{∞}) of spherical particle settling in a stagnant unbound liquid is given by the following:

For Stokes flow ($Re < 0.1$) the drag coefficient of a spherical particle is given by

$$C_D(D_d, v, \rho_c, \mu_c) := \frac{24}{Re(D_d, v, \rho_c, \mu_c)} \quad \text{which leads the following explicit relation for } v_{\infty} \tag{F-6}$$

$$v_{\infty}(D_d, \rho_d, \rho_c, \mu_c) := g \cdot (\rho_d - \rho_c) \cdot (D_d)^2 \cdot (18 \cdot \mu_c)^{-1} \tag{F-7}$$

For Stokes and non-Stokes flow Turian (1971) gives the following correlation for C_D valid for $Re < 1.5 \cdot 10^5$

$$C_D(D_d, v, \rho_c, \mu_c) := \left[\left(\frac{24}{Re(D_d, v, \rho_c, \mu_c)} \right)^{\frac{1}{2}} + 0.34035 \cdot \left(\frac{Re(D_d, v, \rho_c, \mu_c)^{0.06071} \dots}{1 + 1.72013 + 0.018 \cdot Re(D_d, v, \rho_c, \mu_c)} \right) \right]^2 \tag{F-8}$$

and v_{∞} is then given by $v_{\infty v}(D_d, \rho_d, \rho_c, \mu_c, v) := \sqrt{\frac{\frac{4}{3} \cdot D_d \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right) \cdot g}{C_D(D_d, v, \rho_c, \mu_c)}} \tag{F-9}$

which is implicit in v_{∞} but can be iteratively solved as follows $v' := 0.1 \cdot \frac{ft}{sec}$ initial guess

$$v_{\infty}(D_d, \rho_d, \rho_c, \mu_c) := \text{root}(v' - v_{\infty v}(D_d, \rho_d, \rho_c, \mu_c, v'), v') \tag{F-10}$$

To preclude an iterative solution Turian (1971) developed the following approximate correlation for v_{∞} with the introduction of the following non dimensional parameter

$$\Lambda(D_d, \rho_d, \rho_c, \mu_c) := \sqrt{\frac{4}{3} \cdot \frac{g \cdot D_d^3 \cdot \rho_c \cdot (\rho_d - \rho_c)}{\mu_c^2}} \quad \text{with} \tag{F-11}$$

$$\lambda(\Lambda) := -1.38 + 1.94 \cdot \log(\Lambda) - 8.6 \cdot 10^{-2} \cdot \log(\Lambda)^2 - 2.52 \cdot 10^{-2} \cdot \log(\Lambda)^3 + 9.19 \cdot 10^{-4} \cdot \log(\Lambda)^4 + 5.35 \cdot 10^{-4} \cdot \log(\Lambda)^5 \tag{F-12}$$

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to give the terminal settling velocity as

$$v_{\infty a}(D_d, \rho_d, \rho_c, \mu_c) := \frac{\mu_c}{D_d \cdot \rho_c} \cdot 10^{\lambda(D_d, \rho_d, \rho_c, \mu_c)} \quad (\text{F-13})$$

Reynolds Number for a settling spherical particle at terminal velocity is given by

$$Re_{\infty}(D_d, \rho_d, \rho_c, \mu_c) := Re(D_d, v_{\infty}(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c) \quad (\text{F-14})$$

The above drag coefficients correlations, as well as a third correlation referenced in Estey and Hu (1998), are compared in Figure F-2. Equations F-8 through F-10 are used herein for greater accuracy over a full range of particle sizes.

In accordance with Oroskar and Turian (1980), the hindered settling velocity of a spherical particle is given by the following relations:

$$n = 4.65 - 2.32 \cdot \frac{1}{\sqrt{0.5 \cdot \pi}} \cdot \int_{-\infty}^{\infty} \exp\left(\frac{-\log(Re_{\infty})^2}{0.5}\right) d\log(Re_{\infty}) \quad (\text{F-15})$$

$$n(D_d, \rho_d, \rho_c, \mu_c) := (4.65 - 2.32 \cdot \text{pnorm}(\log(Re_{\infty}(D_d, \rho_d, \rho_c, \mu_c)), 0, 0.5)) \quad (\text{F-16})$$

$$v_{\text{hindered}}(D_d, \rho_d, \alpha_d, \rho_c, \mu_c) := (\alpha_c(\alpha_d))^{n(D_d, \rho_d, \rho_c, \mu_c)} \cdot v_{\infty}(D_d, \rho_d, \rho_c, \mu_c) \quad \text{hindered settling velocity} \quad (\text{F-17})$$

A plot of the hindered settling velocity exponent versus Reynolds Number is shown in Figure F-3 for comparison with Figure 5 given in Oroskar and Turian (1980). Equations F-15 and F-16 were derived by Estey and Hu (1998) as an analytical approximation to the graphical relation given in Figure 5 of Oroskar and Turian (1980).

The ratio of particle hindered settling velocity to critical velocity

$$\gamma = \frac{v_{\text{hindered}}}{v_{\text{critical}}} \quad \text{or} \quad \gamma(D_d, \rho_d, \alpha_d, \rho_c, \mu_c, v) := \frac{v_{\text{hindered}}(D_d, \rho_d, \alpha_d, \rho_c, \mu_c)}{v} \quad (\text{F-18})$$

The fraction of the turbulent fluid eddies possessing a velocity larger than the particulate settling velocity is given (in corrected form derived from Equation 42 of Oroskar and Turian 1980) as

$$\chi(\gamma) := \frac{2}{\sqrt{\pi}} \left[\frac{2}{\sqrt{\pi}} \cdot \gamma \cdot \exp\left(\frac{-4}{\pi} \cdot \gamma^2\right) + \frac{\sqrt{\pi}}{2} \cdot \left(1 - \text{erf}\left(\frac{2}{\sqrt{\pi}} \cdot \gamma\right)\right) \right] \quad (\text{F-19})$$

χ versus γ is plotted in Figure F-4 for comparison with Figure 6 in Oroskar and Turian (1980).

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The Oroskar and Turian (1980) semi-theoretical correlation for the critical velocity (v_t) is given by:

$$v_d(D_d, \rho_d, \rho_c) := \sqrt{g \cdot D_d \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)} \quad \text{characteristic velocity of a settling particle} \quad (\text{F-20})$$

$$F_1(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu) := \left[\frac{5 \cdot (\alpha_d) \cdot (\alpha_c(\alpha_d))^{(2 \cdot n(D_d, \rho_d, \rho_c, \mu_c) - 1)} \cdot \left(\frac{D_{\text{pipe}}}{D_d} \right) \cdot \left(\frac{D_{\text{pipe}} \cdot \rho_c}{\mu_c} \cdot v_d(D_d, \rho_d, \rho_c) \right)^{\frac{1}{8}}}{\chi(\gamma(D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu))} \right]^{\frac{8}{15}} \quad (\text{F-21})$$

$$v_{tv}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu) := v_d(D_d, \rho_d, \rho_c) \cdot F_1(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu) \quad (\text{F-22})$$

$$v_{\text{crOTt}}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) := \kappa_{cv} \cdot \text{root}(v - v_{tv}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu), v) \quad \text{semi-theoretical critical velocity correlation} \quad (\text{F-23})$$

where D_{pipe} is the inside diameter of pipe and the other parameters are as defined above.

The Oroskar and Turian (1980) empirical correlation for the critical velocity (v_e) is given by:

$$F_2(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c) := 1.85 \cdot (\alpha_d)^{0.1536} \cdot (\alpha_c(\alpha_d))^{0.3564} \cdot \left(\frac{D_{\text{pipe}}}{D_d} \right)^{0.378} \cdot \left(D_{\text{pipe}} \cdot \frac{\rho_c}{\mu_c} \cdot v_d(D_d, \rho_d, \rho_c) \right)^{0.09} \quad (\text{F-24})$$

$$v_{ev}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu) := v_d(D_d, \rho_d, \rho_c) \cdot F_2(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c) \cdot \chi(\gamma(D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu))^{0.30} \quad (\text{F-25})$$

$$v_{\text{crOTe}}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) := \kappa_{cv} \cdot \text{root}(v - v_{ev}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \nu), v) \quad \text{empirical critical velocity correlation} \quad (\text{F-26})$$

The Oroskar and Turian (1980) semi-theoretical and empirical equations given above were considered to do the best overall job of predicting the critical velocity. The overall percent rms deviation was 25.94 and 21.82, respectively compared to at least twice this for the other seven correlations considered.

Estey and Hsu (1998) recommended that the slurry flow critical velocity be taken as the maximum value of the Oroskar and Turian (1980) semi-theoretical and empirical equations, i.e.,

$$v_{\text{crOT}}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) := \frac{m}{\text{sec}} \cdot \begin{cases} u_t \leftarrow v_{\text{crOTt}}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \cdot \left(\frac{m}{\text{sec}} \right)^{-1} \\ u_e \leftarrow v_{\text{crOTe}}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \cdot \left(\frac{m}{\text{sec}} \right)^{-1} \\ u_t \text{ if } u_t > u_e \\ u_e \text{ otherwise} \end{cases} \quad (\text{F-27})$$

κ_{cv} is the uncertainty factor on the predicted critical velocity, set to 1 to obtain "best-estimate" value.

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Turian, Hsu, and Ma (1987) compared 36 critical velocity correlations. Correlations proposed by Robinson and Graf (1972), Oroskar and Turian (1980), and by Wasp et al. (1977) were determined to do a better job of predicting the critical velocity than the other correlations considered. The relations by Robinson and Graf (1972) and Wasp et al. (1977) are given, respectively as

$$v_{crRG}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) := \kappa_{cv} \cdot 0.901 \cdot \alpha_d^{0.106} \cdot \sqrt{2 \cdot g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)} \quad (F-28)$$

$$v_{crWasp}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) := \kappa_{cv} \cdot 3.116 \cdot \alpha_d^{0.186} \cdot \left(\frac{D_d}{D_{pipe}} \right)^{\frac{1}{6}} \cdot \sqrt{2 \cdot g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)} \quad (F-29)$$

Also of interest are the following correlations.

Zandi and Govatos (1967) proposed a critical velocity relation developed from more than 1000 data points collected from 11 references. The critical velocity at transition from saltation to heterogeneous flow was given as

$$v_{crZG}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) := \kappa_{cv} \cdot \sqrt{\frac{40 \cdot \alpha_d \cdot g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)}{C_D(D_d, v_{\infty}(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)}} \quad (F-30)$$

Shook (1969) used the Durand's pressure loss correlation and determined the velocity corresponding to minimum pressure loss and proposed the following relation which is similar to the Zandi and Govatos (1967) relation.

$$v_{crShook}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) := \kappa_{cv} \cdot 2.43 \cdot \alpha_d^{\frac{1}{3}} \cdot \sqrt{\frac{2 \cdot g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)}{C_D(D_d, v_{\infty}(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)}} \quad (F-31)$$

Gogus and Kokpinar (1999) developed the following empirical correlation based on experimental data for the ranges $230 \mu\text{m} < D_d < 5430 \mu\text{m}$, $1.04 \text{ kg/liter} < \rho_d < 2.68 \text{ kg/liter}$, $0.0254 \text{ m} < D_{pipe} < 0.1524 \text{ m}$, and $0.0075 < \alpha_d < 0.30$.

$$\mu_c(\mu_L, \alpha_d) := \mu_L \cdot \left(1 + 2.5 \cdot \alpha_d + 10.05 \cdot \alpha_d^2 + 0.00273 \cdot \exp(16.6 \cdot \alpha_d) \right) \quad \text{Thomas correlation (Wasp et al. 1977)} \quad (F-32)$$

$$\rho_c(\rho_s, \rho_L, \alpha_d) := \rho_s \cdot \alpha_d + \rho_L \cdot (1 - \alpha_d)$$

$$v_{\infty}(D_d, \rho_s, \rho_L, \mu_L, \alpha_d) := v_{\infty}(D_d, \rho_s, \rho_c(\rho_s, \rho_L, \alpha_d), \mu_c(\mu_L, \alpha_d)) \quad \text{particle settling velocity in slurry mixture}$$

$$v_{crGK}(D_{pipe}, D_d, \rho_s, \alpha_d, \rho_L, \mu_L, \kappa_{cv}) := \kappa_{cv} \cdot 0.0548 \cdot \left(\frac{D_d}{D_{pipe}} \right)^{-0.60} \cdot \alpha_d^{0.27} \cdot \left(\frac{\rho_s}{\rho_L} - 1 \right)^{0.07} \cdot \left(\frac{\rho_L \cdot v_{\infty}(D_d, \rho_s, \rho_L, \mu_L, \alpha_d) \cdot D_d}{\mu_L} \right)^{0.30} \cdot \sqrt{g \cdot D_{pipe}} \quad (F-33)$$

The authors claim only 15% average error in this prediction equation.

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Figure F-5 indicates that the Oroskar and Turian (1980) empirical equation generally predicts a higher critical velocity than the Oroskar and Turian (1980) semi-theoretical equation. The sensitivity of the Oroskar and Turian (1980) empirical equation to the liquid density, liquid viscosity, particle solids density, particle solids diameter, and solids volume fraction are shown in Figures F-6 through F-10 for solid-liquid flow in a 3-inch diameter pipe.

The following relation provides easy access to each of the above correlations and assures that the critical velocity returned is greater than the transition to turbulence velocity

$$v_{cr}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv}) := \frac{m}{sec} \cdot \left\{ \begin{array}{l} v_T \leftarrow \frac{4000 \cdot \mu_c}{D_{pipe} \cdot \rho_c} \quad \text{transition to turbulence velocity} \\ v_{crit} \leftarrow v_T \quad \text{if } \alpha_d < 0.0001 \\ \text{otherwise} \\ \left\{ \begin{array}{l} v_{crit} \leftarrow v_{crOT}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 1 \\ v_{crit} \leftarrow v_{crOT1}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 1.1 \\ v_{crit} \leftarrow v_{crOTe}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 1.2 \\ v_{crit} \leftarrow v_{crWasp}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 2 \\ v_{crit} \leftarrow v_{crZG}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 3 \\ v_{crit} \leftarrow v_{crShook}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 4 \\ v_{crit} \leftarrow v_{crGK}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 5 \\ v_{crit} \leftarrow v_{crRG}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}) \quad \text{if } \beta_{cv} = 6 \end{array} \right. \\ v_T \quad \text{if } v_T > v_{crit} \\ v_{crit} \quad \text{otherwise} \end{array} \right. \quad (F-34)$$

The critical velocity correlation selection key for equation F-34 is given as follows:

- | | |
|--|---|
| <ul style="list-style-type: none"> 1 Oroskar and Turian (1980) maximum of semi-theoretical and empirical equation 1.1 Oroskar and Turian (1980) semi-theoretical equation 1.2 Oroskar and Turian (1980) empirical equation $\beta_{cv} = 2$ Wasp et al. (1977) 3 Zandi and Govatos (1967) 4 Shook (1969) 5 Gogus and Kokpinar (1999) 6 Robinson and Graf (1972) | <p>κ_{cv} is the uncertainty factor on the critical, set to 1 to obtain the "best-estimate" prediction. Recommend at least 1.3 for upper bound estimate to account for uncertainties in the empirical correlation and to include sufficient margin above the critical velocity for minimum operating flow rate.</p> |
|--|---|

Figures F-6 through F-10 show the sensitivity of the slurry parameters on the Oroskar and Turian (1980) empirical equation for the critical velocity best-estimate prediction. The other correlations are compared to the Oroskar and Turian (1980) semi-theoretical and empirical equations in Figure F-16 along with the Turian, Hsu, and Ma (1987) regime transition correlation discussed in Section 6.1.2.2 below.

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6.1.2 Friction Factors

In addition to pipe roughness, the friction factor for pipe flow is affected by the physical properties of the carrier liquid and the particles in suspension.

6.1.2.1 Friction Factor for Pipe Flow Due to Carrier Liquid

Slurry volume entirely liquid, $\alpha_d = 0$.

Colebrook-White correlation for the Darcy or Moody friction factor (f) in the turbulent regime is given by

$$\frac{1}{\sqrt{f}} = -2 \cdot \log \left(\frac{\eta}{3.7} + \frac{2.51}{Re \cdot \sqrt{f}} \right) \quad \text{strictly valid for} \quad 4 \cdot 10^3 \leq Re \leq 10^8 \quad \text{where} \quad f = \frac{h_L}{L \cdot v^2} = \frac{\Delta p}{\left(\frac{L}{D} \cdot \frac{\rho \cdot v^2}{2} \right)} \quad \eta = \frac{\epsilon}{D_{\text{pipe}}}$$

ϵ roughness of pipe

This relation is implicit in the friction factor but Olujic (1981) obtained the following explicit approximation

$$f_t(Re, \eta) := \left[-2 \cdot \log \left[\frac{\eta}{3.7} - \frac{5.02}{Re} \cdot \log \left[\frac{\eta}{3.7} + \left(\frac{14.5}{Re} \right) \right] \right] \right]^{-2} \quad \text{turbulent regime,} \quad Re > 4000 \quad (F-35)$$

For the laminar regime, $Re < 2100$

$$f_l(Re) := \frac{64}{Re} \quad (\text{independent of roughness}) \quad (F-36)$$

Olujic (1981) also reports the following approximation developed by Churchill in 1977 that includes the laminar and turbulent regimes as well as the transition regime between laminar and turbulent flow

$$f_c(Re, \eta) := \left[\begin{array}{l} A \leftarrow \left[2.457 \cdot \ln \left[\frac{1}{\left(\frac{7}{Re} \right)^{0.9} + (0.27 \cdot \eta)} \right] \right]^{16} \\ B \leftarrow \left(\frac{37530}{Re} \right)^{16} \\ 8 \cdot \left[\left(\frac{8}{Re} \right)^{12} + \left(\frac{1}{A+B} \right)^{\frac{3}{2}} \right]^{12} \end{array} \right]^{-1} \quad (F-37)$$

(Note that the transition regime is not well defined in practice but is approximated with a "smooth" transition curve for numerical convenience only. This approximation slightly over shoots the Colebrook-White friction factor correlation at the start of the turbulent regime.)

or $f_c(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon) := f_c \left(Re(D_{\text{pipe}}, v_c, \rho_c, \mu_c), \frac{\epsilon}{D_{\text{pipe}}} \right)$

where v_c flow velocity of carrier liquid

Figure 11 shows the resulting Darcy friction factor for pipe flow of Newtonian carrier fluid as a function of Reynolds Number and relative roughness ($\eta = \epsilon / D_{\text{pipe}}$).

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6.1.2.2 Friction Factor for Pipe Flow Due to Particles

A refined correlation of the particle contribution to the friction factor was given by Turian and Yuan (1977) and Turian, Hsu, and Ma (1987) as a function of flow regime based on the Fanning friction factor definition. The Darcy friction factor is equal to 4 times the Fanning friction factor. The corresponding results in terms of the Darcy friction factor definition become

Turian, Hsu, and Ma (1987)

Turian and Yuan (1977)

s =	$K_s :=$	$\alpha_s :=$	$\beta_s :=$	$\gamma_s :=$	$\delta_s :=$		$K'_s :=$
0	12.127	0.7389	0.7717	-0.4054	-1.096	flow regime 0 (stationary bed)	0.4036
1	107.09	1.018	1.046	-0.4213	-1.354	flow regime 1 (saltation)	0.9857
2	30.115	0.8687	1.2	-0.1677	-0.6938	flow regime 2 (heterogeneous)	0.5513
3	8.538	0.5024	1.428	0.1516	-0.3531	flow regime 3 (homogeneous)	0.8444

$$f_{dm}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, m, \beta_f) := 4 \cdot \frac{\text{if}(\beta_f = 1, K_m, K'_m) \cdot \alpha_d \cdot \left(\frac{f_c(D_{pipe}, v, \rho_c, \mu_c, \epsilon)}{4} \right)^{\beta_m}}{C_D(D_d, v_{\infty}(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)^{-\gamma_m}} \left[\frac{v^2}{g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)} \right]^{\delta_m} \quad (F-38)$$

$$K_t(a, b) := \left(\frac{K_b}{K_a} \right)^{\frac{1}{\delta_a - \delta_b}} \quad \alpha_t(a, b) := \frac{\alpha_b - \alpha_a}{\delta_a - \delta_b} \quad \beta_t(a, b) := \frac{\beta_b - \beta_a}{\delta_a - \delta_b} \quad \gamma_t(a, b) := \frac{\gamma_b - \gamma_a}{\delta_a - \delta_b} \quad K'_t(a, b) := \left(\frac{K'_b}{K'_a} \right)^{\frac{1}{\delta_a - \delta_b}}$$

$K_t(0, 1) = 4641.4$	$\alpha_t(0, 1) = 1.0818$	$\beta_t(0, 1) = 1.0632$	$\gamma_t(0, 1) = -0.0616$	$K'_t(0, 1) = 31.8$
$K_t(1, 2) = 6.8319$	$\alpha_t(1, 2) = 0.2261$	$\beta_t(1, 2) = -0.2333$	$\gamma_t(1, 2) = -0.3841$	$K'_t(1, 2) = 2.4113$
$K_t(2, 3) = 40.4362$	$\alpha_t(2, 3) = 1.0751$	$\beta_t(2, 3) = -0.6692$	$\gamma_t(2, 3) = -0.9372$	$K'_t(2, 3) = 0.2861$
$K_t(1, 3) = 12.5143$	$\alpha_t(1, 3) = 0.5151$	$\beta_t(1, 3) = -0.3817$	$\gamma_t(1, 3) = -0.5724$	$K'_t(1, 3) = 1.1672$
$K_t(0, 2) = 0.1042$	$\alpha_t(0, 2) = -0.3227$	$\beta_t(0, 2) = -1.0649$	$\gamma_t(0, 2) = -0.591$	$K'_t(0, 2) = 0.4605$
$K_t(0, 3) = 1.6038$	$\alpha_t(0, 3) = 0.3183$	$\beta_t(0, 3) = -0.8834$	$\gamma_t(0, 3) = -0.7498$	$K'_t(0, 3) = 0.3702$

Flow regime numbers for selecting appropriate flow regime is given by

$$R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, a, b, \beta_f) := \frac{C_D(D_d, v_{\infty}(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)^{-\gamma_t(a, b)}}{\text{if}(\beta_f = 1, K_t(a, b), K'_t(a, b)) \cdot \alpha_d \cdot \left(\frac{f_c(D_{pipe}, v, \rho_c, \mu_c, \epsilon)}{4} \right)^{\beta_t(a, b)}} \cdot \frac{v^2}{g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)} \quad (F-39)$$

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The flow regime number retrieval function is given by

$$R_d(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, \beta_f) := \begin{cases} r_{01} \leftarrow R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 1, \beta_f) - 1 \\ r_{12} \leftarrow R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 1, 2, \beta_f) - 1 \\ r_{23} \leftarrow R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 2, 3, \beta_f) - 1 \\ r_{02} \leftarrow R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 2, \beta_f) - 1 \\ r_{03} \leftarrow R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 3, \beta_f) - 1 \\ r_{13} \leftarrow R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 1, 3, \beta_f) - 1 \\ 0 \text{ if } (r_{01} < 0) \cdot (r_{12} < 0) \cdot (r_{23} < 0) \\ 1 \text{ if } (r_{01} > 0) \cdot (r_{12} < 0) \cdot (r_{23} < 0) \\ 2 \text{ if } (r_{01} > 0) \cdot (r_{12} > 0) \cdot (r_{23} < 0) \\ 3 \text{ if } (r_{01} > 0) \cdot (r_{12} > 0) \cdot (r_{23} > 0) \\ \text{if } (r_{01} < 0) \cdot (r_{12} < 0) \cdot (r_{23} > 0) \\ \quad \begin{cases} 0 \text{ if } r_{03} < 0 \\ 3 \text{ otherwise} \end{cases} \\ \text{if } (r_{01} < 0) \cdot (r_{12} > 0) \cdot (r_{23} > 0) \\ \quad \begin{cases} 0 \text{ if } r_{03} < 0 \\ 3 \text{ otherwise} \end{cases} \\ \text{if } (r_{01} < 0) \cdot (r_{12} > 0) \cdot (r_{23} < 0) \\ \quad \begin{cases} 0 \text{ if } r_{02} < 0 \\ 2 \text{ otherwise} \end{cases} \\ \text{if } (r_{01} > 0) \cdot (r_{12} < 0) \cdot (r_{23} > 0) \\ \quad \begin{cases} 1 \text{ if } r_{13} < 0 \\ 3 \text{ otherwise} \end{cases} \\ -1 \text{ otherwise} \end{cases}$$

(F-40)

The added particle contribution to the friction factor based on flow regime is then given by

$$f_{dTurian}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, \kappa_f, \beta_f) := \kappa_f f_{dm}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, R_d(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, \beta_f), \beta_f)$$

(F-41)

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Critical velocity based on flow regime transition identification is given by

$$v_{cRab}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \epsilon, a, b, v, \beta_f) := \left[\frac{\text{if}(\beta_f = 1, K_r(a, b), K'_r(a, b)) \cdot \alpha_d^{\alpha_r(a, b)} \cdot \left(\frac{f_c(D_{pipe}, v, \rho_c, \mu_c, \epsilon)}{4} \right)^{\beta_r(a, b)} \cdot g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_c} - 1 \right)}{C_D(D_d, v_\infty(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)^{-\gamma_r(a, b)}} \right]^{\frac{1}{2}}$$

$$v_{cRab}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \epsilon, a, b, \beta_f) := \text{root}(v' - v_{cRab}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \epsilon, a, b, v', \beta_f), v') \quad (F-42)$$

$$v_{crR}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \epsilon, \kappa_{cv}, \beta_f) := \kappa_{cv} \cdot \frac{m}{sec} \cdot \begin{cases} v_{01} \leftarrow v_{cRab}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \epsilon, 0, 1, \beta_f) \\ v_{02} \leftarrow v_{cRab}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \epsilon, 0, 2, \beta_f) \\ v_{03} \leftarrow v_{cRab}(D_{pipe}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \epsilon, 0, 3, \beta_f) \\ R_{02_01} \leftarrow R_{ab}(D_{pipe}, v_{01}, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 2, \beta_f) \\ R_{03_01} \leftarrow R_{ab}(D_{pipe}, v_{01}, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 3, \beta_f) \\ R_{01_02} \leftarrow R_{ab}(D_{pipe}, v_{02}, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 1, \beta_f) \\ R_{03_02} \leftarrow R_{ab}(D_{pipe}, v_{02}, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 3, \beta_f) \\ R_{01_03} \leftarrow R_{ab}(D_{pipe}, v_{03}, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 1, \beta_f) \\ R_{02_03} \leftarrow R_{ab}(D_{pipe}, v_{03}, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 0, 2, \beta_f) \\ v_{01} \text{ if } (R_{02_01} \leq 1) \cdot (R_{03_01} \leq 1) \\ v_{02} \text{ if } (R_{01_02} \leq 1) \cdot (R_{03_02} \leq 1) \\ v_{03} \text{ if } (R_{01_03} \leq 1) \cdot (R_{02_03} \leq 1) \\ -1 \text{ otherwise} \end{cases} \quad (F-43)$$

The following relation is used to plot flow regime (v-D_d) diagrams as shown in Figures 12 through 14.

$$D_{dab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, \rho_d, \alpha_d, a, b, \beta_f) := -1 \cdot \mu m \text{ on error root}(R_{ab}(D_{pipe}, v, \rho_c, \mu_c, \epsilon, D'_d, \rho_d, \alpha_d, a, b, \beta_f) - 1, D'_d) \quad (F-44)$$

Turian and Oroskar's (1978) Figure 1 flow regime diagrams for pipe inside diameters 1.58 and 5.25 cm (1/2 and 2 in.) based on Turian, Hsu, and Ma 1987 corrected equations are given in Figures 12 and 13 as a check and also demonstrate the effect of pipe size on the flow regime locations. A smooth pipe with water at 22.5 °C for the carrier liquid is assumed with a 5% solids concentration by volume with a 2.977 gm/cm³ particle solids density. The flow regime diagrams for the 1.58 and 5.25 cm inside pipe diameters are shown in Figures 12 and 13, respectively. Flow regime diagrams for the 3-inch diameter pipeline of interest are shown in Figure 14 for the conditions indicated.

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Waste Tank Feed Transfer System
 Location: 200 Area - Hanford Site, Richland, Washington

WO/Job No. 110299 & 110300/BA10
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Figure 15 compares the best-estimate critical velocity predictions from Robinson and Graf (1972), Wasp et al. (1977), Oroskar and Turian (1980) semi-theoretical and empirical equations, and the critical velocity derived from the regime transition boundaries defined in Turian and Yuan (1977) for a smooth pipe with corrected relations given in Turian, Hsu, and Ma (1987).

Note that the critical velocity correlations compared in Figure 15 were obtained for narrow band distributions of median diameter solid particles greater than 100 μm . Hence, the results shown in Figure 15 for solid particles less than 100 μm requires extrapolation and should be used with caution. In addition, it must be recognized that the critical velocity data is based on subjective observations and hence introduce uncertainty in the data upon which the correlations were developed. These correlations were developed for solid particles with a narrow distributions. The effect of solid particles with a wide distribution needs to be addressed.

For fine particles (diameter much less than 100 μm) the two-phase approach to determining pressure loss and minimum settling (critical) velocity begins to break down. In this case the liquid-solid flow approaches a homogeneous type behavior in which the fine particle remain suspended but the mixture may exhibit non-Newtonian shear characteristics.

The following additional friction factor correlations ($f - f_c = f_d$) based on large particles are provided for comparison:

Durand (1953) friction factor correlation per equation 11.84a as reported in Grovier and Aziz 1972

$$f_{d\text{Durand}}(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, \kappa_f, K_D) := \kappa_f K_D \alpha_d f_c(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon) \cdot \left[\frac{v^2 \sqrt{C_D(D_d, v_\infty(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)}}{g \cdot D_{\text{pipe}} \left(\frac{\rho_d}{\rho_c} - 1 \right)} \right]^{\frac{-3}{2}} \quad (\text{F-45})$$

Note that Grovier and Aziz (1972) indicate that the abscissa of Figure 10 in the paper by Durand (1953) appears to be mislabeled, and others appear to have erroneously determined from it the value of the constant (K_D) as either 81, 82, or 84.9 rather than 150.

Zandi and Govatos (1967) friction factor correlation per equation 11.85 of Grovier and Aziz 1972

(valid for $N_t = \psi / \alpha_d > 40$)

$$f_{d\text{Zandi}}(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, \kappa_f) := \kappa_f \alpha_d f_c(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon) \cdot \begin{cases} \psi \left[\frac{v^2 \sqrt{C_D(D_d, v_\infty(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)}}{g \cdot D_{\text{pipe}} \left(\frac{\rho_d}{\rho_c} - 1 \right)} \right] & \text{if } \psi > 40 \\ 280 \cdot \psi^{-1.93} & \text{if } \psi < 10 \\ 6.30 \cdot \psi^{-0.354} & \text{otherwise} \end{cases} \quad (\text{F-46})$$

Heywood (1999) refers to a "recent" reassessment of all available data covering both heterogeneous and moving-bed flow patterns which suggested the following correlation:

$$f_{d\text{Heywood}}(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, \kappa_f) := \kappa_f 55.6 \cdot f_c(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon) \cdot \left[\frac{v^2 \sqrt{C_D(D_d, v_\infty(D_d, \rho_d, \rho_c, \mu_c), \rho_c, \mu_c)}}{\alpha_d \cdot g \cdot D_{\text{pipe}} \left(\frac{\rho_d}{\rho_c} - 1 \right)} \right]^{-1.08} \quad (\text{F-47})$$

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The above Durand (1953) and Zandi and Govatos (1967) Darcy friction factor correlations for two-phase liquid-solid particle type pipe flow are compared in Figure 16 to the Turian and Yuan (1977) correlation with the Turian, Hsu, and Ma (1987) correction. The correlations give similar results but the Turian correlation was selected for application because of its purported overall better performance over the full range of flow velocities over each flow regime. However, it is clear that there is a large uncertainty band for any of these correlations.

The combined friction factor for pipe with positive incline to horizontal of θ (deg) per Blevins (1984) is given by

$$f_d(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \beta_f, \kappa_f, \theta) := \begin{cases} f_{d\text{Turian}}(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \kappa_f, \beta_f) \cdot \cos(\theta) & \text{if } (\beta_f = 1) + (\beta_f = 2) \\ f_{d\text{Zandi}}(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \kappa_f) \cdot \cos(\theta) & \text{if } \beta_f = 3 \\ f_{d\text{Heywood}}(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \kappa_f) \cdot \cos(\theta) & \text{if } \beta_f = 4 \\ f_{d\text{Durand}}(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \kappa_f, \beta_f) \cdot \cos(\theta) & \text{otherwise} \end{cases} \quad (\text{F-48a})$$

$$f(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \beta_f, \kappa_f, \theta) := \begin{cases} f \leftarrow f_c(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon) \\ f & \text{if } (\beta_f = 0) + (\alpha_d = 0) \\ f + f_d(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \beta_f, \kappa_f, \theta) & \text{otherwise} \end{cases} \quad (\text{F-48b})$$

The friction factor correlation selection key is given by

0 liquid only, no solid particles, $\alpha_d = 0$
 1 Turian, Hsu, and Ma (1987)
 $\beta_f = 2$ Turian and Yuan (1977)
 3 Zandi and Govatos (1967)
 4 Heywood (1999)
 82
 150 = K_D Durand (1953)

κ_f is the uncertainty factor on the friction factor due to solid particles, set to 1 to obtain the "best-estimate" prediction. Recommend at least 1.25 for upper bound estimate to account for uncertainties in the correlations.

Figure 17 compares the best-estimate predications of the total friction factor for pipe slurry flow based on Turian, Hsu, and Ma (1987), Durand (1953), Zandi and Govatos (1967), and Heywood (1999) as a function of the flow velocity and pipe roughness estimates of interest. The friction factor (f_c) for the liquid only is also shown for comparison. At velocities below the critical velocity, the friction factor of Turian, Hsu, and Ma (1987) or Heywood (1999) are expected to be more reliable because the data was more systematically evaluated in this regime.

Note that application of the above predicted friction factors for slurry flows with median solid particle diameters less than 100 μm requires extrapolation beyond the database used in the development of the correlations and hence should be used with caution.

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6.1.3 Pressure Drop

Head loss for pipe with positive incline to horizontal of θ (deg) per Blevins (1984) is given by

$$\Delta h_L(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta) := \frac{L}{D_{\text{pipe}}} \cdot \frac{v^2}{2 \cdot g} \cdot f(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, \beta_f, \kappa_f, \theta) \dots$$

$$+ \left[\left(\frac{\rho_d}{\rho_c} - 1 \right) \cdot \alpha_d + 1 \right] \cdot L \cdot \sin(\theta) \quad (\text{F-49})$$

The corresponding pressure loss is given by

$$\Delta p(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta) := \Delta h_L(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta) \cdot \rho_c \cdot g \quad (\text{F-50})$$

Hence, the total pressure loss for a combined carbon (CS) and stainless steel (SS) pipeline is given by

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon_{\text{CS}}, \varepsilon_{\text{SS}}, D_d, \rho_d, \alpha_d, L_{\text{CS}}, L_{\text{SS}}, \beta_f, \kappa_f, \theta) := \Delta p(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon_{\text{CS}}, D_d, \rho_d, \alpha_d, L_{\text{CS}}, \beta_f, \kappa_f, \theta) \dots$$

$$+ \Delta p(D_{\text{pipe}}, v, \rho_c, \mu_c, \varepsilon_{\text{SS}}, D_d, \rho_d, \alpha_d, L_{\text{SS}}, \beta_f, \kappa_f, \theta) \quad (\text{F-51})$$

where

L Length of pipe

The flow area and flow rate are given by

$$A(D_{\text{pipe}}) := \pi \cdot \frac{D_{\text{pipe}}^2}{4} \quad \text{flow area of pipe} \quad D_{\text{pipe}} := 3.068 \cdot \text{in}$$

$$Q(v_c, D_{\text{pipe}}) := v_c \cdot A(D_{\text{pipe}}) \quad \text{pipe flow rate} \quad A(D_{\text{pipe}}) = 7.393 \cdot \text{in}^2 \quad (\text{F-52})$$

The following sample pressure drop calculation is provided for comparison of the results from the various pressure drop correlations:

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Sample Pressure Drop Calculation - AN-107-to-BNFL Transfer Route

$$D_{\text{pipe}} := 3.068\text{-in} \quad v := 6 \frac{\text{ft}}{\text{sec}} \quad \rho_c := 1.2 \frac{\text{kg}}{\text{liter}} \quad \kappa_f := 1 \quad \epsilon_{CS} := \epsilon_{\text{new}} \quad \epsilon_{SS} := \epsilon_{\text{new}} \quad D_d := 400\text{-}\mu\text{m}$$

$$\beta_{cv} := 1.2 \quad \kappa_{cv} := 1 \quad \rho_d := 3 \frac{\text{kg}}{\text{liter}} \quad \alpha_d := 15\% \quad L_{CS} := 557\text{-ft} \quad L_{SS} := 7028\text{-ft} \quad H := 40\text{-ft}$$

$$T := 60 \text{ } ^\circ\text{C} \quad \mu_c := \mu_{cL}(\rho_c, T) \quad \mu_c = 0.771 \text{ cP} \quad \theta := \text{atan}\left(\frac{H}{L_{CS} + L_{SS}}\right)$$

$$v_{\text{cr}}(D_{\text{pipe}}, D_d, \rho_d, \alpha_d, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv}) = 5.482 \frac{\text{ft}}{\text{sec}}$$

critical velocity

Total pressure drop

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon_{CS}, \epsilon_{SS}, D_d, \rho_d, \alpha_d, L_{CS}, L_{SS}, 0, \kappa_f, \theta) = 194 \frac{\text{lb}}{\text{in}^2} \quad \text{liquid only, no solid particles}$$

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon_{CS}, \epsilon_{SS}, D_d, \rho_d, \alpha_d, L_{CS}, L_{SS}, 1, \kappa_f, \theta) = 574 \frac{\text{lb}}{\text{in}^2} \quad \text{Turian, Hsu, and Ma (1987)}$$

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon_{CS}, \epsilon_{SS}, D_d, \rho_d, \alpha_d, L_{CS}, L_{SS}, 2, \kappa_f, \theta) = 198 \frac{\text{lb}}{\text{in}^2} \quad \text{Turian and Yuan (1977)}$$

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon_{CS}, \epsilon_{SS}, D_d, \rho_d, \alpha_d, L_{CS}, L_{SS}, 3, \kappa_f, \theta) = 717 \frac{\text{lb}}{\text{in}^2} \quad \text{Zandi and Govatos (1987)}$$

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon_{CS}, \epsilon_{SS}, D_d, \rho_d, \alpha_d, L_{CS}, L_{SS}, 4, \kappa_f, \theta) = 475 \frac{\text{lb}}{\text{in}^2} \quad \text{Heywood (1999)}$$

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon_{CS}, \epsilon_{SS}, D_d, \rho_d, \alpha_d, L_{CS}, L_{SS}, 82, \kappa_f, \theta) = 468 \frac{\text{lb}}{\text{in}^2} \quad K_D = 82, \text{ Durand (1953)}$$

$$\Delta p_T(D_{\text{pipe}}, v, \rho_c, \mu_c, \epsilon_{CS}, \epsilon_{SS}, D_d, \rho_d, \alpha_d, L_{CS}, L_{SS}, 150, \kappa_f, \theta) = 694 \frac{\text{lb}}{\text{in}^2} \quad K_D = 150, \text{ Durand (1953)}$$

Note that there is a wide variation in results between the various correlations.

For slurry flow in a 3-inch pipe, Figures 18 through 20 show best-estimate pressure drop contour plots per 1,000 ft of pipe, corresponding flow regime contour plots, and critical velocity contour plots for the range of carrier liquid density and particle density of interest for the Hanford waste. The pressure drop results are based on the Turian, Hsu, and Ma (1987) friction factor correlation and the critical velocity results are based on the Oroskar and Turian (1980) empirical correlation. The pressure drop results are shown at a flow velocity of 5 ft/s for end-of-life (EOF) pipe roughness estimates for carbon and stainless steel as given above. The solid particle size, solid fraction by volume, and temperature of the flow are indicated in the figures. These figures provide an upper bound estimate to the friction pressure loss, particularly for solid particle diameters less than 100 μm . For slurry flows with broad solid particle distributions the method of Wasp et al. (1977) which accounts for the effect of the "fine" and large particles is expected to provide more realistic predictions.

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6.2 COMBINED HOMOGENEOUS-HETEROGENEOUS SLURRY FLOW - WASP METHOD

Wasp (1977) developed a method to estimate the pipe pressure loss for a combined homogeneous-heterogeneous flow with multiple particle sizes based on the concept of a "two-phase vehicle." Fine particles join with the carrier liquid to form a homogeneous "vehicle" while the coarse particles are suspended as a heterogeneous "bed" within this homogeneous vehicle. The fine particles in the vehicle modify the effective liquid density and rheology of the liquid. High concentrations (approaching 30% by volume) of "fine" particles (particle size diameters less than 100 μm) can result in non-Newtonian viscosity characteristics. In the application herein the viscosity of the vehicle is assumed to remain Newtonian and increase with increasing fine particle concentration in accordance with the Thomas correlation

$$\mu_c(\mu_L, \Phi_v) := \mu_L \cdot (1 + 2.5 \cdot \Phi_v + 10.05 \cdot \Phi_v^2 + 0.00273 \cdot \exp(16.6 \cdot \Phi_v)) \quad \text{Thomas correlation (Wasp et al. 1977)} \quad (F-53)$$

and the effective density of the vehicle is given by

$$\rho_c(\rho_s, \rho_L, \Phi_v) := \rho_s \cdot \Phi_v + \rho_L \cdot (1 - \Phi_v) \quad (F-54)$$

where Φ_v is the concentration of fine particles joining with the carrier fluid.

Although other viscosity correlations are available, they require additional empirical parameters. Hence, the Thomas correlation was selected as a first approximation due to the lack of data applicable to the Hanford waste. The above relations are shown in Figure F-21 for the selected conditions indicated.

The Wasp method requires a particle size distribution. The assumed particle size distribution for the solid particles is based on the measured particle size distribution by volume for Tank 241-SY-101 (O'Rourke 1999). The measured distribution was curve fit to a Rosin-Rammler distribution function (Shook and Roco 1991) and then shifted to obtain a distribution with the target median particle size of interest.

6.2.1 Preliminaries

$$TOL = 1 \times 10^{-5}$$

$$\Delta\epsilon := TOL \cdot 10^{-1} \quad \Delta\epsilon = 1 \times 10^{-6}$$

Return cumulative vector ϕ_{cum} for frequency distribution vector ϕ

$$\text{cum}(\phi) := \begin{cases} \text{for } j \in 0.. \text{length}(\phi) - 1 \\ \phi_{cum_j} \leftarrow \sum_{i=0}^j \phi_i \\ \phi_{cum} \end{cases} \quad (F-55)$$

Return values of vector ϕ_p in accordance with nonzero values in vector $\phi_z > \Delta\epsilon$

$$\text{pack}(\phi_p, \phi_z) := \begin{cases} J \leftarrow \text{length}(\phi_z) - 1 \\ k \leftarrow 0 \\ \text{for } j \in 0..J \\ \text{if } \phi_{z_j} > \Delta\epsilon \\ \left| \begin{array}{l} \phi_k \leftarrow \phi_{p_j} \\ k \leftarrow k + 1 \end{array} \right. \\ \text{if } k = 0 \\ \left| \begin{array}{l} \phi_0 \leftarrow \Delta\epsilon \\ \phi_1 \leftarrow \Delta\epsilon \end{array} \right. \\ \phi \end{cases} \quad (F-56)$$

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Return class size intervals of density distribution data

Return d_{β} particle diameter that is β th percentile value by volumes

$$\Delta(d_p) := \left[\begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Delta d_0 \leftarrow \frac{d_{p_1} - d_{p_0}}{2} \\ \text{for } j \in 1..J - 1 \\ \Delta d_j \leftarrow \frac{d_{p_{j+1}} - d_{p_{j-1}}}{2} \\ \Delta d_J \leftarrow \frac{d_{p_J} - d_{p_{J-1}}}{2} \\ \Delta d \end{array} \right] \quad (F-57)$$

$$D_{\beta}(d_p, \phi_{cum}, \beta) := \left[\begin{array}{l} J \leftarrow \text{length}(\phi_{cum}) - 1 \\ \text{for } k \in 0..J \\ \text{break if } \phi_{cum_k} > \beta \\ \frac{d_{p_k} - d_{p_{k-1}}}{\phi_{cum_k} - \phi_{cum_{k-1}}} \cdot (\beta - \phi_{cum_{k-1}}) + d_{p_{k-1}} \end{array} \right] \quad (F-58)$$

The following cumulative and probability density function from Table 1-1 Shook and Roco (1991) were considered for curve fits of the SY-101 particle size distribution.

Cumulative distribution function

Probability density function

Rosin-Rammler

$$CDF_{RR}(d, d_{50}, p) := \left[1 - \exp \left[-\ln(2) \cdot \left(\frac{d}{d_{50}} \right)^p \right] \right]$$

$$PDF_{RR}(d, d_{50}, p) := \ln(2) \cdot \left(\frac{d}{d_{50}} \right)^p \cdot \frac{p}{d} \cdot \exp \left[-\ln(2) \cdot \left(\frac{d}{d_{50}} \right)^p \right] \quad (F-59)$$

Partial derivative of Rosin-Rammler CDF with respect to parameter p

$$CDF_{RR,p}(d, d_{50}, p) := \ln(2) \cdot \left(\frac{d}{d_{50}} \right)^p \cdot \ln \left(\frac{d}{d_{50}} \right) \cdot \exp \left[-\ln(2) \cdot \left(\frac{d}{d_{50}} \right)^p \right]$$

Log normal

$$CDF_{LN}(d, d_{50}, p) := \frac{1}{\sqrt{2 \cdot \pi}} \int_{-\infty}^{\frac{\ln \left(\frac{d}{d_{50}} \right)}{\ln(p)}} \exp \left(-\frac{t^2}{2} \right) dt$$

$$PDF_{LN}(d, d_{50}, p) := \frac{1}{\sqrt{2 \cdot \pi}} \cdot \exp \left[-\frac{1}{2} \cdot \frac{\left(\ln \left(\frac{d}{d_{50}} \right) \right)^2}{\ln(p)^2} \right] \cdot \frac{1}{d \cdot \ln(p)} \quad (F-60)$$

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Partial derivative of Log-normal CDF with respect to parameter p

$$CDF_{LNp}(d, d_{50}, p) := \frac{-1}{\sqrt{2 \cdot \pi}} \cdot \exp \left[\frac{-1}{2} \cdot \frac{\left(\ln \left(\frac{d}{d_{50}} \right) \right)^2}{\ln(p)^2} \right] \cdot \frac{\ln \left(\frac{d}{d_{50}} \right)}{\ln(p)^2 \cdot p}$$

6.2.2 Wasp's Method for Pressure Drop Estimate

In accordance with Wasp et al. 1977, assume $\beta := 1$ $\kappa := 0.4$ von Karman constant

$$\phi_v(v, \rho_L, \rho_c, \mu_c, d_p, \rho_s, \phi_s, f) := \phi_s \cdot 10^{\frac{-1.8 \cdot v_w(d_p, \rho_s, \rho_c, \mu_c)}{\beta \cdot \kappa} \cdot \sqrt{\frac{f \cdot \rho_L}{\rho_c} \cdot \frac{1}{8}}} \tag{F-61}$$

Return total concentration of solid particle attributed to "vehicle" liquid from each particle of particle size distribution using vehicle properties

$$\Phi_d(v, \rho_L, \mu_L, d_p, \rho_s, \phi_s, f) := \begin{cases} J \leftarrow \text{length}(d_p) - 1 \\ \Phi'_v \leftarrow 0.5 \cdot \sum_{j=0}^J \phi_{s_j} & \text{initial guess} \\ \text{root} \left(\Phi'_v - \sum_{j=0}^J \phi_v(v, \rho_L, \rho_c(\rho_s, \rho_L, \Phi'_v), \mu_c(\mu_L, \Phi'_v), d_{p_j}, \rho_s, \phi_{s_j}, f), \Phi'_v \right) \end{cases} \tag{F-62}$$

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Darcy friction factor and pressure loss based on Wasp method using Durand friction factor correlation (see Equation F-45) with

$$K_D := 82$$

$$f_{Wasp_f}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \theta, f) := \left\{ \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Phi_v \leftarrow \Phi_d(v, \rho_L, \mu_L, d_p, \rho_s, \phi_s, f) \\ \rho_v \leftarrow \rho_d(\rho_s, \rho_L, \Phi_v) \\ \mu_v \leftarrow \mu_d(\mu_L, \Phi_v) \\ f_w \leftarrow f_d(D_{pipe}, v, \rho_v, \mu_v, \epsilon) \cdot \frac{\rho_v}{\rho_L} \\ \text{for } j \in 0..J \\ \left\{ \begin{array}{l} \phi_{vj} \leftarrow \phi_v(v, \rho_L, \rho_v, \mu_v, d_{p_j}, \rho_s, \phi_{s_j}, f) \\ \phi_{aj} \leftarrow \phi_{s_j} - \phi_{vj} \\ f_w \leftarrow f_w + f_d(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_{p_j}, \rho_s, \phi_{aj}, K_D, 1, \theta) \end{array} \right. \\ f_w \end{array} \right. \quad \begin{array}{l} \text{(F-63a)} \\ \\ \\ \text{homogeneous vehicle} \\ \text{friction contribution} \\ \text{normalized to clear liquid} \\ \\ \\ \text{plus friction contribution} \\ \text{from remaining} \\ \text{heterogeneous particles} \\ \text{using clear liquid properties} \end{array}$$

$$\Delta\Phi := \text{TOL} \cdot 10^{-1}$$

$$f_f := 0.025 \text{ initial guess}$$

$$f_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \kappa_f, \theta) := \left\{ \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Phi_s \leftarrow \sum_{j=0}^J \phi_{s_j} \\ f_d(D_{pipe}, v, \rho_L, \mu_L, \epsilon) \text{ if } \Phi_s < \Delta\Phi \\ \kappa_f \cdot \text{root}(f_f - f_{Wasp_f}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \theta, f_f), f_f) \text{ otherwise} \end{array} \right. \quad \text{(F-63b)}$$

Pressure drop

$$\Delta p_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, L, \kappa_f, \theta) := \left\{ \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Phi_s \leftarrow \sum_{j=0}^J \phi_{s_j} \\ \rho_L \cdot g \cdot \left[\frac{L}{D_{pipe}} \cdot \frac{v^2}{2 \cdot g} \cdot f_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \kappa_f, \theta) \dots \right. \\ \left. + \left[\left(\frac{\rho_s}{\rho_L} - 1 \right) \cdot \Phi_s + 1 \right] \cdot L \cdot \sin(\theta) \right] \end{array} \right. \quad \text{(F-64)}$$

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The total pressure drop for a combined carbon (CS) and stainless steel (SS) pipeline is given by

$$\Delta P_{T_Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon_{CS}, \epsilon_{SS}, d_p, \rho_s, \phi_s, L_{CS}, L_{SS}, \kappa_f, \theta) := \Delta p_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon_{CS}, d_p, \rho_s, \phi_s, L_{CS}, \kappa_f, \theta) \dots \quad (F-65)$$

$$+ \Delta p_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon_{SS}, d_p, \rho_s, \phi_s, L_{SS}, \kappa_f, \theta)$$

6.2.3 Critical Velocity Based on Wasp "Vehicle" Concept

This procedure predicts a lower critical velocity for a given particle size distribution as the median particle size for the distribution decreases. Although this procedure appears logical, it is provided here for comparison only because it has not been validated by experimental results.

Critical Deposition Velocity

$$v_{crDvWasp}(D_{pipe}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \beta_{cv}, v) := \left\{ \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ f_w \leftarrow f_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, 1, \theta) \\ \Phi_v \leftarrow \Phi_c(v, \rho_L, \mu_L, d_p, \rho_s, \phi_s, f_w) \\ \rho_v \leftarrow \rho_c(\rho_s, \rho_L, \Phi_v) \\ \mu_v \leftarrow \mu_c(\mu_L, \Phi_v) \\ \phi_a \leftarrow 0 \\ \text{for } j \in 0..J \\ \quad \left\{ \begin{array}{l} \phi_{vj} \leftarrow \phi_v(v, \rho_L, \rho_v, \mu_v, d_{p_j}, \rho_s, \phi_{s_j}, f_w) \\ \phi_{a_j} \leftarrow \phi_{s_j} - \phi_{vj} \end{array} \right. \\ d_{pp} \leftarrow 0 \\ \phi_{ap} \leftarrow 0 \\ d_{pp} \leftarrow \text{pack}(d_p, \phi_a) \\ \phi_{ap} \leftarrow \text{pack}(\phi_a, \phi_a) \\ \phi_{cum} \leftarrow \text{cum}(\phi_{ap}) \\ \Phi_a \leftarrow \sum_{j=0}^{\text{length}(\phi_{ap})-1} \phi_{ap_j} \\ D_{p\beta} \leftarrow D_\beta\left(d_{pp}, \frac{\phi_{cum}}{\Phi_a}, 0.5\right) \\ v_{cr}(D_{pipe}, D_{p\beta}, \rho_s, \Phi_a, \rho_v, \mu_v, 1, \beta_{cv}) \end{array} \right. \quad (F-66a)$$

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$$v' := 1 \cdot \frac{\text{ft}}{\text{sec}} \quad \text{initial guess}$$

$$v_{\text{crD_Wasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \kappa_{\text{cv}}, \beta_{\text{cv}}) := \kappa_{\text{cv}} \cdot \text{root}(v' - v_{\text{crDvWasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \beta_{\text{cv}}, v'), v') \quad (\text{F-66b})$$

Transition to Turbulence Velocity

$$Re_T := 4000$$

$$v_{\text{crTvWasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \kappa_{\text{cv}}, v) := \left| \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ f_w \leftarrow f_{\text{Wasp}}(D_{\text{pipe}}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, 1, \theta) \\ \Phi_v \leftarrow \Phi_d(v, \rho_L, \mu_L, d_p, \rho_s, \phi_s, f_w) \\ \rho_v \leftarrow \rho_c(\rho_s, \rho_L, \Phi_v) \\ \mu_v \leftarrow \mu_c(\mu_L, \Phi_v) \\ \frac{Re_T \cdot \mu_v}{D_{\text{pipe}} \cdot \rho_v} \end{array} \right. \quad (\text{F-67a})$$

$$v_{\text{crT_Wasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \kappa_{\text{cv}}) := \text{root}(v' - v_{\text{crTvWasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \kappa_{\text{cv}}, v'), v') \quad (\text{F-67b})$$

Critical Velocity

$$v_{\text{cr_Wasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \kappa_{\text{cv}}, \beta_{\text{cv}}) := \frac{\text{m}}{\text{sec}} \cdot \left| \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Phi_s \leftarrow \sum_{j=0}^J \phi_{s_j} \\ \max \left(\left(\begin{array}{l} v_{\text{crD_Wasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \kappa_{\text{cv}}, \beta_{\text{cv}}) \\ v_{\text{crT_Wasp}}(D_{\text{pipe}}, d_p, \rho_s, \phi_s, \rho_L, \mu_L, \epsilon, \kappa_{\text{cv}}) \end{array} \right) \right) \text{ if } \Phi_s > \Delta \Phi \\ \frac{Re_T \cdot \mu_L}{D_{\text{pipe}} \cdot \rho_L} \text{ otherwise} \end{array} \right. \quad (\text{F-68})$$

where

- 1 Oroskar and Turian (1980) maximum of semi-theoretical and empirical equation
 1.1 semi-theoretical equation
 1.2 empirical equation
 $\beta_{\text{cv}} =$ 2 Wasp et al. (1977)
 3 Zandi and Govatos (1967)
 4 Shook (1969)
 5 Gogus and Kokpinar (1999)
 6 Robinson and Graf (1972)

κ_{cv} is the uncertainty factor on the critical, set to 1 to obtain the "best-estimate" prediction. Recommend at least 1.3 for upper bound estimate to account for uncertainties in the empirical correlation and to include sufficient margin above the critical velocity for minimum operating flow rate.

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6.2.4 Generic Particle Size Distribution for Hanford Waste Feed Delivery

Tank SY-101 particle size distribution (partial listing displayed) (O'Rourke 1999)

d_p (μm)	volume frequency (%)
0.022	0.000
0.026	0.000
0.029	0.000
0.034	0.000
0.039	0.000
0.044	0.000
0.051	0.000
0.058	0.000
0.067	0.000
0.076	0.000
0.087	0.000
0.100	0.000
0.115	0.000
0.131	0.000
0.150	0.000
0.172	0.000
0.197	0.000
0.226	0.160
0.259	0.340
0.296	0.270
0.339	0.120

$d_p := \text{SY101}^{(0)} \cdot \mu\text{m}$

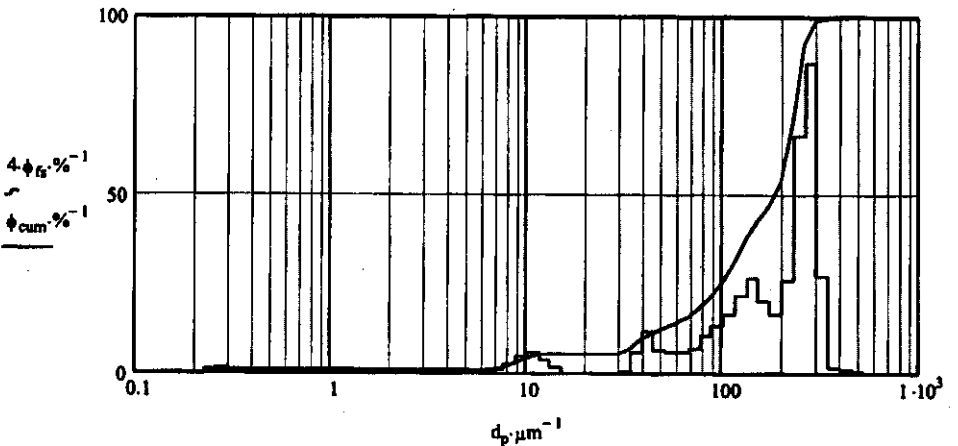
$\phi_{fs} := \text{SY101}^{(1)} \cdot \%$

$\text{length}(\phi_{fs}) - 1 = 113$

$\Phi_{fs} := \sum_{j=0}^{\text{length}(\phi_{fs})-1} \phi_{fs,j} \quad \Phi_{fs} = 1$

Cumulative undersize volume distribution

$\phi_{cum} := \text{cum}(\phi_{fs})$



$d_{50R} := D_{\beta}(d_p, \phi_{cum}, 50\%) \quad d_{50R} = 184.063 \mu\text{m}$

Curve fit to Rosin-Rammler and Log Normal distribution functions

$F_{RR}(d, p) := \begin{pmatrix} \text{CDF}_{RR}(d, d_{50R} \cdot \mu\text{m}^{-1}, p) \\ \text{CDF}_{RR,p}(d, d_{50R} \cdot \mu\text{m}^{-1}, p) \end{pmatrix}$

$F_{LN}(d, p) := \begin{pmatrix} \text{CDF}_{LN}(d, d_{50R} \cdot \mu\text{m}^{-1}, p) \\ \text{CDF}_{LN,p}(d, d_{50R} \cdot \mu\text{m}^{-1}, p) \end{pmatrix} \quad \text{(F-69)}$

$v_{PRR} := 1.5$

$v_{PLN} := 1.5$

$p_{PRR} := \text{genfit}(d_p \cdot \mu\text{m}^{-1}, \phi_{cum}, v_{PRR}, F_{RR})$

$p_{PLN} := \text{genfit}(d_p \cdot \mu\text{m}^{-1}, \phi_{cum}, v_{PLN}, F_{LN}) \quad \Delta d_p := \Delta(d_p)$

$p_{PRR} = 1.933 \quad d_{50} := d_{50R} \\ d_{50} := 200 \cdot \mu\text{m}$

$p_{PLN} = 1.811$

$J := \text{length}(\phi_{fs}) - 1 \quad j := 0..J$

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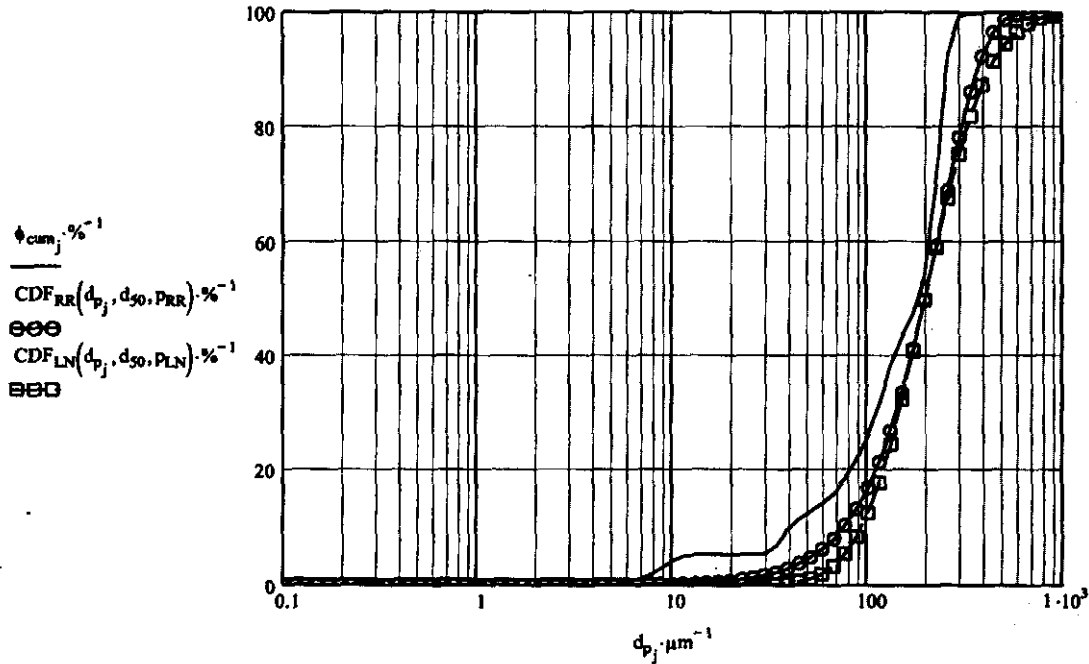
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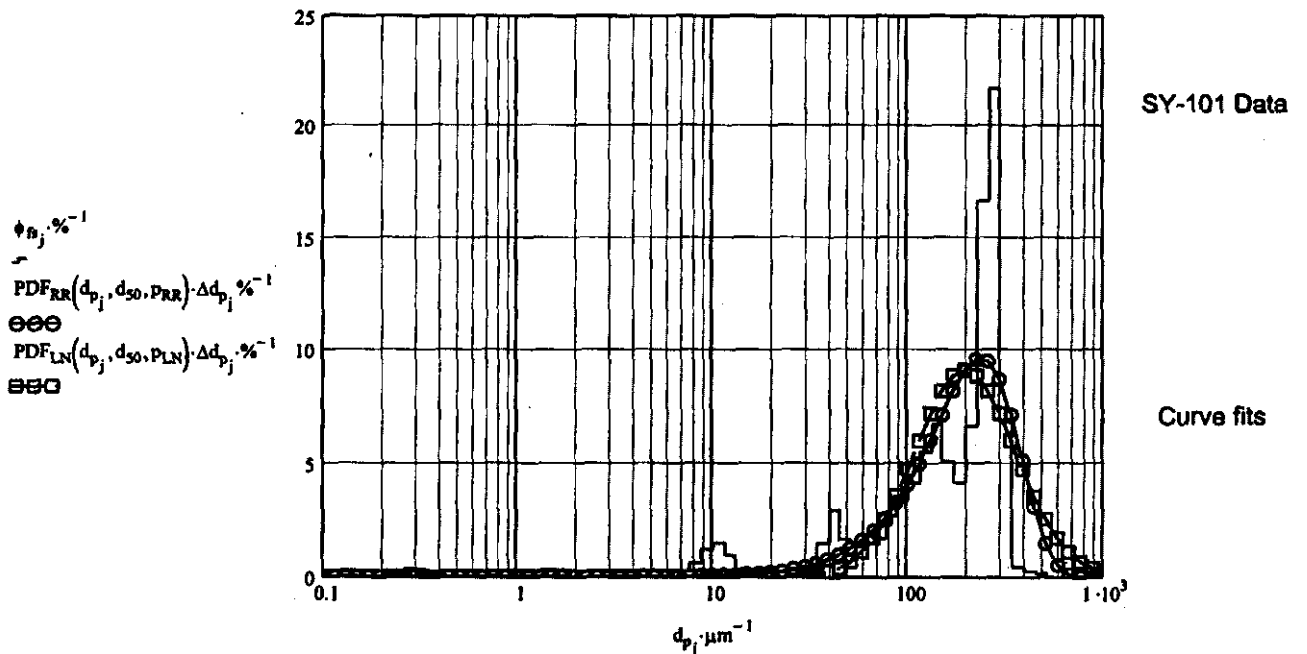
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Comparison of SY-101 cumulative and frequency data to Rosin-Rammler and Log Normal distribution function curve fits.

Particle Size Cumulative Distribution



Particle Size Frequency Distribution



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$$d_{pR} := d_p \quad \Delta d_{pR} := \Delta d_p \quad j_R := 0.. \text{length}(d_p) - 1 \quad \phi_R := \phi_{fs} \quad \phi_{cumR} := \phi_{cum}$$

Pack original solid particle distribution for analysis to remove zero frequency data

$$d_p := \text{pack}(d_p, \phi_{fs})$$

$$\phi_{fs} := \text{pack}(\phi_{fs}, \phi_{fs}) \quad \phi_{cum} := \text{cum}(\phi_{fs}) \quad \text{length}(\phi_{fs}) - 1 = 30$$

$$D_p := D_\beta(d_p, \phi_{cum}, 50\%) \quad C_s := 10\%$$

$$D_p = 184.063 \mu\text{m} \quad \phi_s := \phi_{fs} \cdot C_s \quad \Phi_s := \sum_{j=0}^{\text{length}(\phi_s)-1} \phi_{s_j} \quad \Phi_s = 0.1$$

Return Rosin-Rammler probability density function values (d φ) shifted to d₅₀ from reference distribution and packed for analysis

Return Log-Normal probability density function values (d φ) shifted to d₅₀ from reference distribution and packed for analysis

$$\text{dist}_{RR}(d_p, d_{50}) := \begin{cases} J \leftarrow \text{length}(d_p) - 1 \\ \Delta d \leftarrow \Delta(d_p) \\ \text{for } j \in 0..J \\ \phi_j \leftarrow \text{PDF}_{RR}(d_p, d_{50}, P_{RR}) \cdot \Delta d_j \\ d \leftarrow \text{pack}(d_p, \phi) \cdot \mu\text{m}^{-1} \\ \phi \leftarrow \text{pack}(\phi, \phi) \\ (d \ \phi) \end{cases}$$

$$\text{dist}_{LN}(d_p, d_{50}) := \begin{cases} J \leftarrow \text{length}(d_p) - 1 \\ \Delta d \leftarrow \Delta(d_p) \\ \text{for } j \in 0..J \\ \phi_j \leftarrow \text{PDF}_{LN}(d_p, d_{50}, P_{LN}) \cdot \Delta d_j \\ d \leftarrow \text{pack}(d_p, \phi) \cdot \mu\text{m}^{-1} \\ \phi \leftarrow \text{pack}(\phi, \phi) \\ (d \ \phi) \end{cases}$$

(F-70)

$$(d_{pRR} \ \phi_{RR}) := \text{dist}_{RR}(d_{pR}, d_{50}) \quad d_{pRR} := d_{pRR} \cdot \mu\text{m} \quad \text{reset units}$$

$$\phi_{RRcum} := \text{cum}(\phi_{RR}) \quad \text{length}(\phi_{RR}) - 1 = 57$$

$$D_{pRR} := D_\beta(d_{pRR}, \phi_{RRcum}, 50\%)$$

$$D_{pRR} = 186.7 \mu\text{m}$$

$$(d_{pLN} \ \phi_{LN}) := \text{dist}_{LN}(d_{pR}, d_{50}) \quad d_{pLN} := d_{pLN} \cdot \mu\text{m} \quad \text{reset units}$$

$$\phi_{LNcum} := \text{cum}(\phi_{LN}) \quad \text{length}(\phi_{LN}) - 1 = 40$$

$$D_{pLN} := D_\beta(d_{pLN}, \phi_{LNcum}, 50\%)$$

$$D_{pLN} = 186.872 \mu\text{m}$$

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Appendix D provides a tabular listing of the critical velocity, pressure drop at critical velocity, and flow rate at a reference pipeline design pressure of 400 lbf/in² for each of the 200-East Area transfer routes identified in Cases 3S4 and 3S6 from the HTWOS model for a range of waste transfer properties. An alternate solutions to reduce the predicted pressure drops for worst-case conditions was also considered. This alternate solution assumed a pump lift station in AP Farms located at the AP "new" valve pit. Selected long intermediate tank-to-tank transfers and all transfers to BNFL (Case 4-st) would go through the pump lift station. Appendix E provides the results of a "sensitivity" analysis on the longest transfer route (AN-107 to BNFL) for the range of waste transfer properties considered appropriate. See main body of this report for a discussion of the detail results. Appendix G provides results of Wasp model verification.

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7.0 ANALYSIS OF TRANSFER PIPELINES FOR PROPOSED TRANSFERS - Wasp Method

$$\begin{pmatrix} DL \\ D0 \\ D1 \end{pmatrix} :=$$



Worksheet

read transfer pipeline physical parameters and data sets for waste transfer parameters from worksheet

TOL = 5×10^{-4}

Pipe design pressure

$D_{pipe} := 3.068 \cdot \text{in}$

$P_{pipe_design} := 400 \cdot \frac{\text{lbf}}{\text{in}^2}$

$\beta_{cv} := 1.2$

$L_{CS} := DL^{(0)} \cdot \text{ft}$

$L_{SS} := DL^{(1)} \cdot \text{ft}$

$H := DL^{(2)} \cdot \text{ft}$

$N := \text{length}(L_{CS})$

$N = 75$

$i := 0..N - 1$

$j := 0..5$

$$\Theta := \text{atan}\left(\frac{H}{L_{CS} + L_{SS}}\right)$$

The above critical velocity and pressure drop correlations are "best-estimate" correlations. In application of these correlations, the uncertainty in the predictive correlation must be considered.

approximate 2σ uncertainty factors on predictive correlations

$\kappa_{cv} := 1.30$

uncertainty factor on predicted critical velocity plus factor to assure operation above the critical velocity

$\kappa_f := 1$

uncertainty factor on predicted friction factor - for best-estimate $\kappa_f = 1$

Data Set 0 - Group 1 Properties

$T := 10 \text{ } ^\circ\text{C}$

$\rho_{c0} := D0^{(0)} \cdot \frac{\text{kg}}{\text{liter}}$

$\rho_{d0} := D0^{(1)} \cdot \frac{\text{kg}}{\text{liter}}$

$R0_{i,j} := 0$

$d_{pRR} := 0$

$\phi_{RR} := 0$

$\mu_{c0_i} := \mu_{cL}(\rho_{c0_i}, T)$

$\alpha_{d0} := D0^{(2)} \cdot \%$

$D_{d0} := D0^{(3)} \cdot \mu\text{m}$

$(d_{pRR}^{(i)} \phi_{RR}^{(i)}) := \text{dist}_{RR}(d_{pR}, D_{d0_i})$

$d_{pRR} := d_{pRR} \cdot \mu\text{m}$

reset units

$v_{cm0_i} := v_{cr}(D_{pipe}, D_{d0_i}, \rho_{d0_i}, \alpha_{d0_i}, \rho_{c0_i}, \mu_{c0_i}, \kappa_{cv}, \beta_{cv})$

$Q_{cm0_i} := Q(v_{cm0_i}, D_{pipe})$

$\Delta P_{new0_i} := \Delta P_{T_Wasp}(D_{pipe}, v_{cm0_i}, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i)$

$\Delta P_{EOL0_i} := \Delta P_{T_Wasp}(D_{pipe}, v_{cm0_i}, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i)$

$v0 := 10 \cdot \frac{\text{ft}}{\text{sec}}$

$v_{max_new0_i} := -1 \cdot \frac{\text{ft}}{\text{sec}} \text{ on error root}\left(\begin{matrix} P_{pipe_design} \dots \\ + \Delta P_{T_Wasp}(D_{pipe}, v0, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i) \end{matrix}, v0\right)$

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$$v_{max_EOL0_i} := -1 \cdot \frac{ft}{sec} \text{ on error root} \left(P_{pipe_design} \dots \right. \\ \left. + -\Delta P_{T_Wasp} \left(D_{pipe}, v_0, \rho_{c0}, \mu_{c0}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0}, \alpha_{d0}, \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i \right) \right) , v_0$$

$$R0^{(0)} := v_{cm0} \left(\frac{ft}{sec} \right)^{-1} \quad R0^{(1)} := Q_{cm0} \left(\frac{gal}{min} \right)^{-1} \quad R0^{(2)} := \Delta P_{new0} \left(\frac{lbf}{in^2} \right)^{-1} \quad R0^{(3)} := \Delta P_{EOL0} \left(\frac{lbf}{in^2} \right)^{-1}$$

$$R0^{(4)} := Q(v_{max_new0}, D_{pipe}) \cdot \left(\frac{gal}{min} \right)^{-1} \quad R0^{(5)} := Q(v_{max_EOL0}, D_{pipe}) \cdot \left(\frac{gal}{min} \right)^{-1}$$

Data Set 0 - Group 1 Properties

T := 60 °C

R1_{i,j} := 0

$\mu_{c0} := \mu_{cL}(\rho_{c0}, T)$

$v_{cm0} := v_{cr}(D_{pipe}, D_{d0}, \rho_{d0}, \alpha_{d0}, \rho_{c0}, \mu_{c0}, \kappa_{cv}, \beta_{cv})$

$Q_{cm0} := Q(v_{cm0}, D_{pipe})$

$\Delta P_{new0} := \Delta P_{T_Wasp} \left(D_{pipe}, v_{cm0}, \rho_{c0}, \mu_{c0}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0}, \alpha_{d0}, \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i \right)$

$\Delta P_{EOL0} := \Delta P_{T_Wasp} \left(D_{pipe}, v_{cm0}, \rho_{c0}, \mu_{c0}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0}, \alpha_{d0}, \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i \right)$

$$v_{max_new0_i} := -1 \cdot \frac{ft}{sec} \text{ on error root} \left(P_{pipe_design} \dots \right. \\ \left. + -\Delta P_{T_Wasp} \left(D_{pipe}, v_0, \rho_{c0}, \mu_{c0}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0}, \alpha_{d0}, \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i \right) \right) , v_0$$

$$v_{max_EOL0_i} := -1 \cdot \frac{ft}{sec} \text{ on error root} \left(P_{pipe_design} \dots \right. \\ \left. + -\Delta P_{T_Wasp} \left(D_{pipe}, v_0, \rho_{c0}, \mu_{c0}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0}, \alpha_{d0}, \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i \right) \right) , v_0$$

$$R1^{(0)} := v_{cm0} \left(\frac{ft}{sec} \right)^{-1} \quad R1^{(1)} := Q_{cm0} \left(\frac{gal}{min} \right)^{-1} \quad R1^{(2)} := \Delta P_{new0} \left(\frac{lbf}{in^2} \right)^{-1} \quad R1^{(3)} := \Delta P_{EOL0} \left(\frac{lbf}{in^2} \right)^{-1}$$

$$R1^{(4)} := Q(v_{max_new0}, D_{pipe}) \cdot \left(\frac{gal}{min} \right)^{-1} \quad R1^{(5)} := Q(v_{max_EOL0}, D_{pipe}) \cdot \left(\frac{gal}{min} \right)^{-1}$$

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Data Set 1 - Group 2 Properties

T := 10 °C

$$\rho_{c0} := D1^{(0)} \cdot \frac{\text{kg}}{\text{liter}}$$

$$\rho_{d0} := D1^{(1)} \cdot \frac{\text{kg}}{\text{liter}}$$

$$R2_{i,j} := 0$$

$$d_{pRR} := 0$$

$$\phi_{RR} := 0$$

$$\mu_{c0_i} := \mu_{cL}(\rho_{c0}, T)$$

$$\alpha_{d0} := D1^{(2)} \cdot \%$$

$$D_{d0} := D1^{(3)} \cdot \mu\text{m}$$

$$(d_{pRR}^{(i)} \phi_{RR}^{(i)}) := \text{dist}_{RR}(d_{pR}, D_{d0_i})$$

$$d_{pRR} := d_{pRR} \cdot \mu\text{m}$$

reset units

$$v_{cm0_i} := v_{cr}(D_{pipe}, D_{d0_i}, \rho_{d0_i}, \alpha_{d0_i}, \rho_{c0_i}, \mu_{c0_i}, \kappa_{cv}, \beta_{cv})$$

$$Q_{cm0_i} := Q(v_{cm0_i}, D_{pipe})$$

$$\Delta p_{new0_i} := \Delta p_{T_Wasp}(D_{pipe}, v_{cm0_i}, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i)$$

$$\Delta p_{EOL0_i} := \Delta p_{T_Wasp}(D_{pipe}, v_{cm0_i}, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i)$$

$$v0 := 10 \cdot \frac{\text{ft}}{\text{sec}}$$

$$v_{max_new0_i} := -1 \cdot \frac{\text{ft}}{\text{sec}} \text{ on error root} \left(p_{pipe_design} \dots \right. \\ \left. + -\Delta p_{T_Wasp}(D_{pipe}, v0, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i) \right), v0$$

$$v_{max_EOL0_i} := -1 \cdot \frac{\text{ft}}{\text{sec}} \text{ on error root} \left(p_{pipe_design} \dots \right. \\ \left. + -\Delta p_{T_Wasp}(D_{pipe}, v0, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i) \right), v0$$

$$R2^{(0)} := v_{cm0} \cdot \left(\frac{\text{ft}}{\text{sec}} \right)^{-1}$$

$$R2^{(1)} := Q_{cm0} \cdot \left(\frac{\text{gal}}{\text{min}} \right)^{-1}$$

$$R2^{(2)} := \Delta p_{new0} \cdot \left(\frac{\text{lbf}}{\text{in}^2} \right)^{-1}$$

$$R2^{(3)} := \Delta p_{EOL0} \cdot \left(\frac{\text{lbf}}{\text{in}^2} \right)^{-1}$$

$$R2^{(4)} := Q(v_{max_new0}, D_{pipe}) \cdot \left(\frac{\text{gal}}{\text{min}} \right)^{-1}$$

$$R2^{(5)} := Q(v_{max_EOL0}, D_{pipe}) \cdot \left(\frac{\text{gal}}{\text{min}} \right)^{-1}$$

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Data Set 1 - Group 2 Properties

T := 60 °C

$$\mu_{c0_i} := \mu_{cL}(\rho_{c0_i}, T)$$

$$R3_{i,j} := 0$$

$$v_{cm0_i} := v_{cr}(D_{pipe}, D_{d0_i}, \rho_{d0_i}, \alpha_{d0_i}, \rho_{c0_i}, \mu_{c0_i}, \kappa_{cv}, \beta_{cv})$$

$$Q_{cm0_i} := Q(v_{cm0_i}, D_{pipe})$$

$$\Delta P_{new0_i} := \Delta P_{T_Wasp}(D_{pipe}, v_{cm0_i}, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i)$$

$$\Delta P_{EOL0_i} := \Delta P_{T_Wasp}(D_{pipe}, v_{cm0_i}, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i)$$

$$v_{max_new0_i} := -1 \cdot \frac{ft}{sec} \text{ on error root} \left(P_{pipe_design} \dots \left(+ \Delta P_{T_Wasp}(D_{pipe}, v0, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{new}, \epsilon_{new}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i) \right), v0 \right)$$

$$v_{max_EOL0_i} := -1 \cdot \frac{ft}{sec} \text{ on error root} \left(P_{pipe_design} \dots \left(+ \Delta P_{T_Wasp}(D_{pipe}, v0, \rho_{c0_i}, \mu_{c0_i}, \epsilon_{EOL_CS}, \epsilon_{EOL_SS}, d_{pRR}^{(i)}, \rho_{d0_i}, \alpha_{d0_i} \cdot \phi_{RR}^{(i)}, L_{CS_i}, L_{SS_i}, \kappa_f, \Theta_i) \right), v0 \right)$$

$$R3^{(0)} := v_{cm0} \left(\frac{ft}{sec} \right)^{-1} \quad R3^{(1)} := Q_{cm0} \left(\frac{gal}{min} \right)^{-1} \quad R3^{(2)} := \Delta P_{new0} \left(\frac{lbf}{in^2} \right)^{-1} \quad R3^{(3)} := \Delta P_{EOL0} \left(\frac{lbf}{in^2} \right)^{-1}$$

$$R3^{(4)} := Q(v_{max_new0}, D_{pipe}) \left(\frac{gal}{min} \right)^{-1} \quad R3^{(5)} := Q(v_{max_EOL0}, D_{pipe}) \left(\frac{gal}{min} \right)^{-1}$$



Worksheet

return summary results
of analysis to worksheet

(R0 R1 R2 R3)

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WO/Job No. 110299 & 110300/BA10

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Date: 03/08/2000

By: L. J. Julvik

Waste Tank Feed Transfer System

Checked: 03/10/2000

By: T. C. Oten

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TOL = 1×10^{-6}

Figure F-1a. Viscosity of Water vs. Temperature.

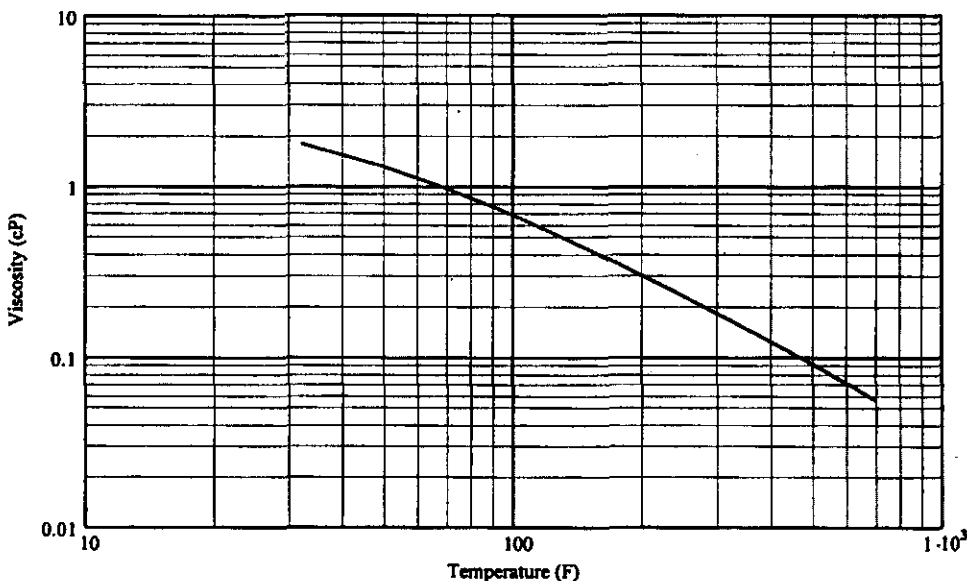
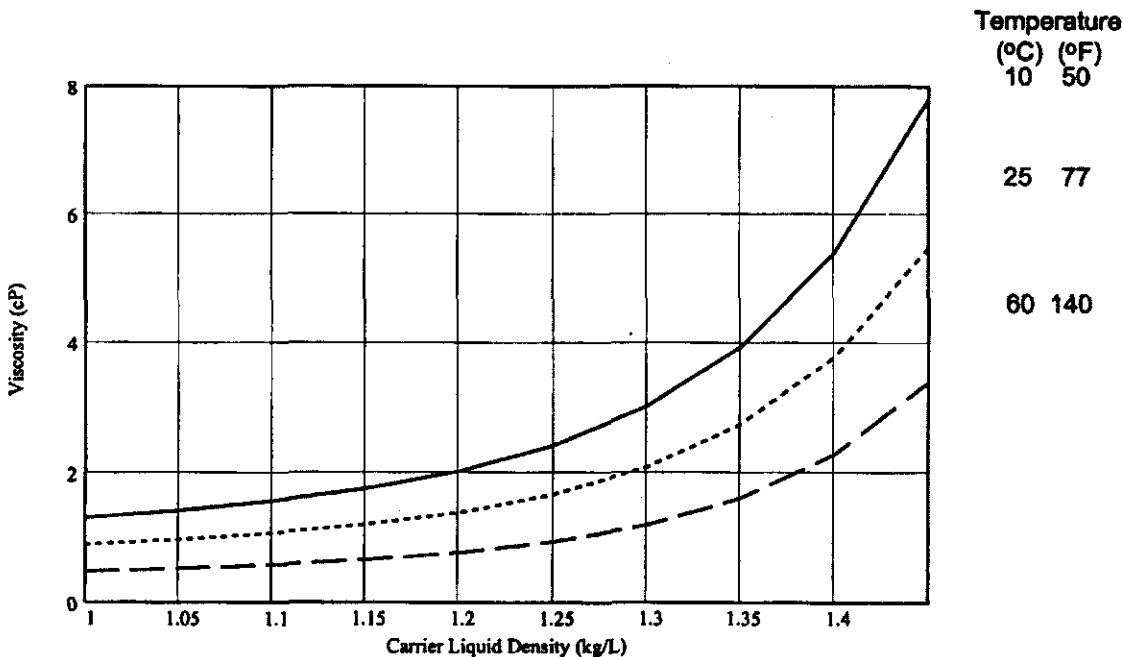


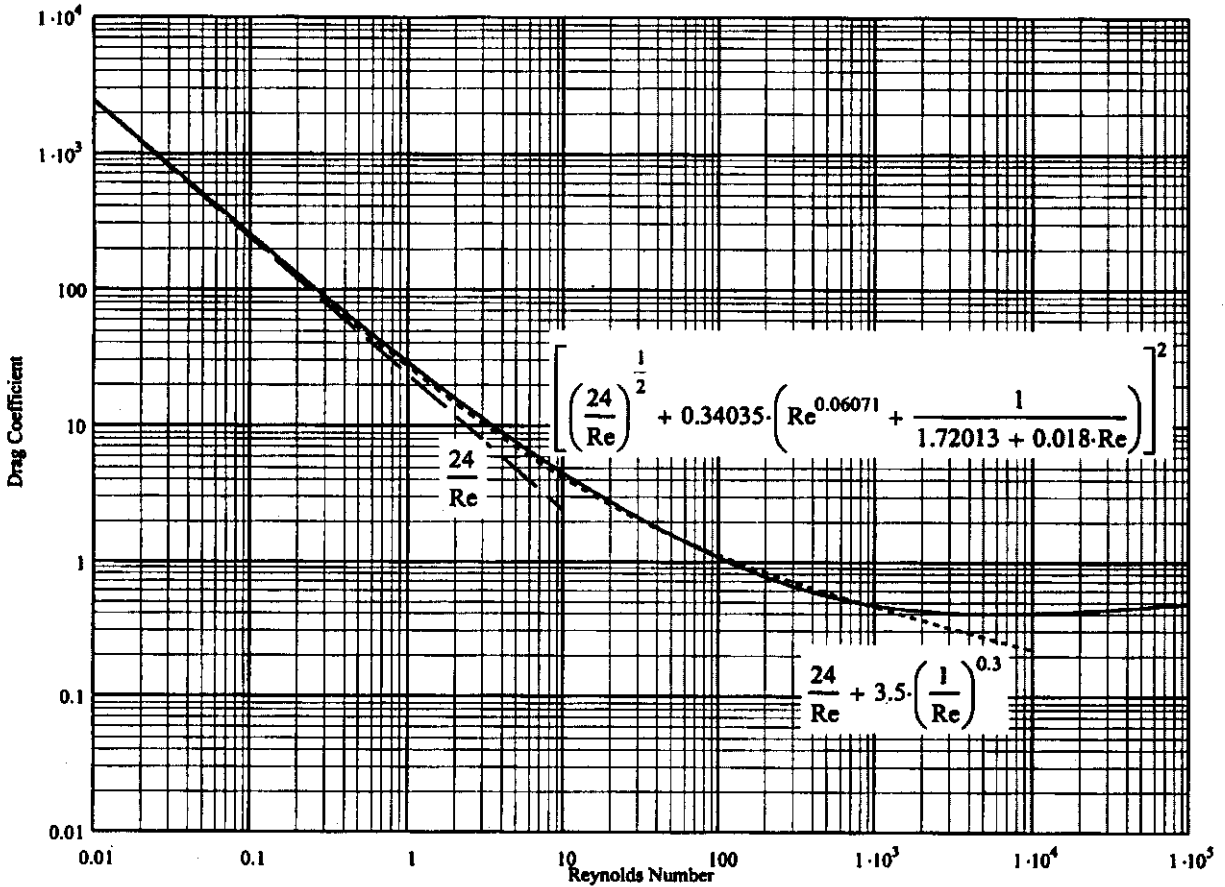
Figure F-1b. Viscosity of Hanford Waste vs. Density of Carrier Liquid at Selected Temperatures.



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Figure F-2. Correlations for the Drag Coefficient of a Sphere in Free Fall in a Stagnant Unbound Liquid.



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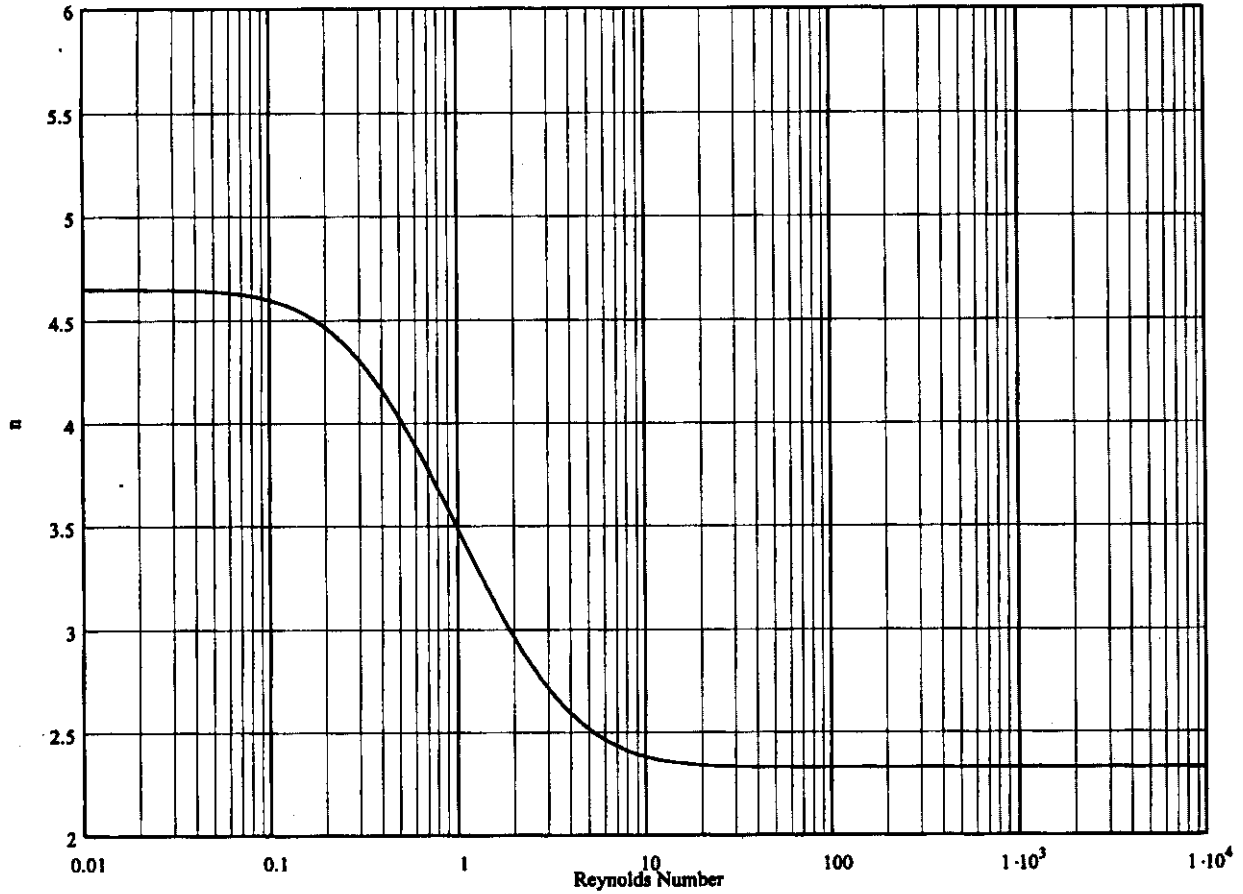
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Figure F-3. Hindering Settling Velocity Exponent as a Function of Particle Reynolds Number.

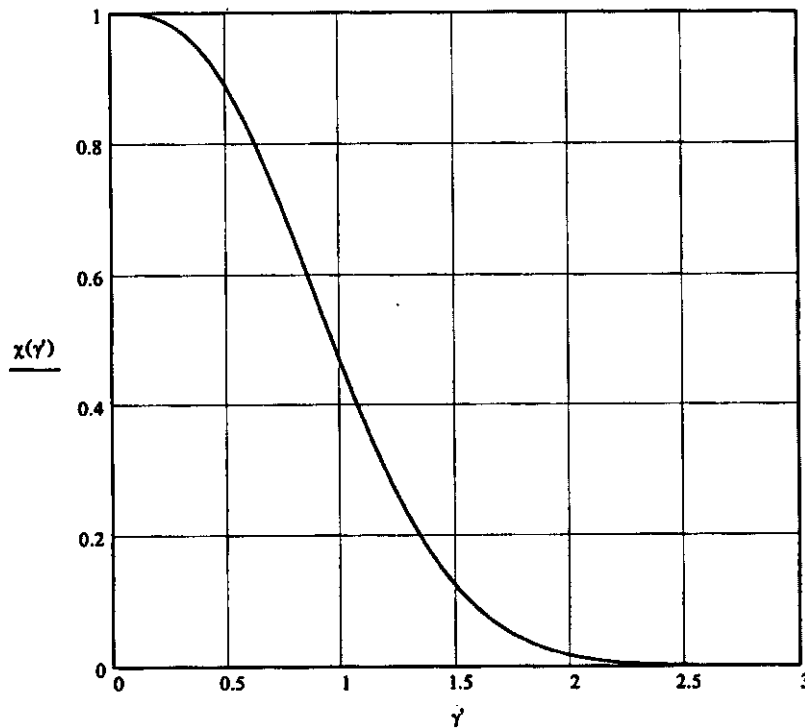


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Figure F-4. Plot of $\chi(\gamma)$.



$D_d := 400 \cdot \mu\text{m}$

$\alpha_d := 10\%$

$D_{\text{pipe}} := 3.068 \cdot \text{in}$

$K_{cv} := 1$

$N := 50 \quad i := 0..N \quad j := 0..N$

$\rho_{dLL} := 1.401 \cdot \frac{\text{kg}}{\text{liter}} \quad \rho_{dUL} := 4 \cdot \frac{\text{kg}}{\text{liter}} \quad \Delta\rho_d := \frac{\rho_{dUL} - \rho_{dLL}}{N}$

$\rho_{d_j} := \rho_{dLL} + \Delta\rho_d \cdot j$

$\rho_{cLL} := 1 \cdot \frac{\text{kg}}{\text{liter}} \quad \rho_{cUL} := 1.4 \cdot \frac{\text{kg}}{\text{liter}} \quad \Delta\rho_c := \frac{\rho_{cUL} - \rho_{cLL}}{N}$

$\rho_{c_i} := \rho_{cLL} + \Delta\rho_c \cdot i$

$T := 60 \quad \mu_{c_i} := \mu_{cL}(\rho_{c_i}, T)$

$v_{\text{cr60}_{i,j}} := v_{\text{crOTt}}(D_{\text{pipe}}, D_d, \rho_{d_j}, \alpha_d, \rho_{c_i}, \mu_{c_i}, K_{cv}) \quad v_{\text{cr60}_{i,j}} := v_{\text{crOTe}}(D_{\text{pipe}}, D_d, \rho_{d_j}, \alpha_d, \rho_{c_i}, \mu_{c_i}, K_{cv})$

$T := 10 \quad \mu_{c_i} := \mu_{cL}(\rho_{c_i}, T)$

$v_{\text{cr10}_{i,j}} := v_{\text{crOTt}}(D_{\text{pipe}}, D_d, \rho_{d_j}, \alpha_d, \rho_{c_i}, \mu_{c_i}, K_{cv}) \quad v_{\text{cr10}_{i,j}} := v_{\text{crOTe}}(D_{\text{pipe}}, D_d, \rho_{d_j}, \alpha_d, \rho_{c_i}, \mu_{c_i}, K_{cv})$

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Figure F-5a. Contour Plot of Oroskar and Turian (1980) Semi-Theoretical Equation for the Critical Velocity (ft/s) vs. Particle Solids Density and Carrier Liquid Density - T=60°C (solid line), T=10°C (dash line).

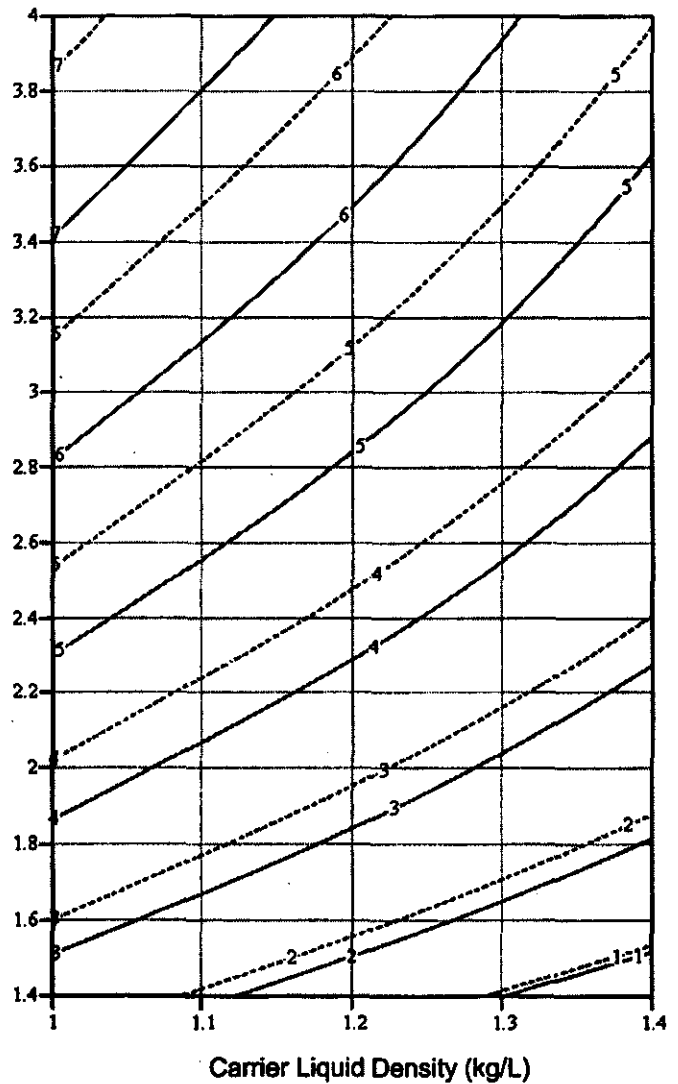
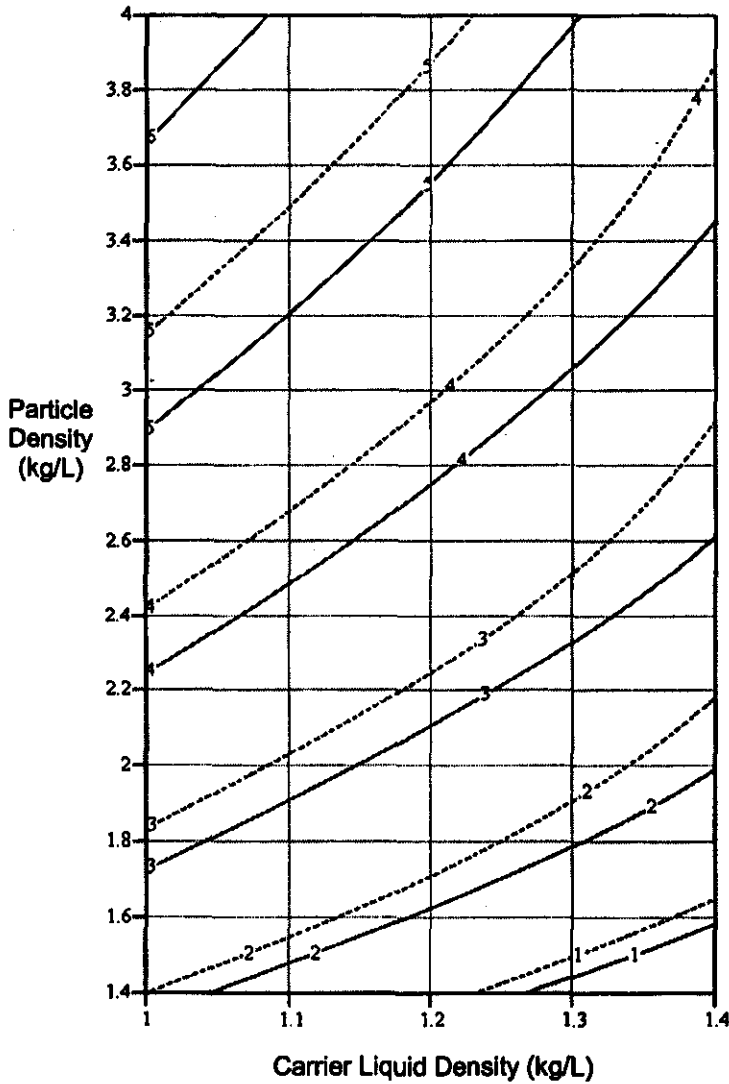
Figure F-5b. Contour Plot of Oroskar and Turian (1980) Empirical Equation for the Critical Velocity (ft/s) vs. Particle Solids Density and Carrier Liquid Density - T=60°C (solid line), T=10°C (dash line).

$K_{cv} = 1$

$D_{pipe} = 3.068 \text{ in}$

$D_d = 400 \mu\text{m}$

$\alpha_d = 10\%$



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Figure F-6a. Best-Estimate Critical Velocity Prediction vs. Carrier Liquid Density at Selected Solids Volume Fractions - Oroskar and Turian (1980).

$T := 60$ °C $\beta_{cv} := 1.2$ $\kappa_{cv} := 1$ $D_{pipe} = 3.068$ in $D_d := 400$ - μ m $\rho_d := 3 \cdot \frac{kg}{liter}$

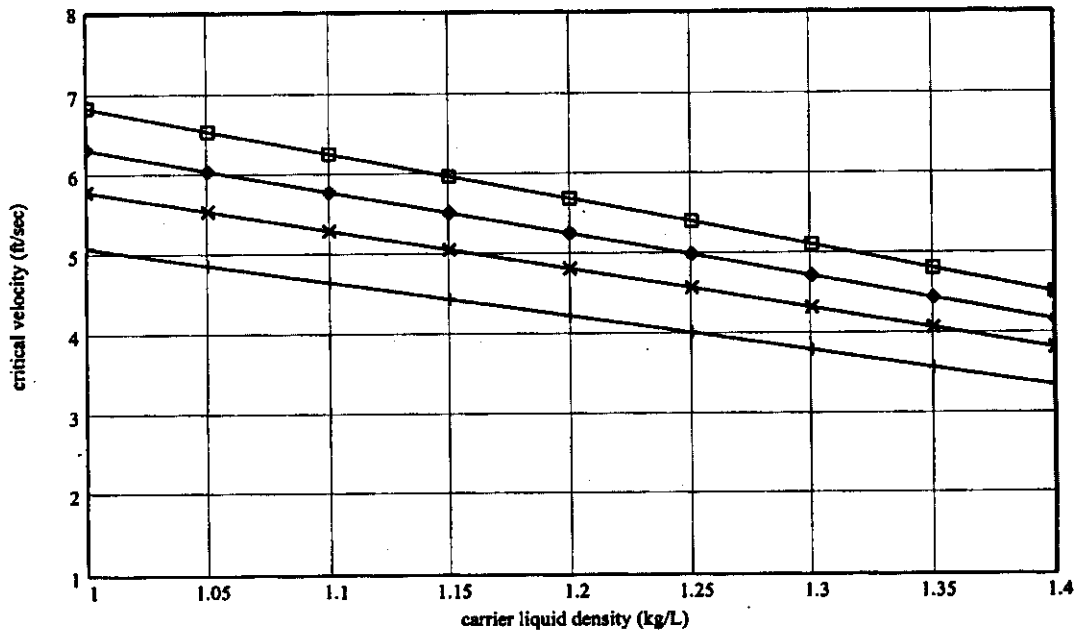
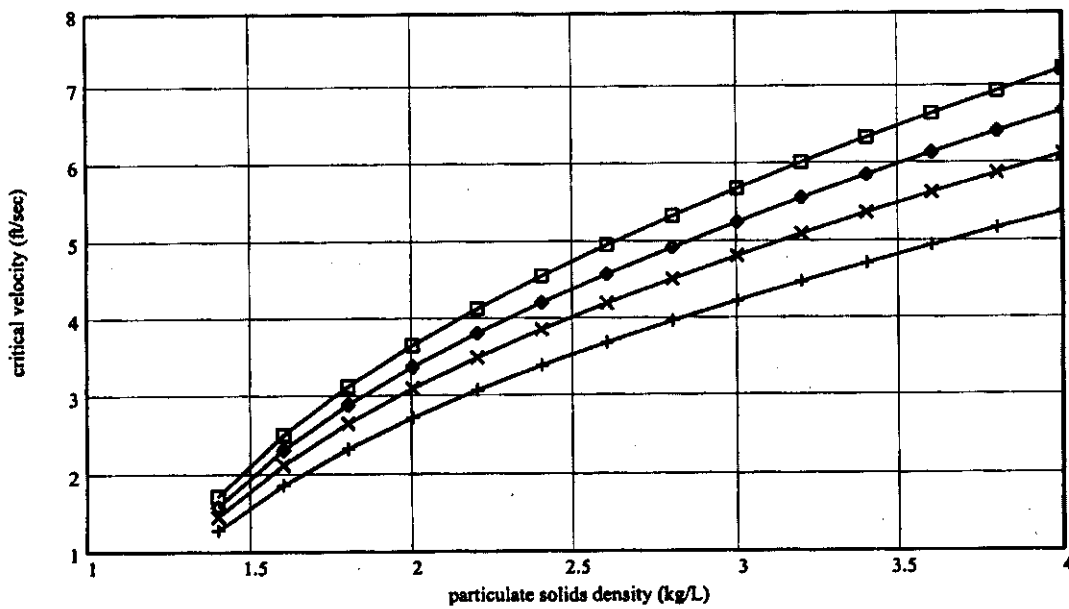


Figure F-8b. Best-Estimate Critical Velocity Prediction vs. Particle Solids Density at Selected Solids Volume Fractions - Oroskar and Turian (1980).



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Figure F-7a. Best-Estimate Critical Velocity Prediction vs. Carrier Liquid Density at Selected Solids Volume Fractions - Oroskar and Turlan (1980).

T = 60 °C

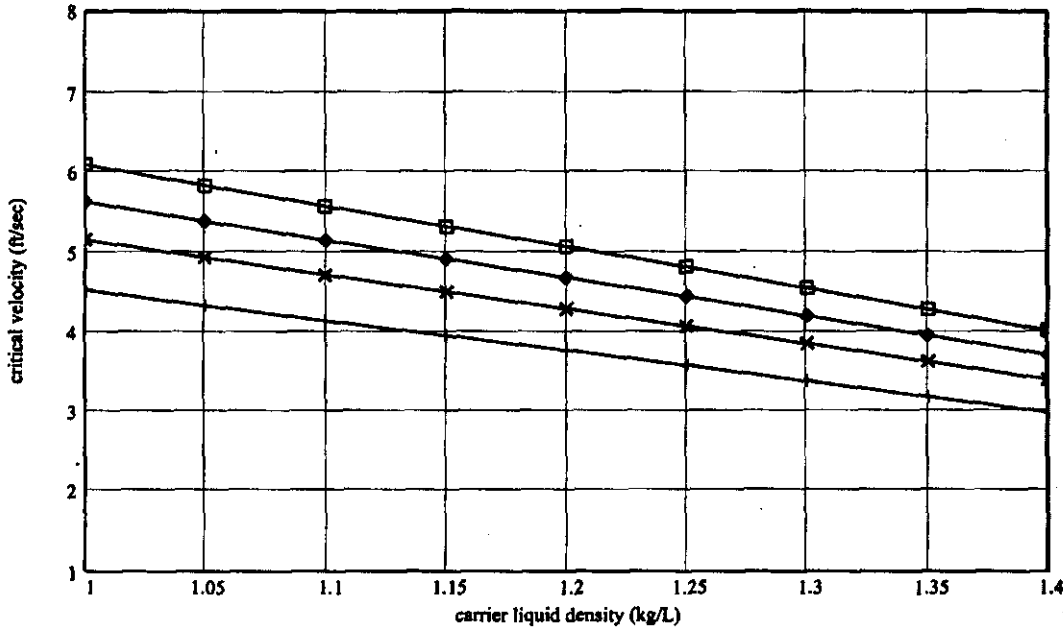
$\beta_{cv} = 1.2$

$\kappa_{cv} = 1$

$D_{pipe} = 3.068$ in

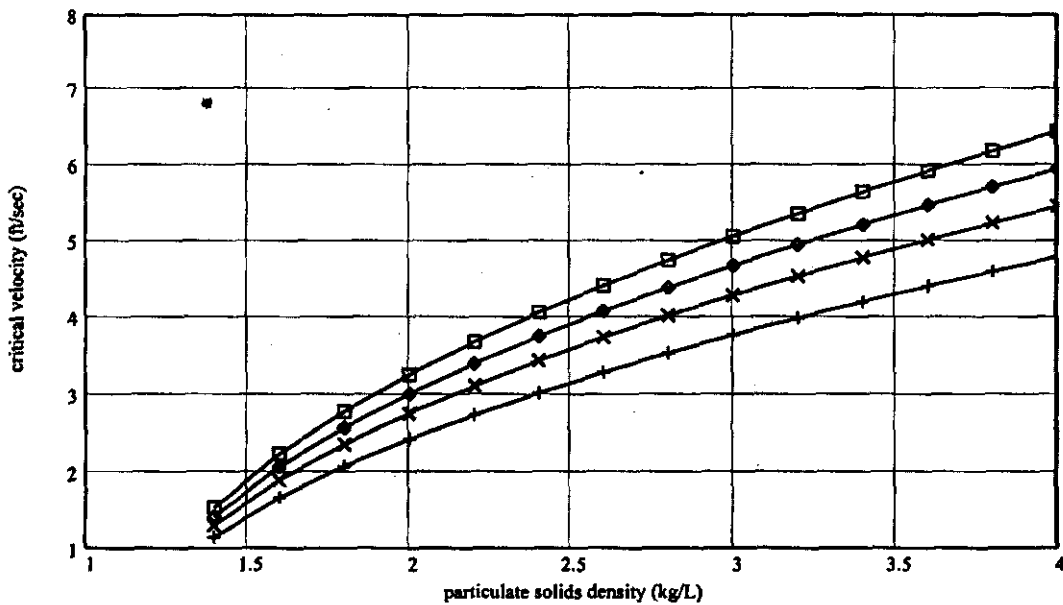
$D_d := 200\text{-}\mu\text{m}$

$\rho_d := 3 \frac{\text{kg}}{\text{liter}}$



α_d
0.30
0.10
0.05
0.02

Figure F-7b. Best-Estimate Critical Velocity Prediction vs. Particle Solids Density at Selected Solids Volume Fractions - Oroskar and Turlan (1980).



α_d
0.30
0.10
0.05
0.02

$\rho_c := 1.2 \frac{\text{kg}}{\text{liter}}$

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Figure F-8a. Best-Estimate Critical Velocity Prediction vs. Carrier Liquid Density at Selected Solids Volume Fractions - Oroskar and Turian (1980).

T = 60 PC

$\beta_{cv} = 1.2$

$\kappa_{cv} = 1$

$D_{pipe} = 3.068$ in

$D_d := 100\text{-}\mu\text{m}$

$\rho_d := 3 \cdot \frac{\text{kg}}{\text{liter}}$

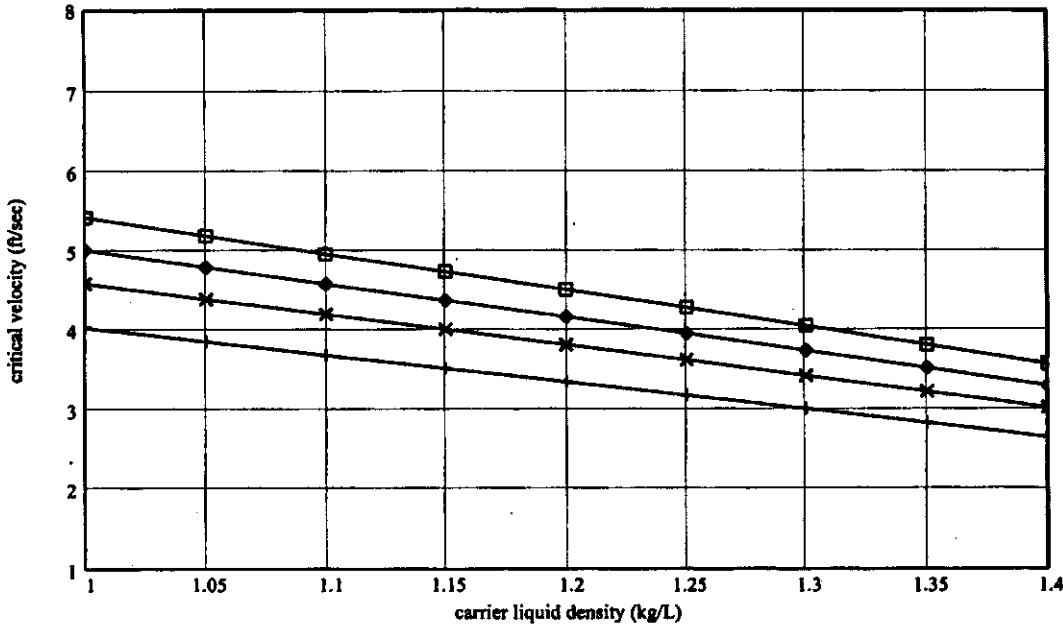
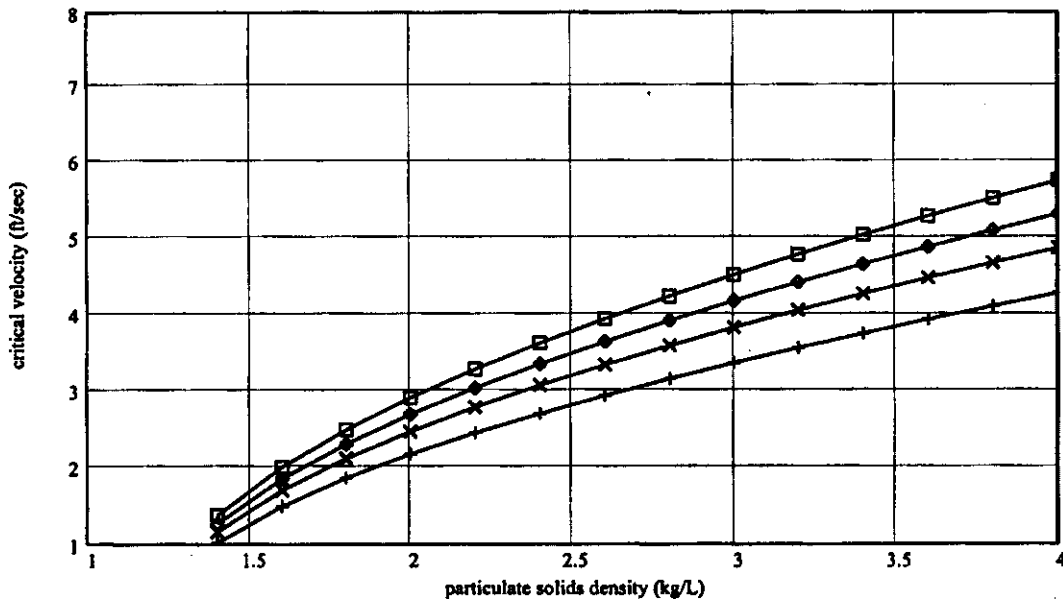


Figure F-8b. Best-Estimate Critical Velocity Prediction vs. Particle Solids Density at Selected Solids Volume Fractions - Oroskar and Turian (1980).

$\rho_c := 1.2 \cdot \frac{\text{kg}}{\text{liter}}$



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Figure F-9a. Best-Estimate Critical Velocity Prediction vs. Particle Solids Median Diameter at Selected Solids Volume Fractions - Oroskar and Turian (1980).

$T = 60$ °C $\beta_{cv} = 1.2$ $\kappa_{cv} = 1$ $D_{pipe} = 3.068$ in $\rho_c := 1.2 \frac{kg}{liter}$ $\rho_d := 3 \frac{kg}{liter}$

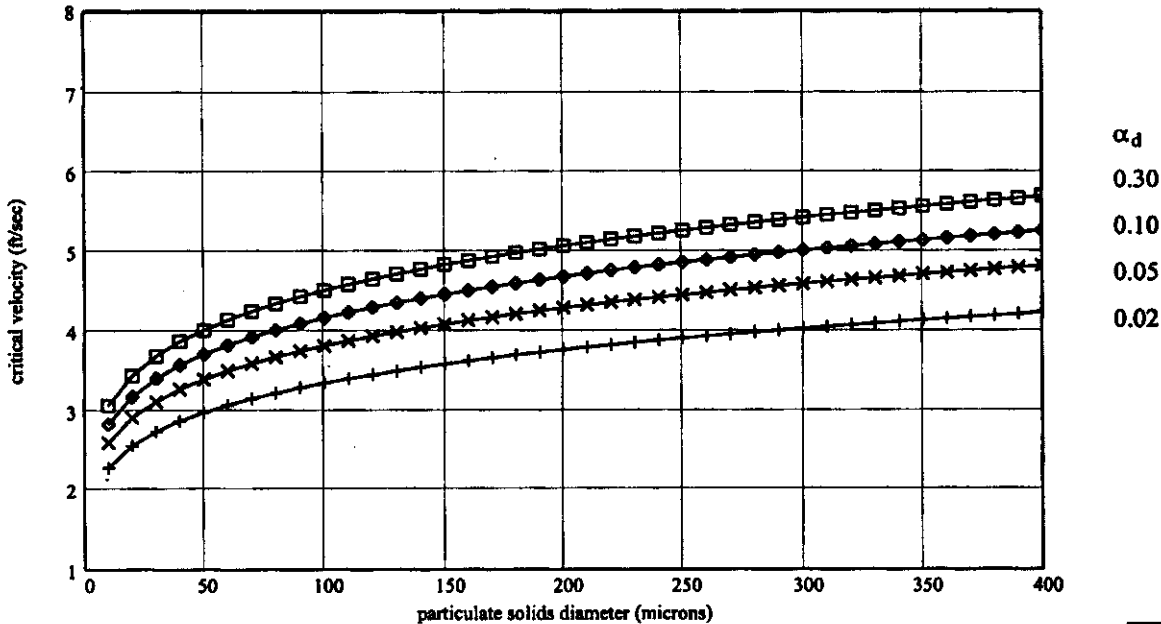
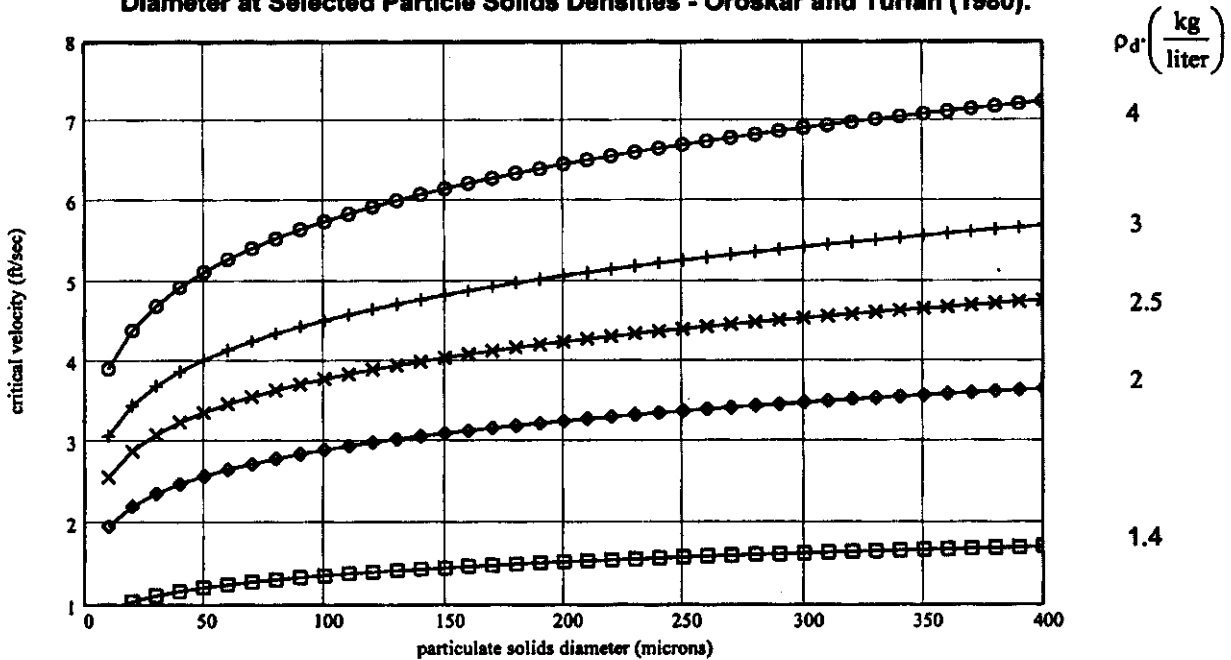


Figure F-9b. Best-Estimate Critical Velocity Prediction vs. Particle Solids Median Diameter at Selected Particle Solids Densities - Oroskar and Turian (1980).

$\alpha_d := 0.30$



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Figure F-9c. Best-Estimate Critical Velocity Prediction vs. Particle Solids Median Diameter at Selected Particle Solids Densities - Oroskar and Turian (1980).

$T = 60$ °C $\beta_{cv} = 1.2$ $\kappa_{cv} = 1$ $D_{pipe} = 3.068$ in $\rho_c := 1.2 \frac{kg}{liter}$ $\alpha_d := 0.10$

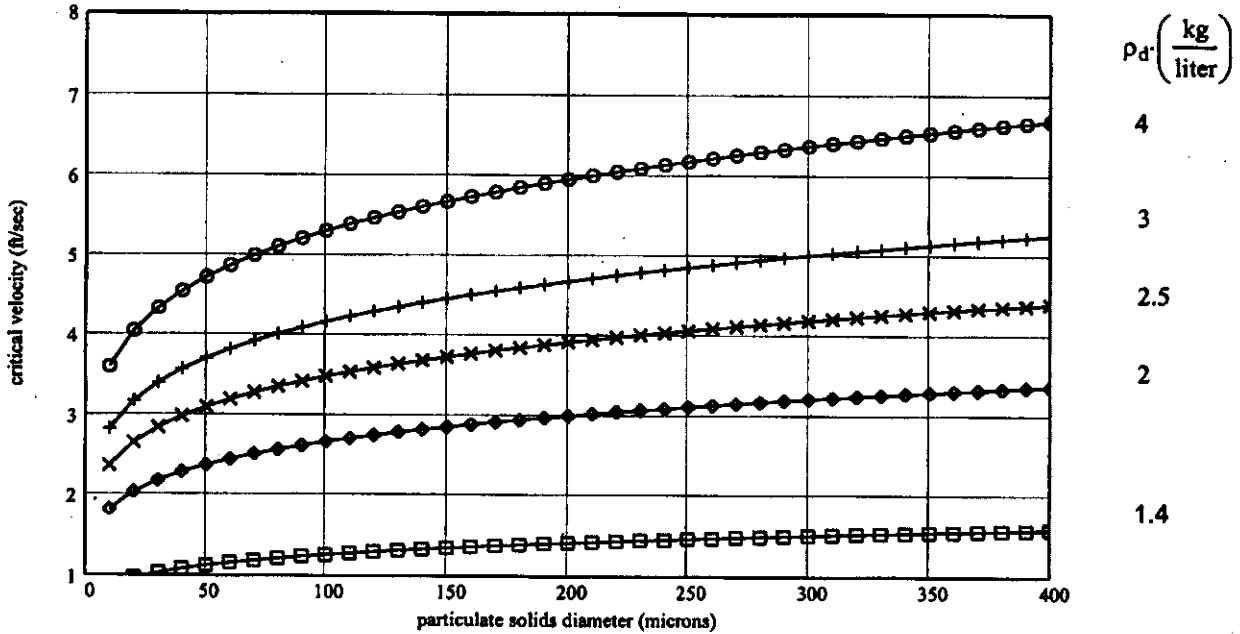
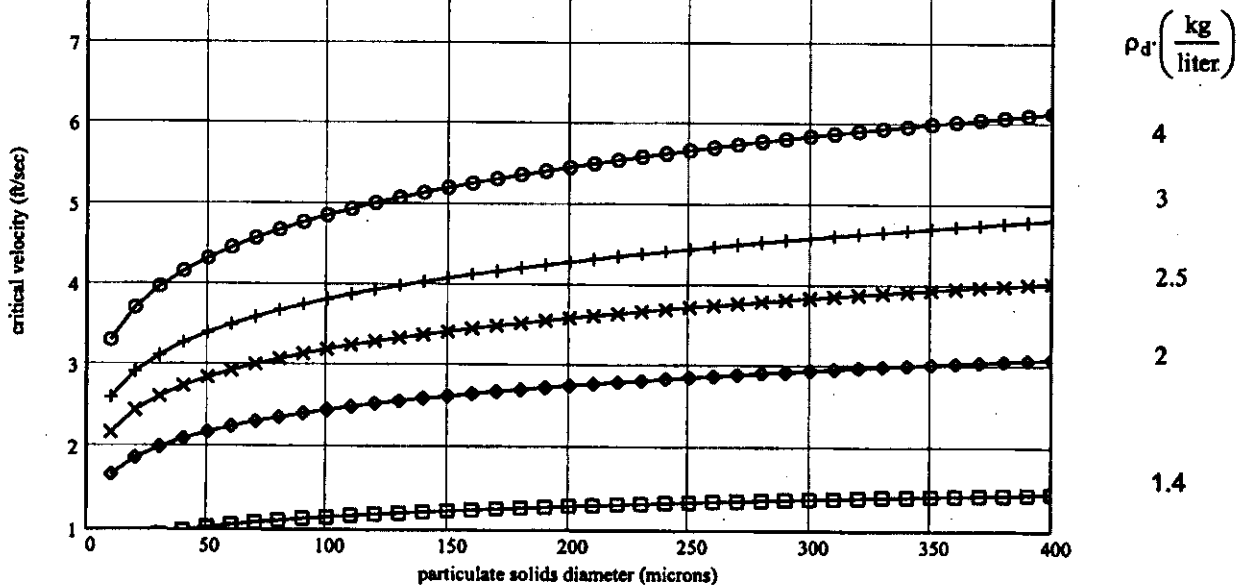


Figure F-9d. Best-Estimate Critical Velocity Prediction vs. Particle Solids Median Diameter at Selected Particle Solids Densities - Oroskar and Turian (1980).

$\alpha_d := 0.05$



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Figure F-10a. Best-Estimate Critical Velocity Prediction vs. Carrier Liquid Density at Selected Solids Volume Fractions - Oroskar and Turlan (1980).

$T := 10$ °C

$\beta_{cv} = 1.2$

$\kappa_{cv} = 1$

$D_{pipe} = 3.068$ in

$\rho_d := 3 \frac{kg}{liter}$

$D_d := 400 \mu m$

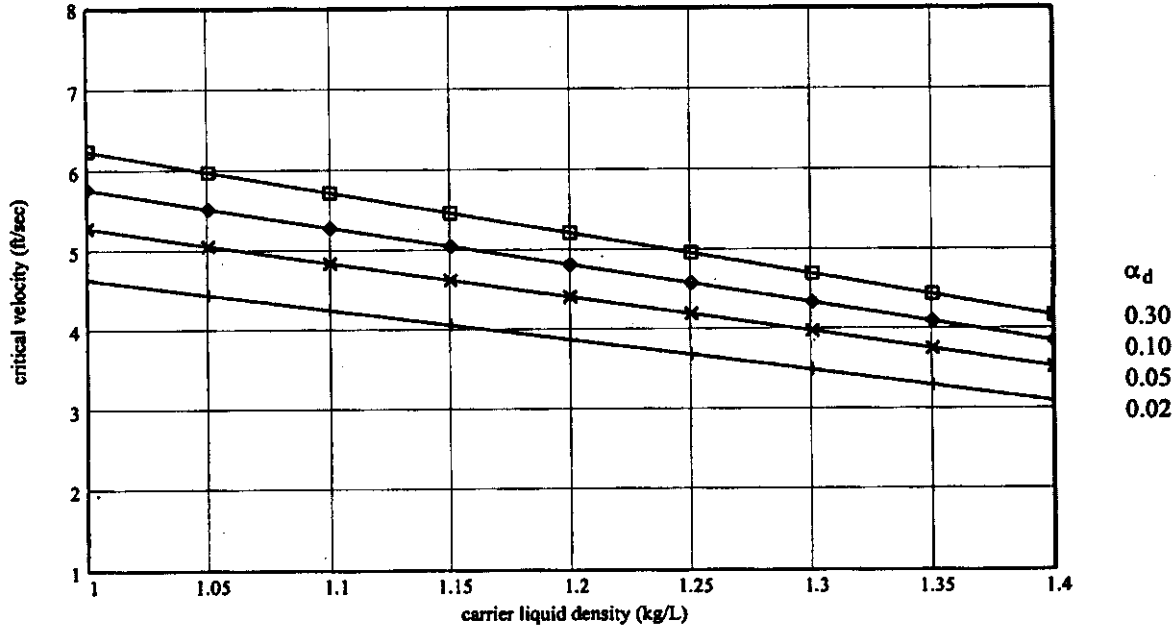
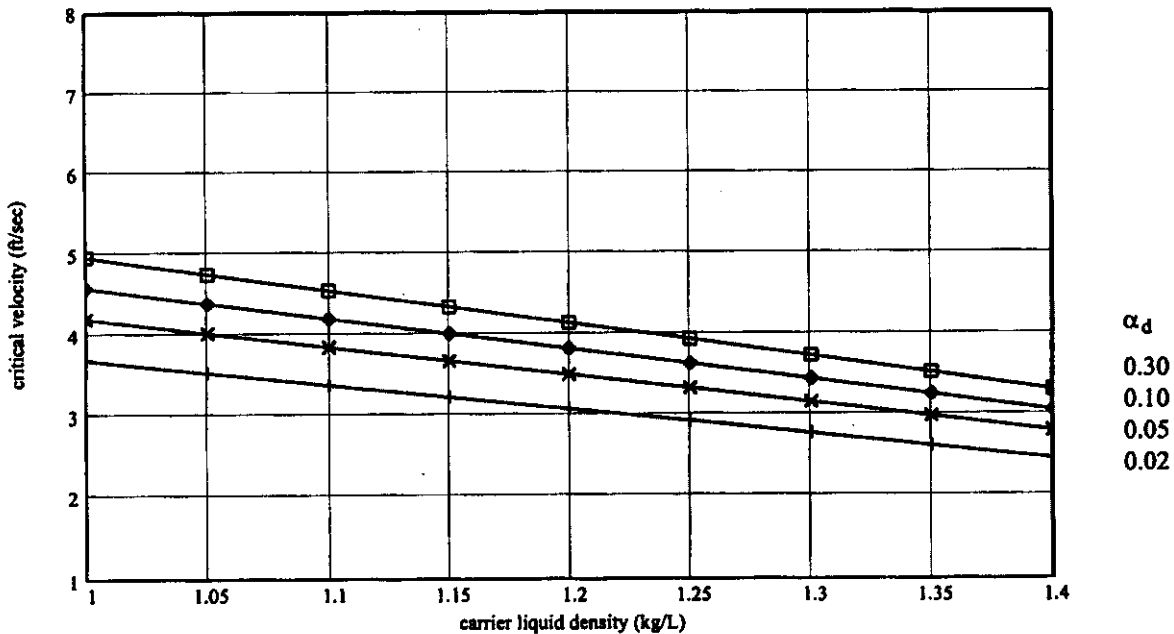


Figure 10b. Best-Estimate Critical Velocity Prediction vs. Carrier Liquid Density at Selected Solids Volume Fractions - Oroskar and Turlan (1980).

$D_d := 100 \mu m$



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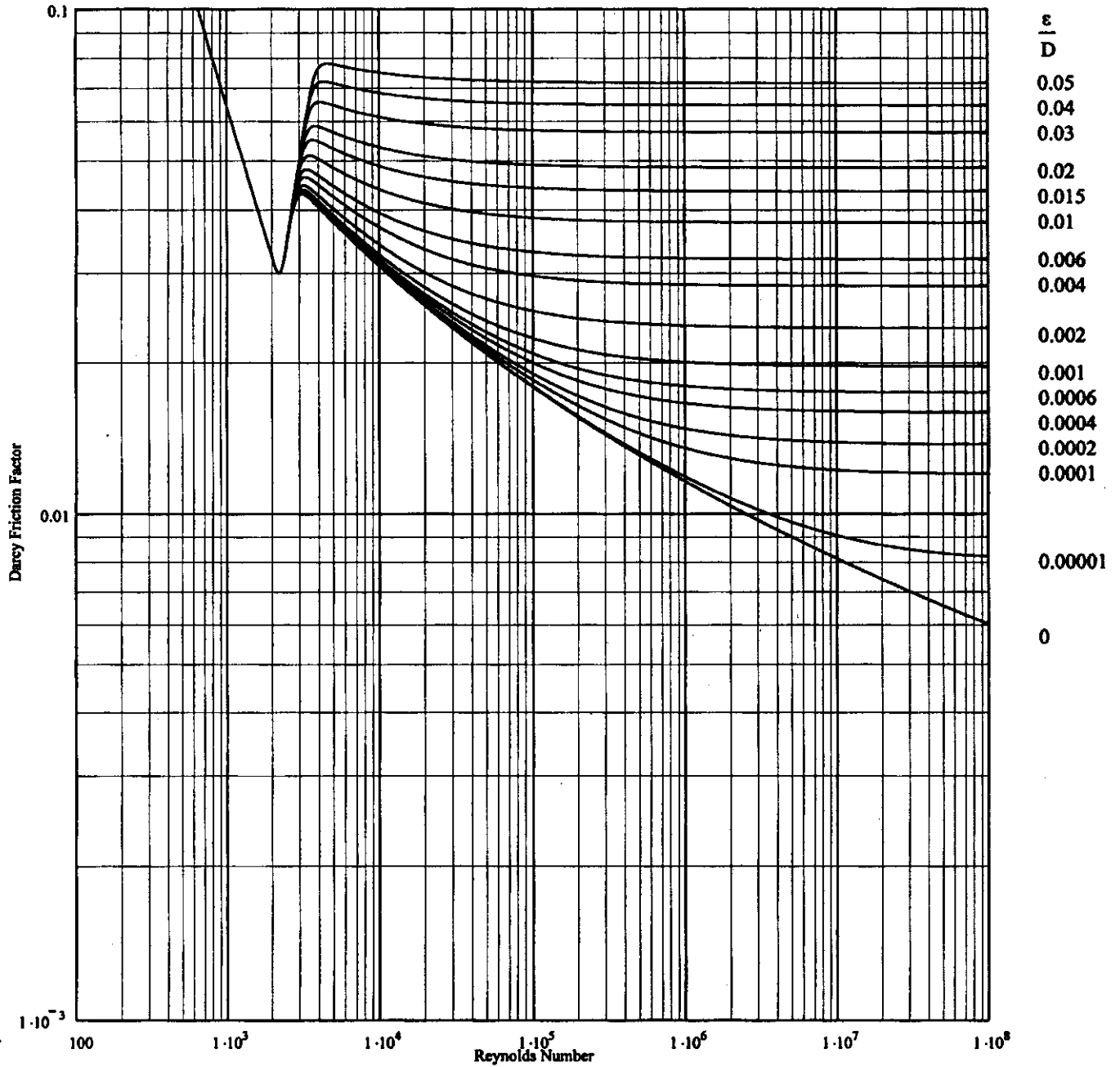
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Figure F-11. Darcy Friction Factor vs. Reynolds Number and Relative Roughness.



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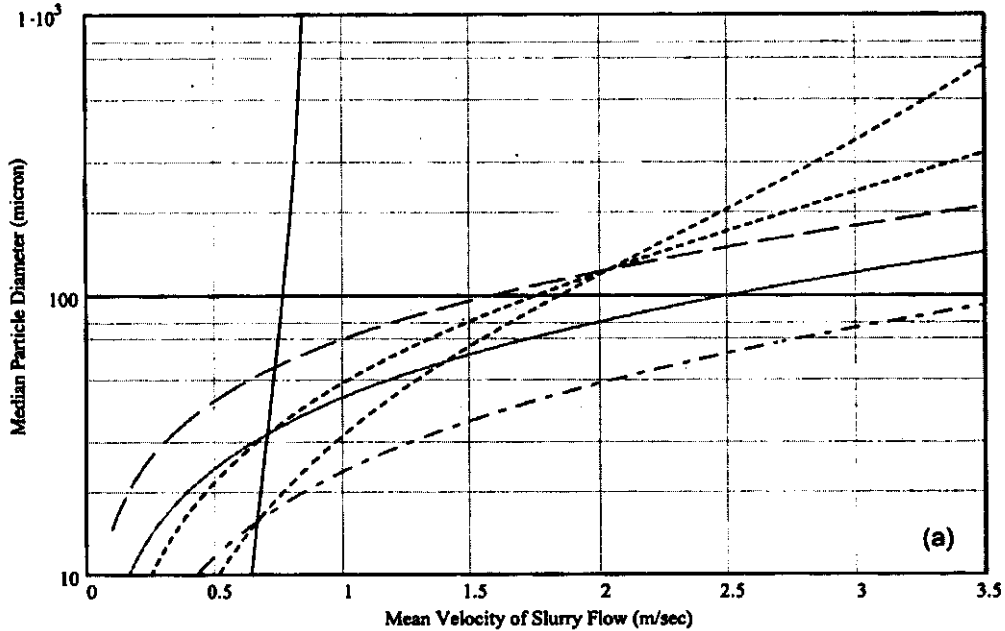
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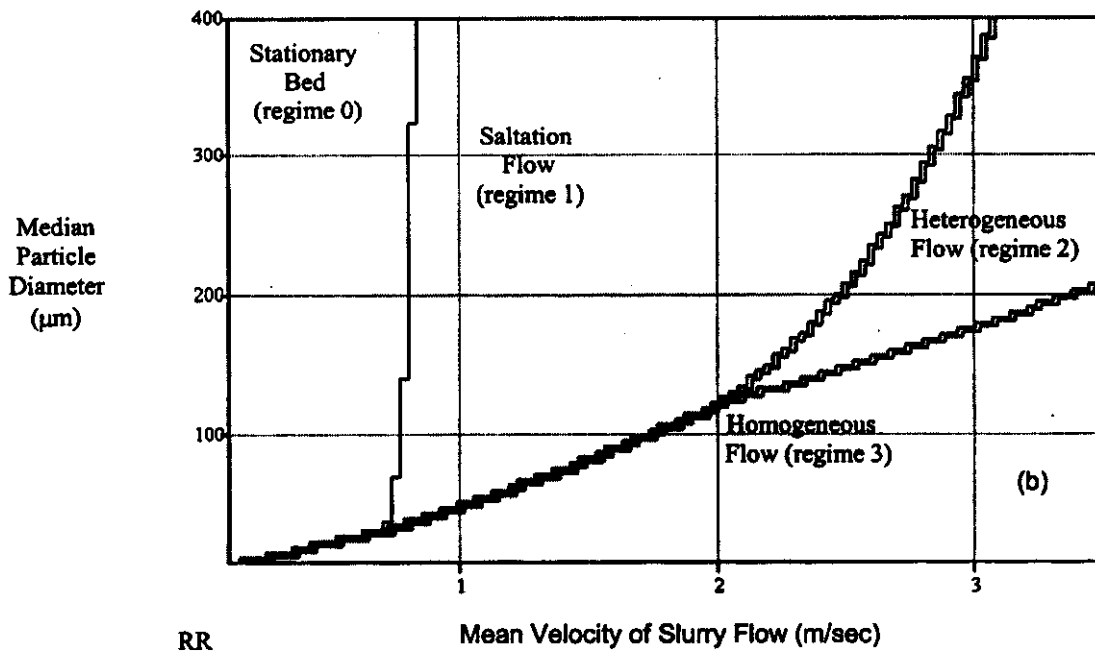
$$D_{\text{pipe}} := 5.25 \text{ cm}$$

Figure F-13. Flow Regime Diagram for 2-in. Pipe - Check Case.

$$R_{01}=1$$



$$RR_{i,j} := R_d(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon, D_d, \rho_d, \alpha_d, 1)$$



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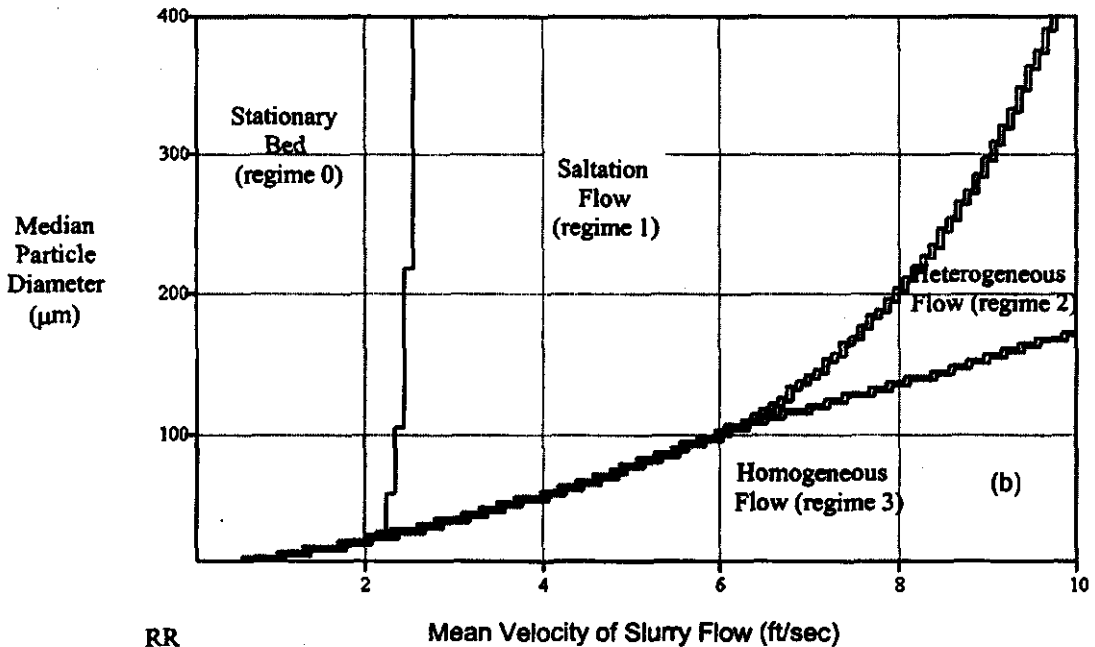
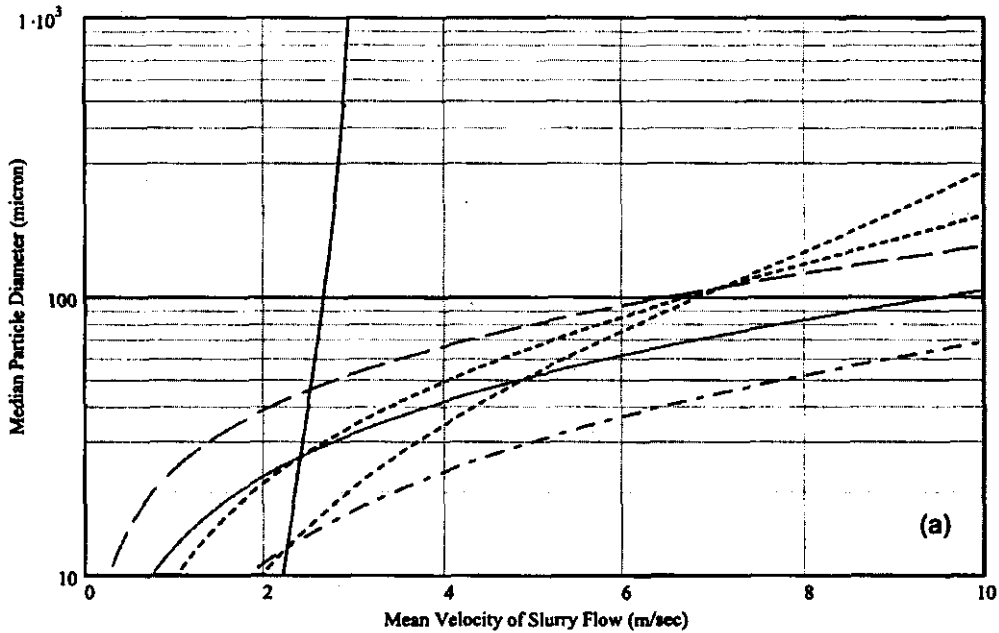
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Figure F-14. Flow Regime Diagram for 3-in. Pipe.

$T := 60$ °C $\rho_c := 1.2 \frac{\text{kg}}{\text{liter}}$ $\mu_c := \mu_{cL}(\rho_c, T)$ $\mu_c = 0.771 \text{ cP}$ $\rho_d := 3 \frac{\text{kg}}{\text{liter}}$ $\alpha_d := 5\%$ $\epsilon := \epsilon_{\text{new}}$ $\beta_f := 1$
 $D_{\text{pipe}} := 3.068 \text{ in}$ $R_{01} = 1$ $\epsilon = 2 \text{ mil}$ $\kappa_f := 1$



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$T = 60 \text{ } ^\circ\text{C}$ $\rho_c = 1.2 \frac{\text{kg}}{\text{liter}}$

$\mu_c = 0.771 \text{ cP}$ $\rho_d = 3 \frac{\text{kg}}{\text{liter}}$

$\alpha_d := 10\%$

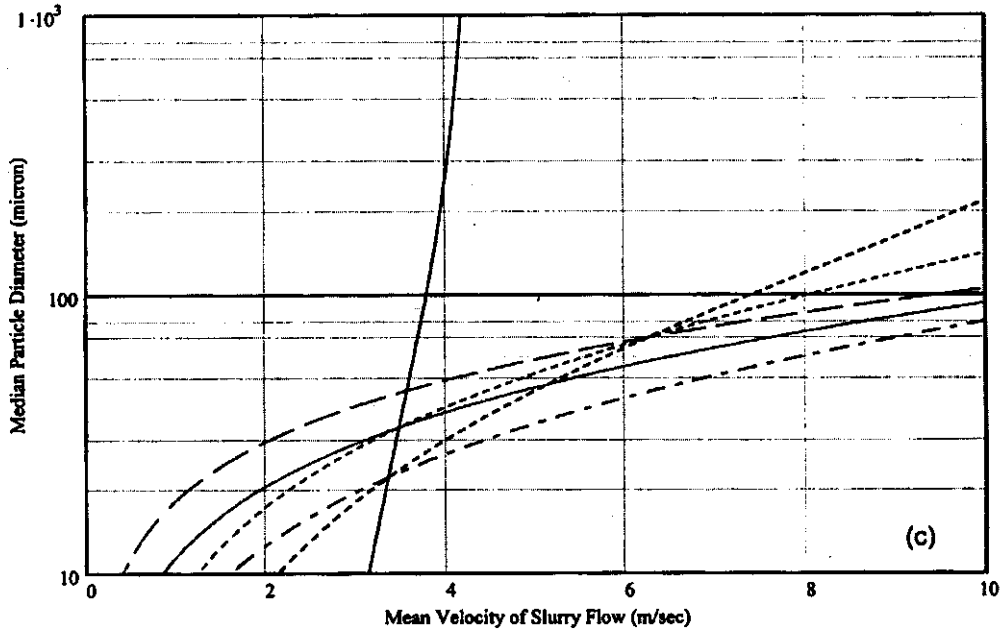
$\epsilon = 2 \text{ mil}$

$\beta_f = 1$

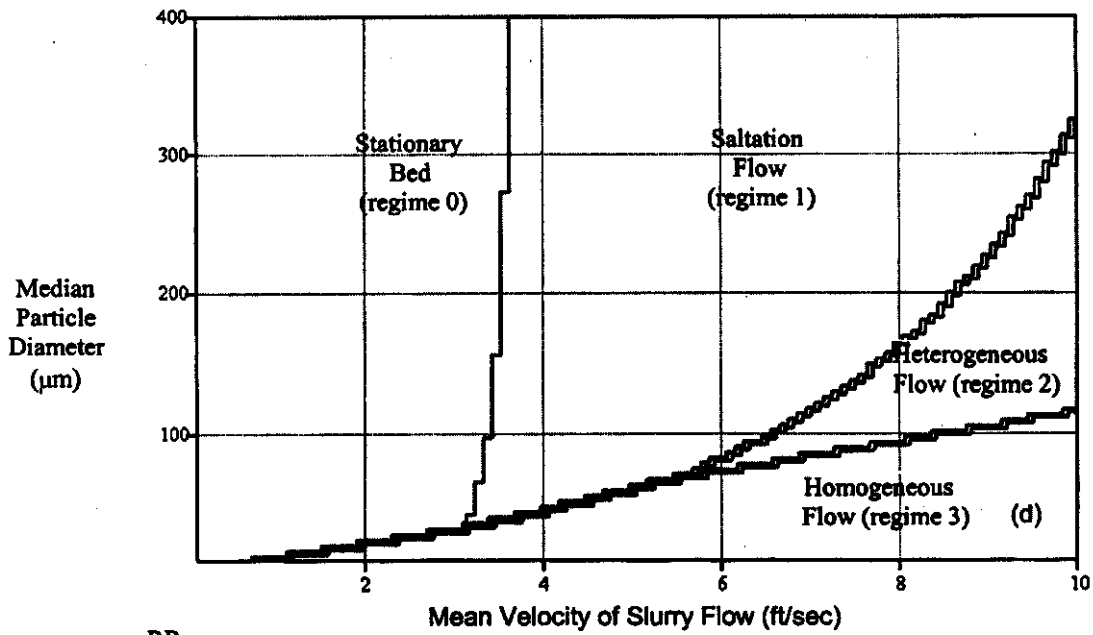
$D_{\text{pipe}} = 3.068 \text{ in}$

$R_{01} = 1$

$\kappa_f = 1$



$R_{12} = 1$
 $R_{13} = 1$
 $R_{02} = 1$
 $R_{03} = 1$ $R_{23} = 1$



RR

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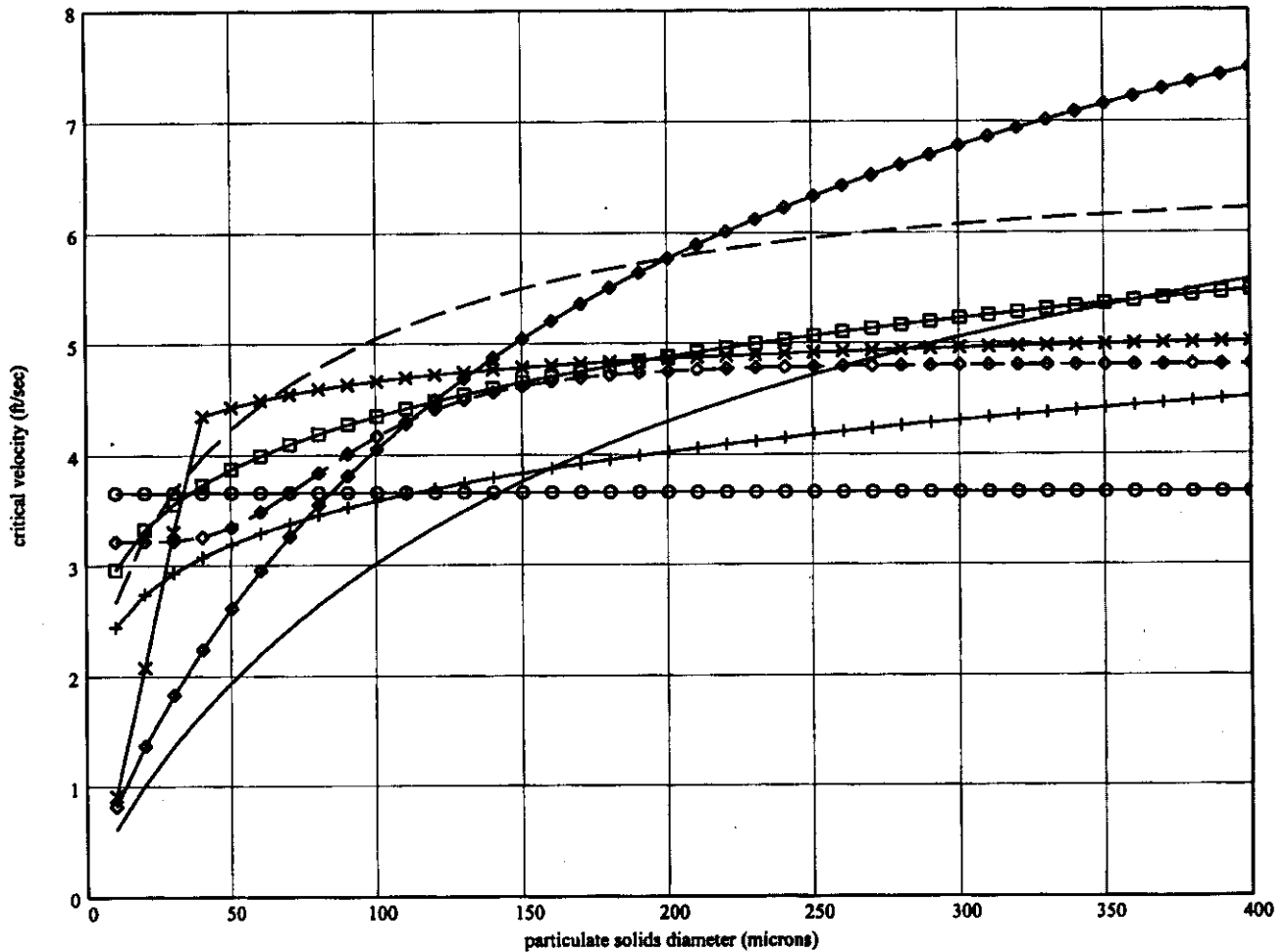
By: T. C. Oter

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By: _____

Figure F-15a. Comparison of Best-Estimate Critical Velocity Predictions vs. Particle Solids Median Diameter.

$T := 60 \text{ } ^\circ\text{C}$ $\kappa_{cv} = 1$ $D_{\text{pipe}} := 3.068\text{-in}$ $\rho_c := 1.2 \frac{\text{kg}}{\text{liter}}$ $\mu_c := \mu_{cL}(\rho_c, T)$ $\mu_c = 0.771 \text{ cP}$ $\rho_d := 3 \frac{\text{kg}}{\text{liter}}$ $\alpha_d := 15\%$



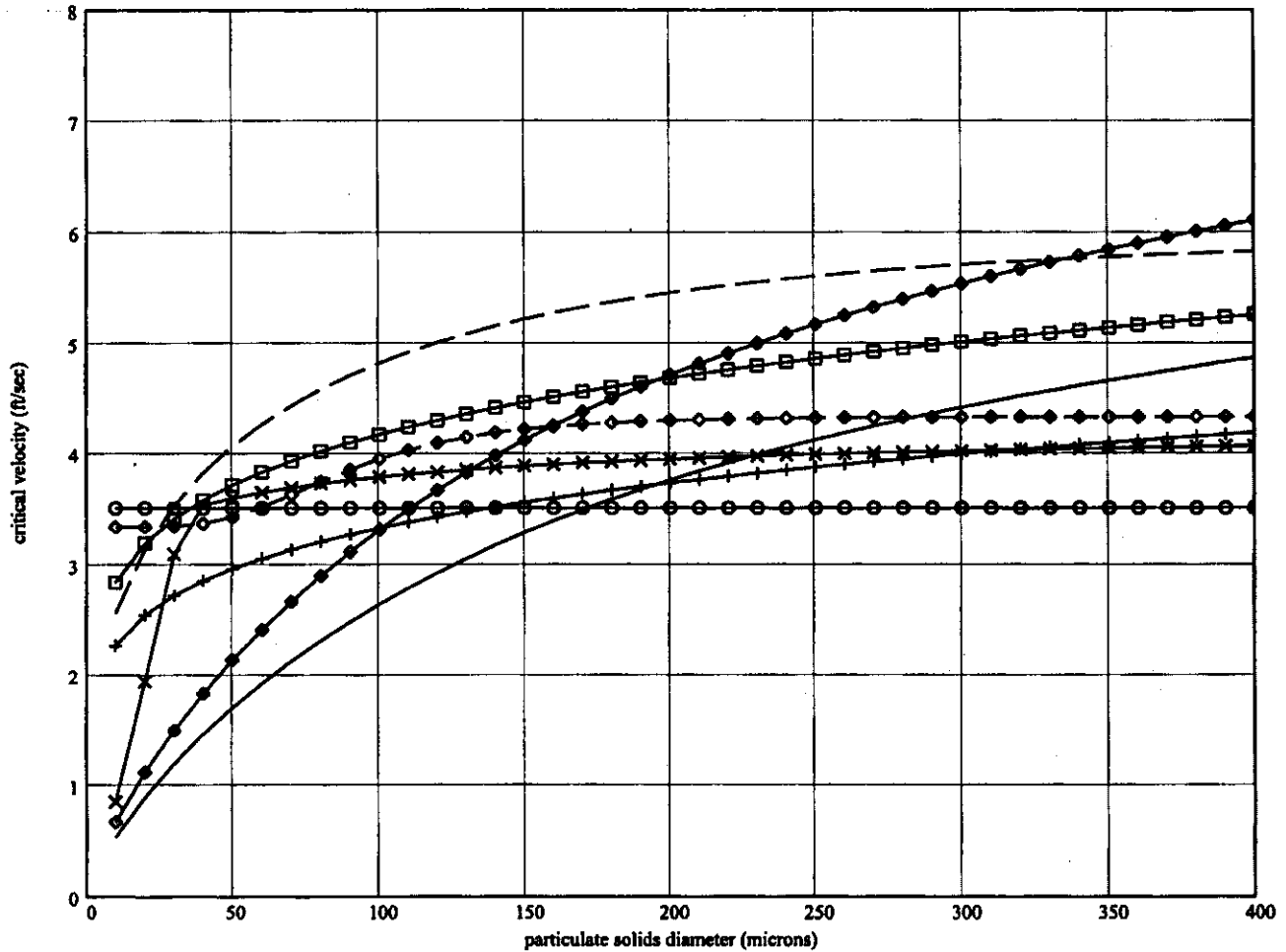
- ▣▣▣ Oroskar and Turian (1980) empirical
- ◆ - Oroskar and Turian (1980) semi-theoretical
- *** Turian, Hsu, and Ma (1987) (based on regime transition)
- +++ Wasp et al. (1977)
- Robinson and Graf (1972)
- Zandi and Govatos (1967)
- Shook (1969)
- - Gogus and Kokpinar (1999)

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Figure F-15b. Comparison of Best-Estimate Critical Velocity Predictions vs. Particle Solids Median Diameter.

$T = 60 \text{ }^\circ\text{C}$ $\kappa_{cv} = 1$ $D_{\text{pipe}} = 3.068 \text{ in}$ $\rho_c = 1.2 \frac{\text{kg}}{\text{liter}}$ $\mu_c = 0.771 \text{ cP}$ $\rho_d = 3 \frac{\text{kg}}{\text{liter}}$ $\alpha_d := 10\%$



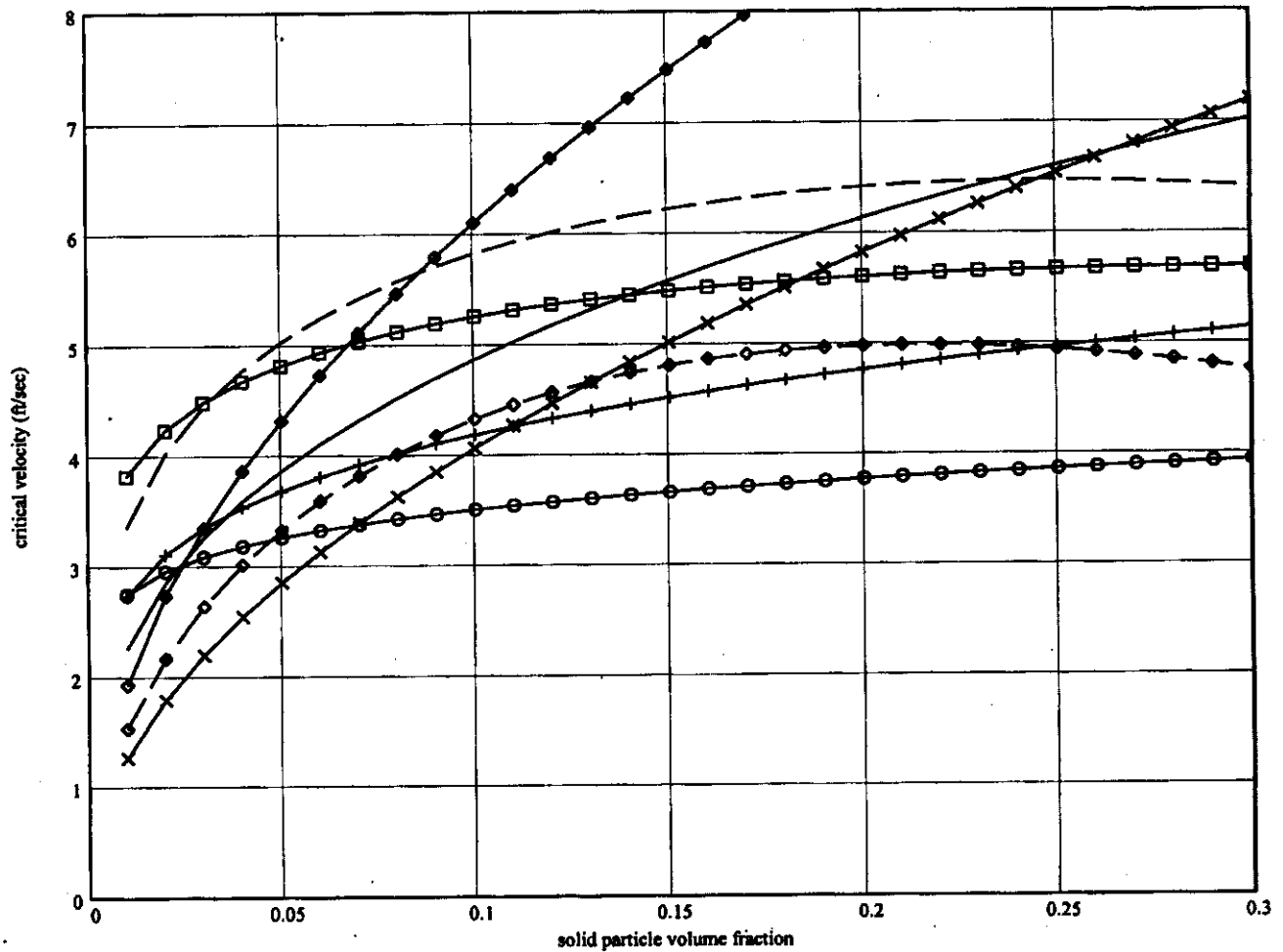
- Oroskar and Turian (1980) empirical
- ◆◆◆ Oroskar and Turian (1980) semi-theoretical
- ××× Turian, Hsu, and Ma (1987) (based on regime transition)
- +++ Wasp et al. (1977)
- Robinson and Graf (1972)
- ◆◆ Zandi and Govatos (1967)
- Shook (1969)
- Gogus and Kokpinar (1999)

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Figure F-15c. Comparison of Best-Estimate Critical Velocity Predictions vs. Solid Particle Volume Fraction.

$T = 60 \text{ }^\circ\text{C}$ $\kappa_{cv} = 1$ $D_{\text{pipe}} = 3.068 \text{ in}$ $\rho_c = 1.2 \frac{\text{kg}}{\text{liter}}$ $\mu_c = 0.771 \text{ cP}$ $\rho_d = 3 \frac{\text{kg}}{\text{liter}}$ $D_d = 400 \text{ }\mu\text{m}$



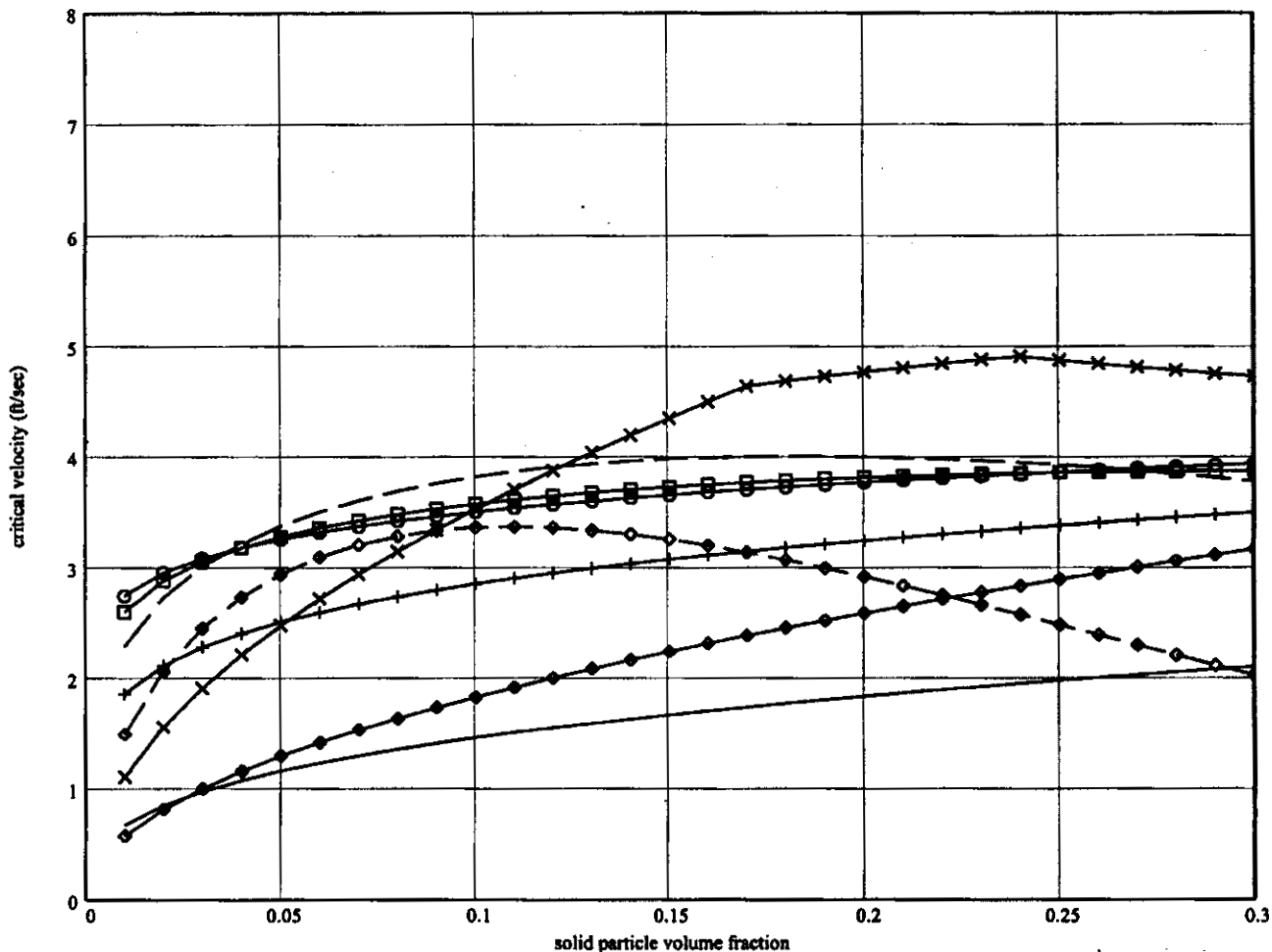
- ☐☐☐ Oroskar and Turian (1980) empirical
- ◆◆◆ Oroskar and Turian (1980) semi-theoretical
- *** Turian, Hsu, and Ma (1987) (based on regime transition)
- +++ Wasp et al. (1977)
- ⊙⊙⊙ Robinson and Graf (1972)
- ◆ Zandi and Govatos (1967)
- Shook (1969)
- Gogus and Kokpinar (1999)

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Figure F-15d. Comparison of Best-Estimate Critical Velocity Predictions vs. Solid Particle Volume Fraction.

T = 60 °C $\kappa_{cv} = 1$ $D_{pipe} = 3.068$ in $\rho_c = 1.2 \frac{kg}{liter}$ $\mu_c = 0.771$ cP $\rho_d = 3 \frac{kg}{liter}$ $D_d := 40\text{-}\mu m$



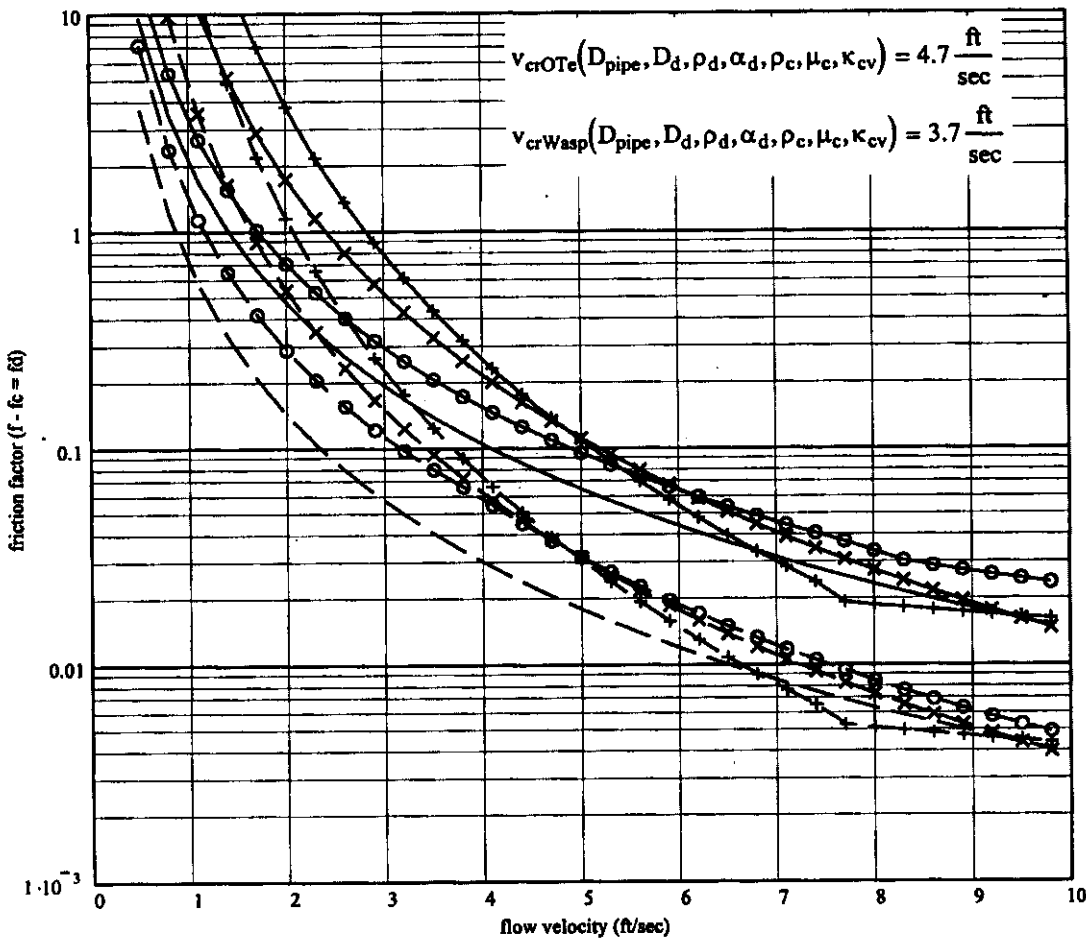
- Oroskar and Turian (1980) empirical
- ◆◆◆ Oroskar and Turian (1980) semi-theoretical
- ×××× Turian, Hsu, and Ma (1987) (based on regime transition)
- +++ Wasp et al. (1977)
- Robinson and Graf (1972)
- ◆◆◆ Zandi and Govatos (1967)
- Shook (1969)
- Gogus and Kokpinar (1999)

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Figure F-16. Δ Friction Factor ($f_d = f - f_c$) Comparison.

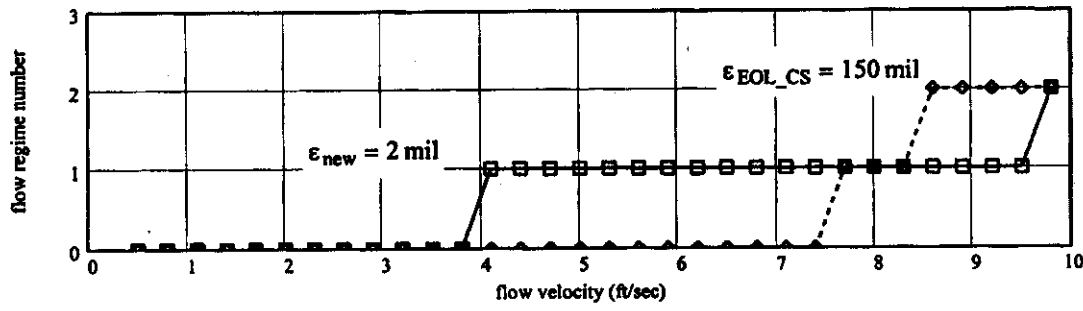
$T = 60 \text{ }^\circ\text{C}$ $D_{\text{pipe}} = 3.068 \text{ in}$ $\rho_c := 1.2 \frac{\text{kg}}{\text{liter}}$ $D_d := 200 \text{ } \mu\text{m}$ $\rho_d := 3 \frac{\text{kg}}{\text{liter}}$ $\theta := 0 \text{ deg}$ $\kappa_f := 1$
 $\mu_c := \mu_{cl}(\rho_c, T)$ $\mu_c = 0.771 \text{ cP}$ $\alpha_d := 10\%$ $\kappa_{cv} := 1$



o Turian, Hsu, and Ma (1987)
 x Durand (1953) with $K_D = 150$
 + Zandi and Govatos (1967)
 — Heywood (1999)

$\epsilon_{\text{EOL_CS}} = 150 \text{ mil}$

$\epsilon_{\text{new}} = 2 \text{ mil}$

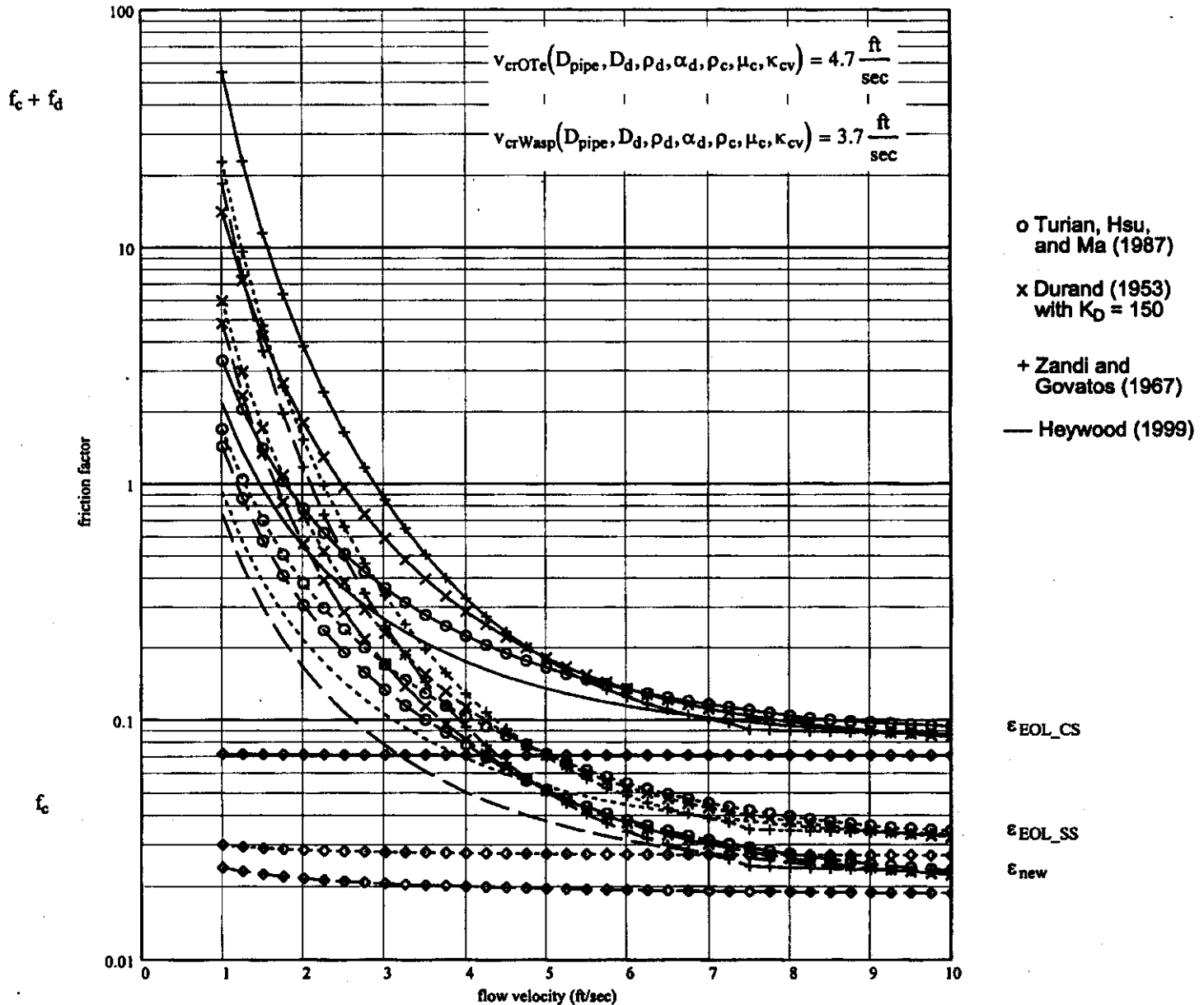


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 By: T. C. Oten

Figure 17. Friction Factor vs. Carrier Liquid Flow Velocity Comparison.

$T = 60 \text{ } ^\circ\text{C}$ $D_{\text{pipe}} = 3.068 \text{ in}$ $\rho_c = 1.2 \frac{\text{kg}}{\text{liter}}$ $D_d := 200\text{-}\mu\text{m}$ $\kappa_{cv} := 1$
 $\mu_c = 0.771 \text{ cP}$ $\alpha_d = 10\%$ $\rho_d := 3. \frac{\text{kg}}{\text{liter}}$ $\theta = 0 \text{ deg}$ $\kappa_f := 1$



f_c , non-slurry liquid without particles, $\alpha_d = 0$
 $f_c + f_d$, slurry with particles

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$\kappa_f := 1$ $\beta_f := 1$ $T = 60$ °C

$D_d := 400 \cdot \mu\text{m}$

$\alpha_d := 10\%$

$D_{\text{pipe}} := 3.068\text{-in}$

$L := 1000\text{-ft}$

$\theta := 0\text{-deg}$

$v_c := 5 \frac{\text{ft}}{\text{sec}}$

$\Delta p_{CS_{i,j}} := \Delta p(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_CS}, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta)$

$R_{CS_{i,j}} := R_d(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_CS}, D_d, \rho_d, \alpha_d, 1)$

$\Delta p_{SS_{i,j}} := \Delta p(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_SS}, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta)$

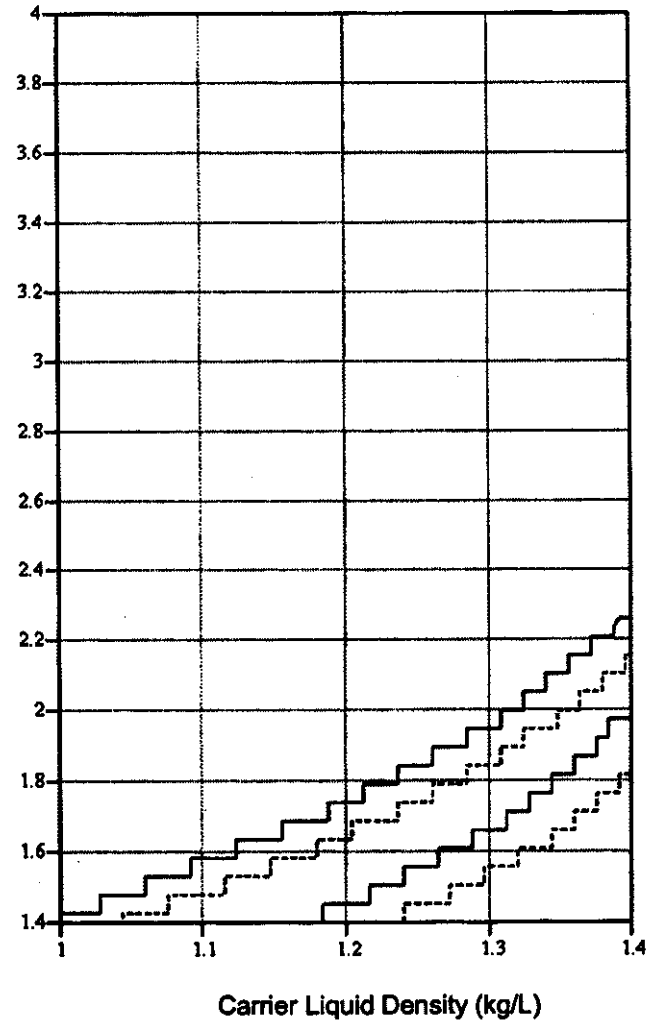
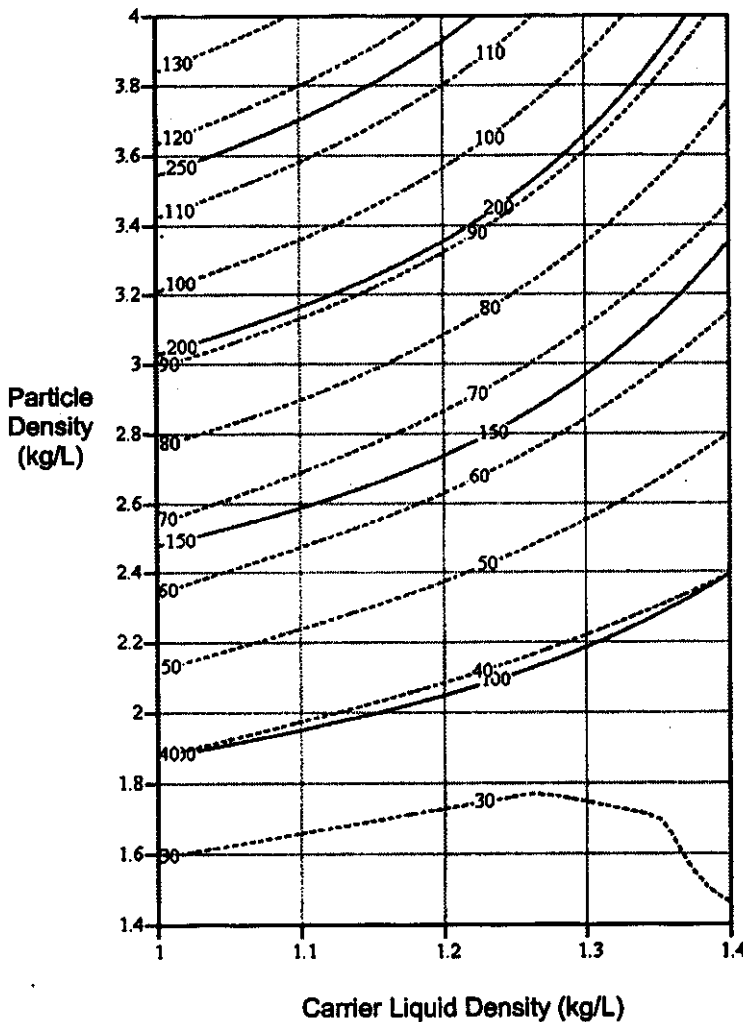
$R_{SS_{i,j}} := R_d(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_SS}, D_d, \rho_d, \alpha_d, 1)$

Figure 18a. Contour Plot of Best-Estimate Pressure Drop (psi) per 1000 ft of Pipe at Specified Flow Velocity (5 ft/s) (CS solid line, SS dashed line) vs. Particle Solids Density and Carrier Liquid Density - (Turian, Hsu, and Ma 1987)

Figure 18b. Corresponding Flow Regime Contour Plot.

$R_{CS_{0,N}} = 0$ $R_{SS_{0,N}} = 0$ $R_{CS_{N,N}} = 0$ $R_{SS_{N,N}} = 0$

$R_{CS_{0,0}} = 2$ $R_{SS_{0,0}} = 1$ $R_{CS_{N,0}} = 3$ $R_{SS_{N,0}} = 3$



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Date: 03/08/2000

By: L. J. Julyk

Checked: 03/10/2000

By: T. C. Oten

Location: 200 Area - Hanford Site, Richland, Washington

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$\kappa_{cv} := 1$ $\beta_{cv} := 1.2$ $T = 60$ °C

$D_d = 400$ μm

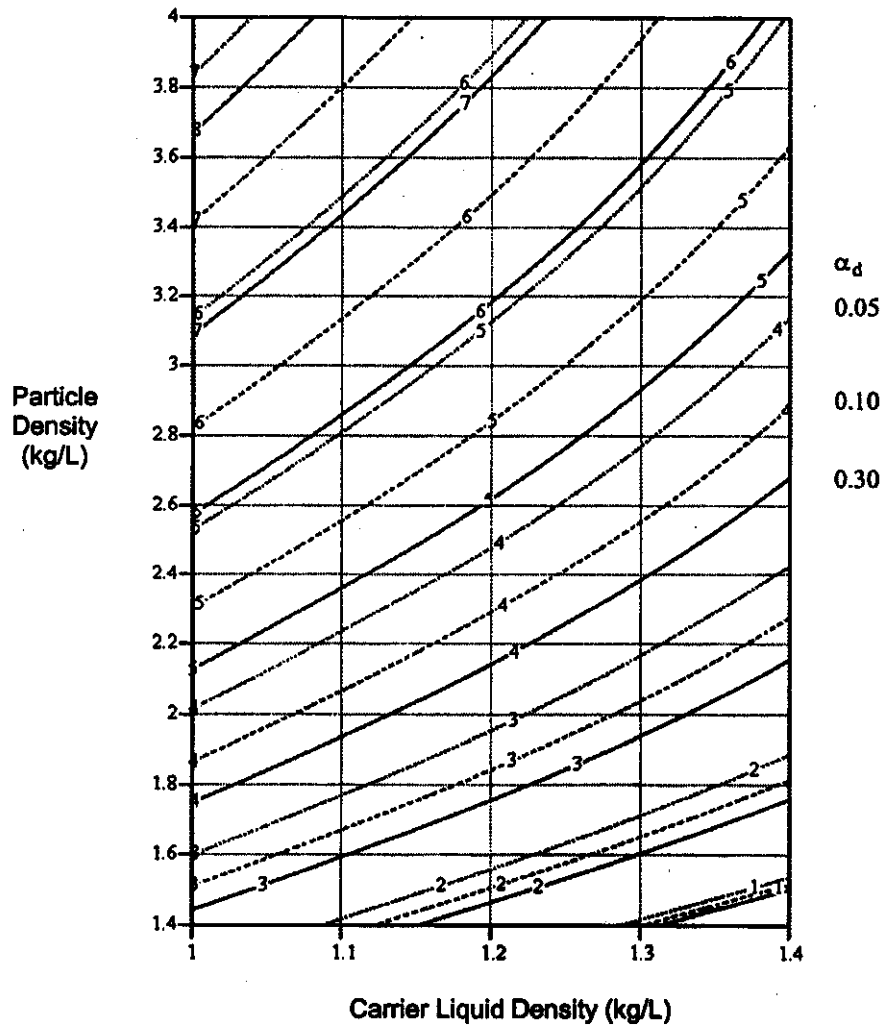
$D_{\text{pipe}} = 3.068$ in

$v_{1\text{crit},i,j} := v_{\text{cr}}(D_{\text{pipe}}, D_d, \rho_d, 0.30, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$

$v_{2\text{crit},i,j} := v_{\text{cr}}(D_{\text{pipe}}, D_d, \rho_d, 0.10, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$

$v_{3\text{crit},i,j} := v_{\text{cr}}(D_{\text{pipe}}, D_d, \rho_d, 0.05, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$

Figure 18c. Contour Plot of Best-Estimate Critical Velocity (ft/s) (Oroskar and Turlan 1980) vs. Carrier Liquid Density and Particle Density for Selected Particle Volume Fractions.



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 Date: 03/08/2000 By: L. J. Julyk
 Checked: 03/10/2000 By: T. C. Oten
 Revised: _____ By: _____

$\kappa_f = 1$ $T = 60 \text{ }^\circ\text{C}$

$D_d := 100 \cdot \mu\text{m}$

$\alpha_d := 10 \cdot \%$

$D_{\text{pipe}} = 3.068 \text{ in}$

$L = 1000 \text{ ft}$ $\theta = 0 \text{ deg}$

$v_c := 5 \cdot \frac{\text{ft}}{\text{sec}}$

$\Delta p_{CS_{i,j}} := \Delta p(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_CS}, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta)$

$R_{CS_{i,j}} := R_d(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_CS}, D_d, \rho_d, \alpha_d, 1)$

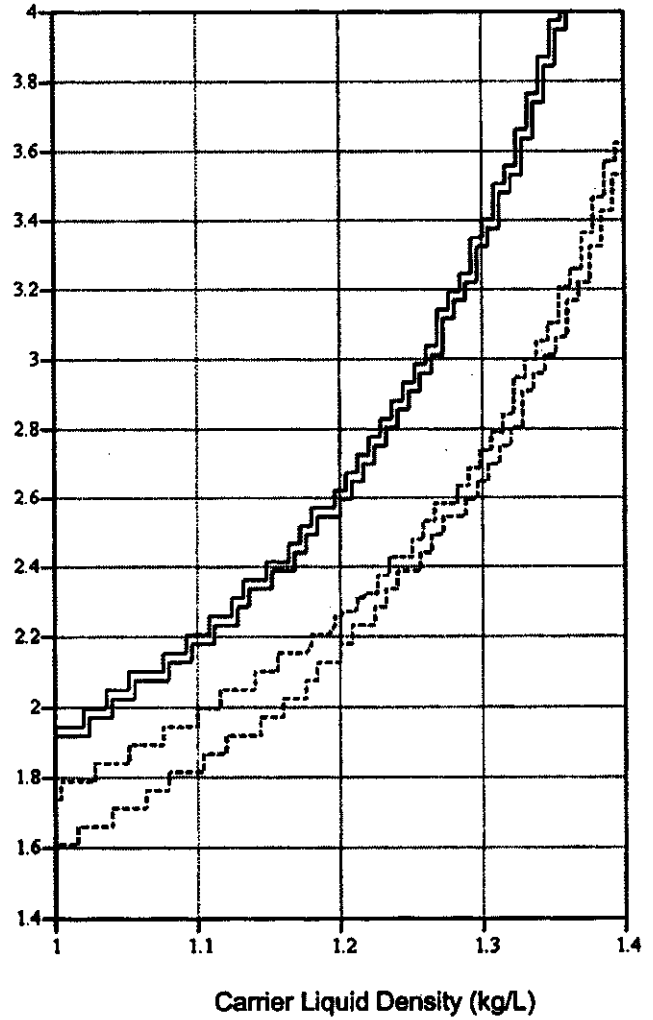
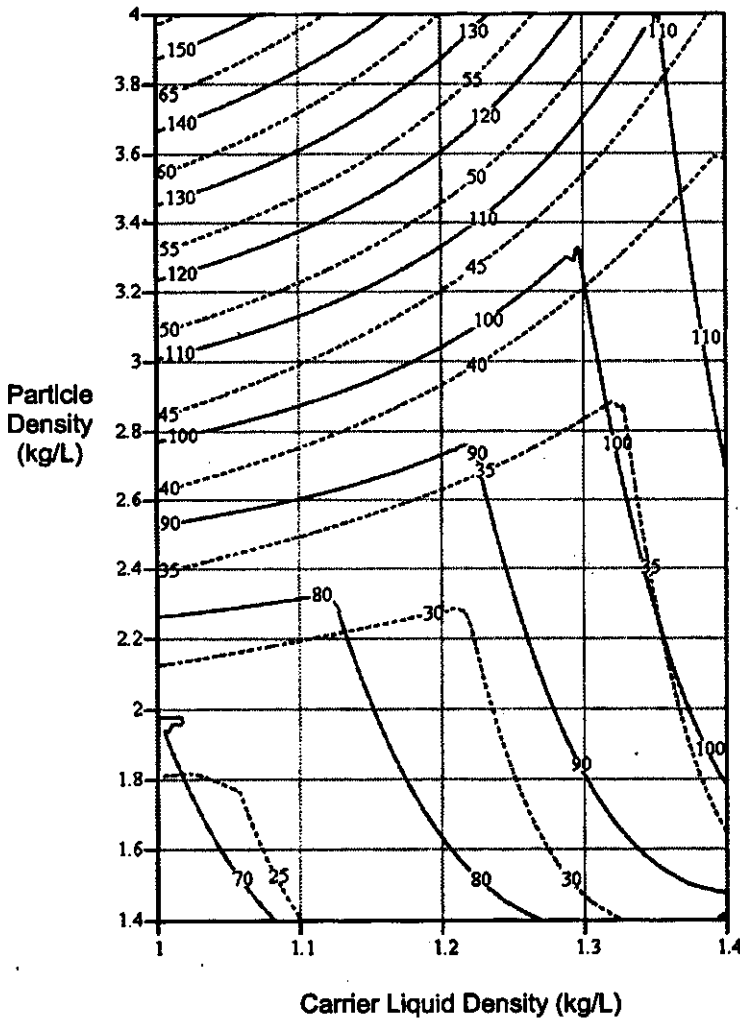
$\Delta p_{SS_{i,j}} := \Delta p(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_SS}, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta)$

$R_{SS_{i,j}} := R_d(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_SS}, D_d, \rho_d, \alpha_d, 1)$

Figure 19a. Contour Plot of Best-Estimate Pressure Drop (psi) per 1000 ft of Pipe at Specified Flow Velocity (5 ft/s) (CS solid line, SS dashed line) vs. Particle Solids Density and Carrier Liquid Density - (Turian, Hsu, and Ma 1987).

Figure 19b. Corresponding Flow Regime Contour Plot.

$R_{CS_{0,N}} = 0$ $R_{SS_{0,N}} = 0$ $R_{CS_{N,N}} = 3$ $R_{SS_{N,N}} = 1$
 $R_{CS_{0,0}} = 3$ $R_{SS_{0,0}} = 3$ $R_{CS_{N,0}} = 3$ $R_{SS_{N,0}} = 3$



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$\kappa_{cv} = 1 \quad \beta_{cv} = 1.2 \quad T = 60 \text{ } ^\circ\text{C}$

$D_d = 100 \text{ } \mu\text{m}$

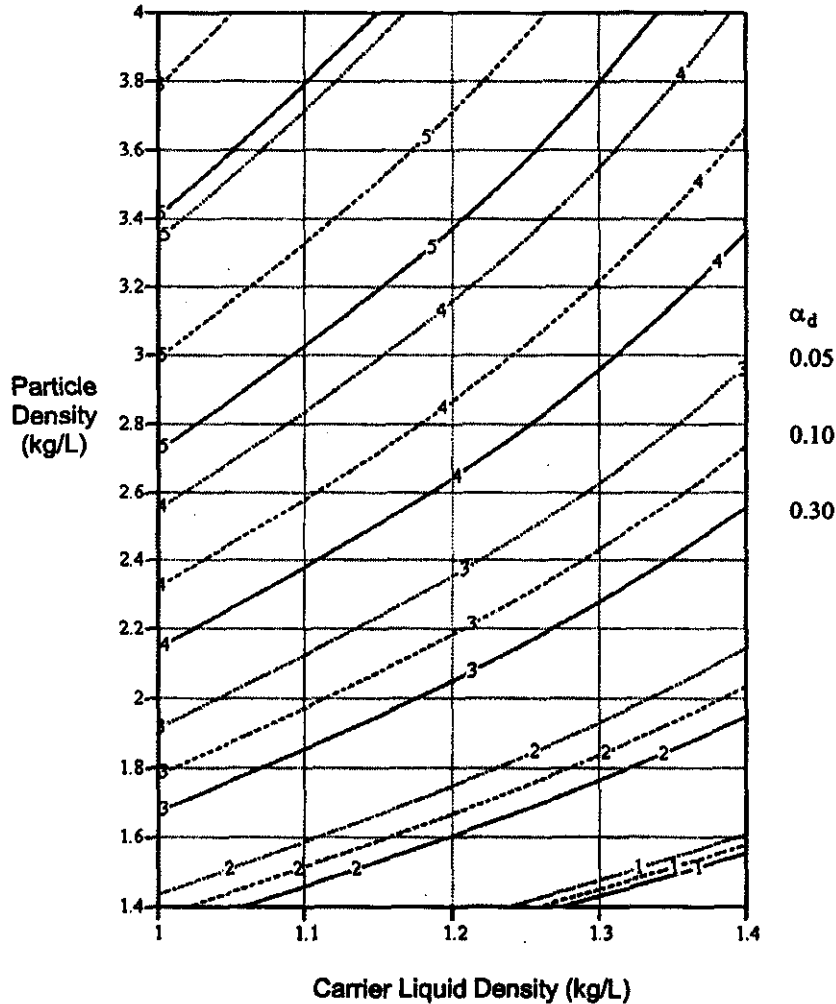
$D_{pipe} = 3.068 \text{ in}$

$v1_{crit,i,j} := v_{cr}(D_{pipe}, D_d, \rho_d, 0.30, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$

$v2_{crit,i,j} := v_{cr}(D_{pipe}, D_d, \rho_d, 0.10, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$

$v3_{crit,i,j} := v_{cr}(D_{pipe}, D_d, \rho_d, 0.05, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$

Figure 19c. Contour Plot of Best-Estimate Critical Velocity (ft/s) (Oroskar and Turian 1980) vs. Carrier Liquid Density and Particle Density for Selected Particle Volume Fractions.



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$\kappa_f = 1$ $T = 60 \text{ } ^\circ\text{C}$

$D_d := 40 \text{ }\mu\text{m}$

$\alpha_d := 10\%$

$D_{\text{pipe}} = 3.068 \text{ in}$

$L = 1000 \text{ ft}$ $\theta = 0 \text{ deg}$

$v_c := 5 \frac{\text{ft}}{\text{sec}}$

$\Delta p_{CS_{i,j}} := \Delta p(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_CS}, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta)$

$R_{CS_{i,j}} := R_d(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_CS}, D_d, \rho_d, \alpha_d, l)$

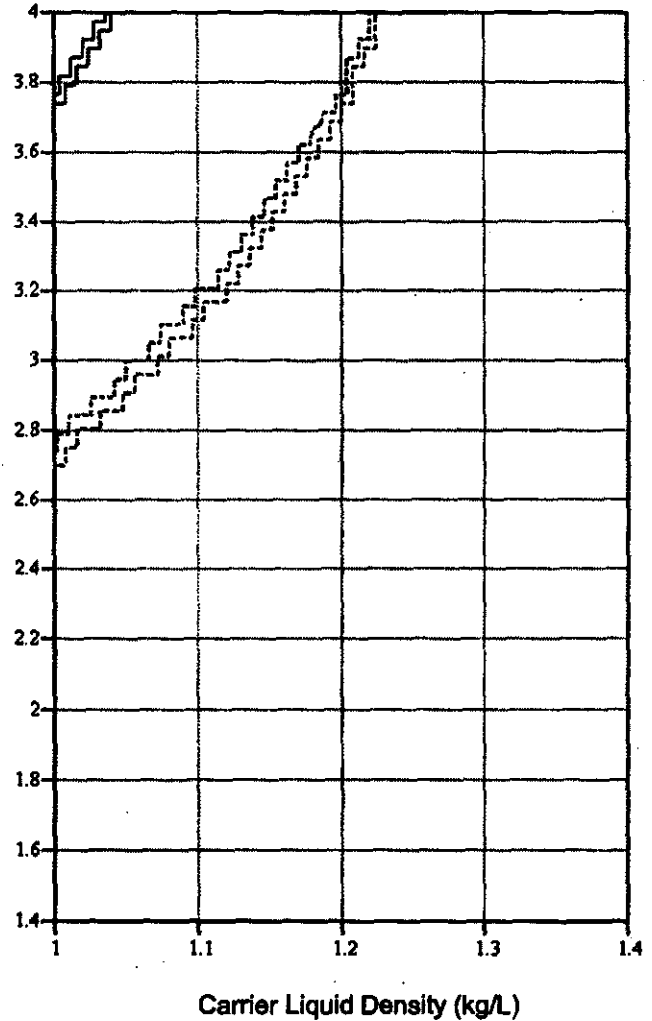
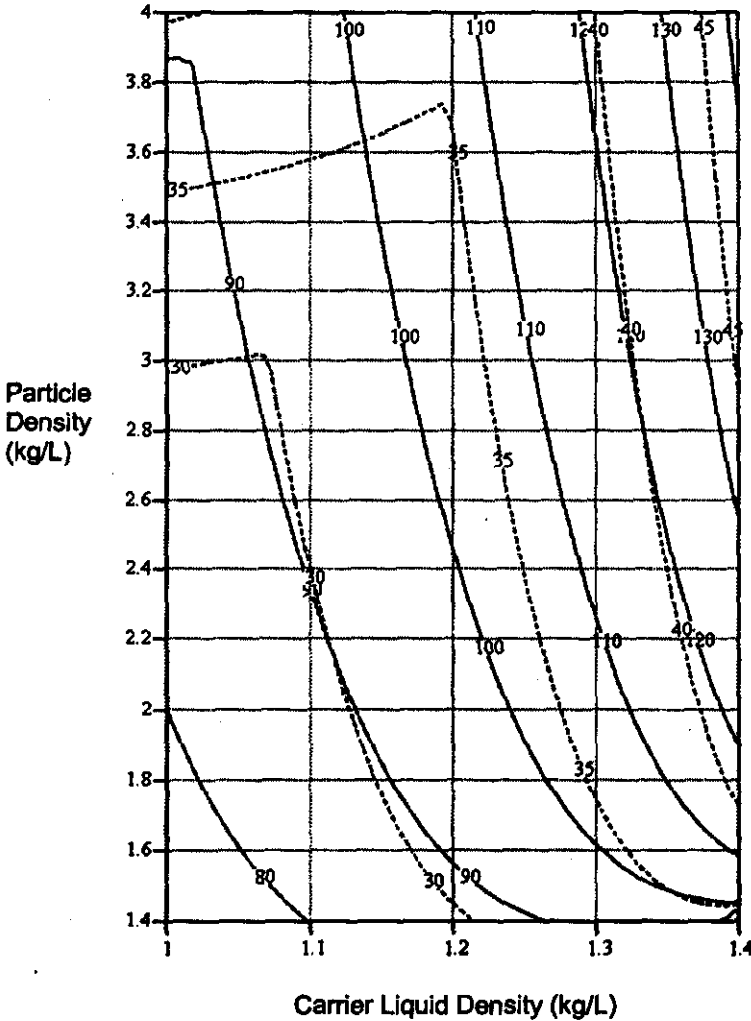
$\Delta p_{SS_{i,j}} := \Delta p(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_SS}, D_d, \rho_d, \alpha_d, L, \beta_f, \kappa_f, \theta)$

$R_{SS_{i,j}} := R_d(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \epsilon_{EOL_SS}, D_d, \rho_d, \alpha_d, l)$

Figure 20a. Contour Plot of Best-Estimate Pressure Drop (psi) per 1000 ft of Pipe at Specified Flow Velocity (5 ft/s) (CS solid line, SS dashed line) vs. Particle Solids Density and Carrier Liquid Density - (Turian, Hsu, and Ma 1987).

Figure 20b. Corresponding Flow Regime Contour Plot.

$R_{CS_{0,N}} = 0$ $R_{SS_{0,N}} = 0$ $R_{CS_{N,N}} = 3$ $R_{SS_{N,N}} = 3$
 $R_{CS_{0,0}} = 3$ $R_{SS_{0,0}} = 3$ $R_{CS_{N,0}} = 3$ $R_{SS_{N,0}} = 3$



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$\kappa_{cv} = 1$ $\beta_{cv} = 1.2$ $T = 60$ °C

$D_d = 40 \mu\text{m}$

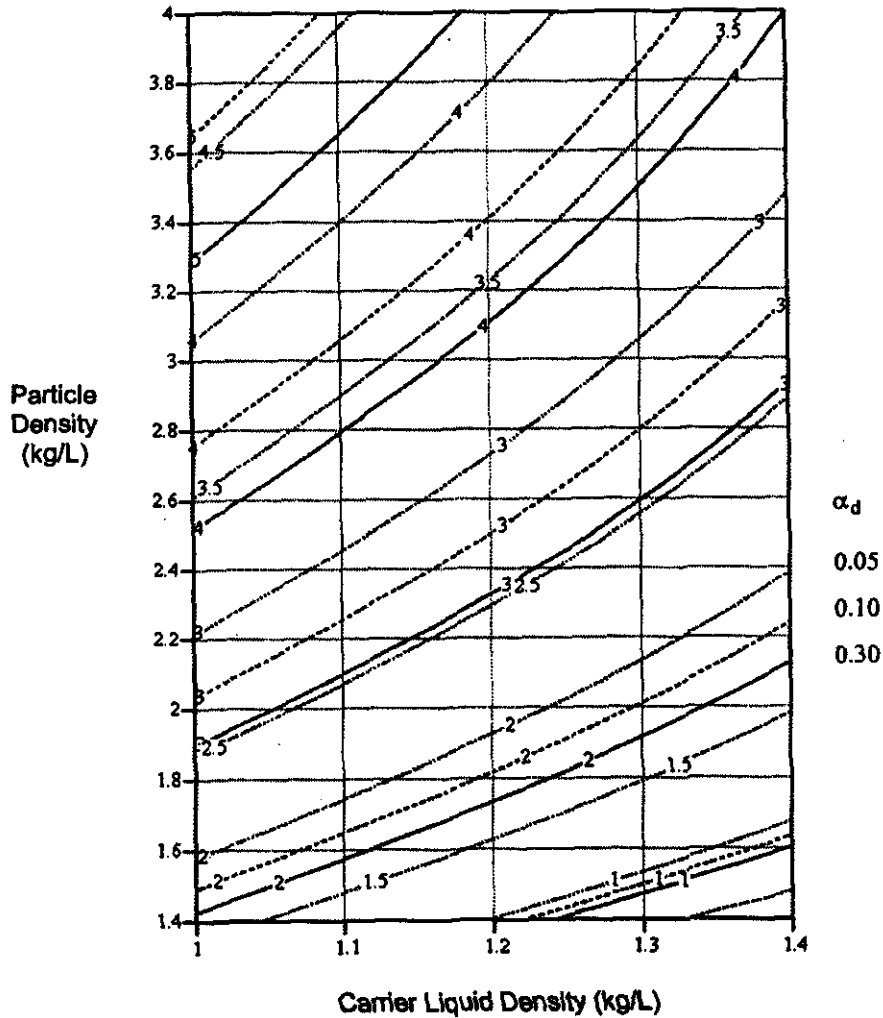
$D_{\text{pipe}} = 3.068$ in

$$v1_{\text{crit},i,j} := v_{\text{cr}}(D_{\text{pipe}}, D_d, \rho_d, 0.30, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$$

$$v2_{\text{crit},i,j} := v_{\text{cr}}(D_{\text{pipe}}, D_d, \rho_d, 0.10, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$$

$$v3_{\text{crit},i,j} := v_{\text{cr}}(D_{\text{pipe}}, D_d, \rho_d, 0.05, \rho_c, \mu_c, \kappa_{cv}, \beta_{cv})$$

Figure 20c. Contour Plot of Best-Estimate Critical Velocity (ft/s) (Oroskar and Turian 1980) vs. Carrier Liquid Density and Particle Density for Selected Particle Volume Fractions.



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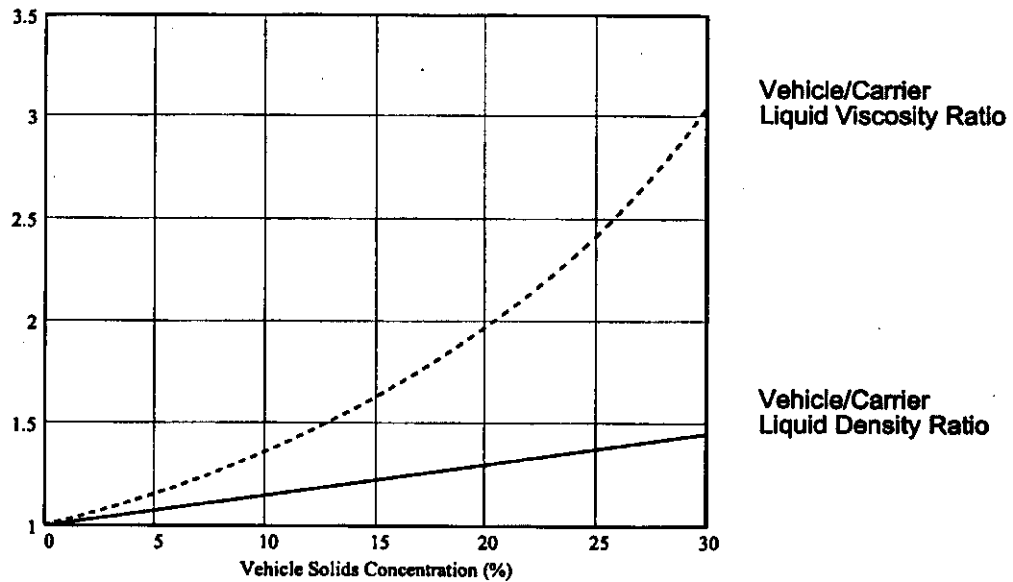
By: _____

Figure F-21. Normalized "Vehicle" Liquid Density and Viscosity Relations vs. "Vehicle" Solids Concentration.

$$\mu_c(\mu_L, \Phi_v) := \mu_L \cdot (1 + 2.5 \cdot \Phi_v + 10.05 \cdot \Phi_v^2 + 0.00273 \cdot \exp(16.6 \cdot \Phi_v))$$

$$\rho_c(\rho_s, \rho_L, \Phi_v) := \rho_s \cdot \Phi_v + \rho_L \cdot (1 - \Phi_v)$$

T := 60 °C $\rho_L := 1.2 \cdot \frac{\text{kg}}{\text{liter}}$ $\mu_L := \mu_{cL}(\rho_L, T)$ $\mu_L = 0.771 \text{ cP}$ $\rho_s := 3 \cdot \frac{\text{kg}}{\text{liter}}$



APPENDIX G

WASP MODEL VERIFICATION

Calculations contained herein were produced in Mathcad 2000 Professional (Mathcad is a registered trademark of MathSoft, Inc. of Cambridge, Massachusetts).

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CONVERSIONS

ft	feet = 0.3048 meter (m)
in.	inch = 2.54 centimeters (cm)
lbf/in ²	pounds per square inch = 0.06895 megapascals (MPa)
mils	0.001 in. = 0.0254 millimeter (mm)
gpm	0.0631 liter/second (L/s)
cP	centipose = 0.001 kilogram/meter-second (kg/m-s)

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 Checked: 03/10/2000 By: T. C. Oten
 Revised: _____ By: _____

PROBLEM

Verification check of Wasp method against published example.

APPROACH

Compare results of Wasp et al. 1977, illustrative example, page 95, with results obtained through application of equations given in Appendix F. All necessary equations are copied from Appendix F with following modifications unique to the check problem:

1. The viscosity relation from Wasp et al. 1977, illustrative example, page 95 is used.
2. Stokes flow for the settling particles is assumed in accordance with the illustrative example.

CONCLUSION

Less than 0.2% error is shown between results obtained through application of equations given in Appendix F and results of Wasp et al. 1977, illustrative example, page 95. This validates the implementation of the Wasp method in Appendix F.

REFERENCES

Wasp, E. J., J. P. Kenny, and R. L. Gandhi, 1977, *Solid-Liquid Flow – Slurry Pipeline Transportation*, Trans. Tech. Publ., Rockport, MA.

PRELIMINARIES

$$\Delta\epsilon := \text{TOL} \cdot 10^{-1} \quad \Delta\epsilon = 1 \times 10^{-8}$$

Return values of vector ϕ_p in accordance with nonzero values in vector $\phi_z > \Delta\epsilon$

```

pack( $\phi_p, \phi_z$ ) :=
    J ← length( $\phi_z$ ) - 1
    k ← 0
    for j ∈ 0..J
        if  $\phi_z_j > \Delta\epsilon$ 
             $\phi_k$  ←  $\phi_{p_j}$ 
            k ← k + 1
    if k = 0
         $\phi_0$  ←  $\Delta\epsilon$ 
         $\phi_1$  ←  $\Delta\epsilon$ 
     $\phi$ 
    
```

Return d_β particle diameter that is β th percentile value by volumes

$$D_\beta(d_p, \phi_{cum}, \beta) := \left| \begin{array}{l} J \leftarrow \text{length}(\phi_{cum}) - 1 \\ \text{for } k \in 0..J \\ \text{break if } \phi_{cum_k} > \beta \\ \frac{d_{p_k} - d_{p_{k-1}}}{\phi_{cum_k} - \phi_{cum_{k-1}}} \cdot (\beta - \phi_{cum_{k-1}}) + d_{p_{k-1}} \end{array} \right.$$

$$\text{TOL} = 1 \times 10^{-7}$$

$$\mu\text{m} = 10^{-6} \cdot \text{m}$$

$$\text{cP} = 10^{-2} \cdot \text{poise}$$

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 Date: 03/08/2000 By: L. J. Julyk
 Checked: 03/10/2000 By: T. C. Oten
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The Reynolds Number for Newtonian flow is defined as

$$Re(D, v, \rho, \mu) := D \cdot v \cdot \frac{\rho}{\mu}$$

where D is a characteristic length, v is the flow velocity, ρ is the fluid density, and μ is the fluid viscosity.

The terminal settling velocity (v_{∞}) of spherical particle settling in a stagnant unbound liquid is given by the following:

For Stokes flow ($Re < 0.1$) the drag coefficient of a spherical particle is given by

$$C_D(D_d, v, \rho_c, \mu_c) := \frac{24}{Re(D_d, v, \rho_c, \mu_c)} \quad \text{which leads the following explicit relation for } v_{\infty}$$

$$v_{\infty}(D_d, \rho_d, \rho_c, \mu_c) := g \cdot (\rho_d - \rho_c) \cdot (D_d)^2 \cdot (18 \cdot \mu_c)^{-1} \quad \text{Note that the check problem assumes Stokes flow.}$$

The approximation developed by Churchill in 1977 that includes the laminar and turbulent regimes as well as the transition regime between laminar and turbulent flow is used for the friction factor, i.e.;

$$f_c(Re, \eta) := \left[\begin{array}{l} A \left[\frac{2.457 \cdot \ln \left[\frac{1}{\left(\frac{7}{Re}\right)^{0.9} + (0.27 \cdot \eta)} \right]} \right]^{16} \\ B \left[\frac{37530}{Re} \right]^{16} \\ 8 \cdot \left[\left(\frac{8}{Re}\right)^{12} + \left(\frac{1}{A+B}\right)^{\frac{3}{2}} \right]^{\frac{1}{12}} \end{array} \right]$$

$$\text{or } f_c(D_{\text{pipe}}, v_c, \rho_c, \mu_c, \varepsilon) := f_c \left(Re(D_{\text{pipe}}, v_c, \rho_c, \mu_c), \frac{\varepsilon}{D_{\text{pipe}}} \right)$$

where v_c flow velocity of carrier liquid

Density of water as a function of temperature (°C)

$$\rho_{\text{water}}(T) := \left[999.7 - 0.10512 \cdot (T - 10) - 0.005121 \cdot (T - 10)^2 + 0.00001329 \cdot (T - 10)^3 \right] \cdot \frac{\text{kg}}{\text{m}^3} \quad \text{mass density of water}$$

(5 °C < T < 100 °C)

Viscosity of water as a function of temperature (°C)

$$\mu_{\text{water}}(T) := \left[\begin{array}{l} 100 \cdot \exp \left[\ln(10) \cdot \left[\frac{1301}{998.333 + 8.1855 \cdot (T - 20) + 0.00585 \cdot (T - 20)^2} - 3.30233 \right] \right] \cdot \text{cP} \quad \text{if } T \leq 20 \\ (1.002) \cdot \exp \left[\ln(10) \cdot \left[\frac{1.3272 \cdot (20 - T) - 0.001053 \cdot (T - 20)^2}{T + 105} \right] \right] \cdot \text{cP} \quad \text{otherwise} \end{array} \right]$$

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$$T_F(t_C) := \frac{9}{5} \cdot t_C + 32 \quad T_C(t_F) := \frac{5}{9} (t_F - 32) \quad T_C(68) = 20$$

$$\rho_L := \rho_{\text{water}}(T_C(68)) \quad \mu_L := \mu_{\text{water}}(T_C(68))$$

$$\rho_L = 0.998 \frac{\text{kg}}{\text{liter}}$$

$$\mu_L = 1.002 \text{ cP}$$

Liquid Density as a Function of Solids Concentration

$$\rho_c(\rho_s, \rho_L, \Phi_v) := \rho_s \cdot \Phi_v + \rho_L \cdot (1 - \Phi_v)$$

Viscosity as a function of solids concentration per

Wasp 1977, illustrative example, page 95

$$i1 := 0..5 \quad v\phi_{i1} := v\mu_{c1} :=$$

0	1.002
14	3.1
16	3.7
18	4.6
20	5.6
23	7.9

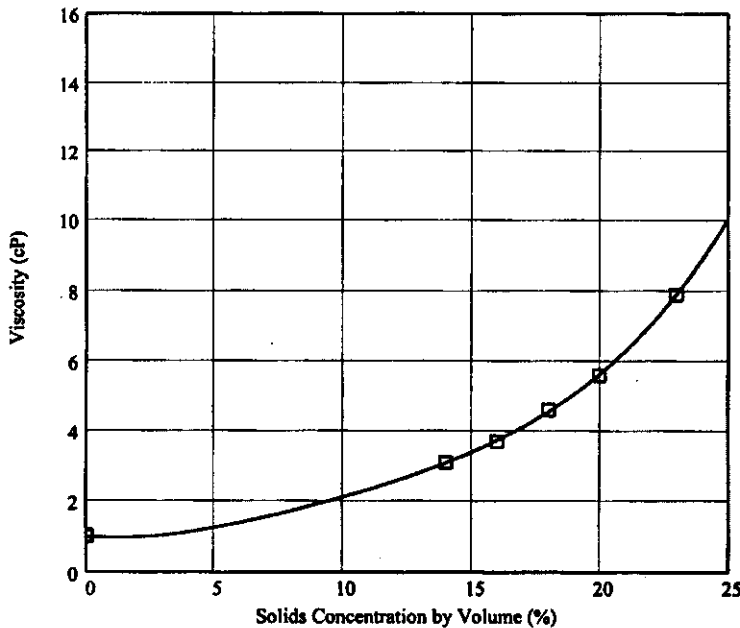
$$v\phi_1 := v\phi_1 \cdot \%$$

$$v\mu_{c1} := v\mu_{c1} \cdot \text{cP}$$

$$vs_1 := \text{regress}\left(v\phi_1, \frac{v\mu_{c1}}{\text{cP}}, 4\right)$$

$$\mu_c(\mu_L, \Phi_v) := \text{interp}\left(vs_1, v\phi_1, \frac{v\mu_{c1}}{\text{cP}}, \Phi_v\right) \cdot \text{cP}$$

Comparison of Viscosity Relations vs. "Vehicle" Solids Concentration.



Wasp et al. 1977, illustrative example, page 95

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Wasp's Method for Pressure Drop Estimate (Wasp et al. 1977)

assume $\beta := 1$ $\kappa := 0.4$

$$\frac{-1.8 v_{\infty}(d_p, \rho_s, \rho_c, \mu_c)}{\beta \cdot \kappa} \sqrt{\frac{f \cdot \rho_L}{\rho_c}} \cdot \frac{1}{8}$$

$$\phi_v(v, \rho_L, \rho_c, \mu_c, d_p, \rho_s, \phi_s, f) := \phi_s \cdot 10$$

$$\Phi_d(v, \rho_L, \mu_L, d_p, \rho_s, \phi_s, f) := \left| \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Phi'_v \leftarrow 0.5 \cdot \sum_{j=0}^J \phi_{s_j} \\ \text{root} \left(\Phi'_v - \sum_{j=0}^J \phi_v(v, \rho_L, \rho_c(\rho_s, \rho_L, \Phi'_v), \mu_c(\mu_L, \Phi'_v), d_{p_j}, \rho_s, \phi_{s_j}, f), \Phi'_v \right) \end{array} \right.$$

Durand friction factor correlation ($f_d = f - f_c$) using clear liquid density and viscosity properties of carrier liquid.

$$f_{dDurand}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, D_d, \rho_d, \alpha_d, K_D) := K_D \cdot \alpha_d \cdot f_c(D_{pipe}, v, \rho_L, \mu_L, \epsilon) \cdot \left[\frac{v^2 \cdot \sqrt{C_D(D_d, v_{\infty}(D_d, \rho_d, \rho_L, \mu_L), \rho_L, \mu_L)}}{g \cdot D_{pipe} \cdot \left(\frac{\rho_d}{\rho_L} - 1 \right)} \right]^{\frac{-3}{2}}$$

$$\Delta P_{Durand}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, D_d, \rho_s, \alpha_d, L, \theta, K_D) := \rho_L \cdot g \cdot \left[\frac{L}{D_{pipe}} \cdot \frac{v^2}{2 \cdot g} \cdot \left(f_c(D_{pipe}, v, \rho_L, \mu_L, \epsilon) \dots \right. \right. \\ \left. \left. + f_{dDurand}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, D_d, \rho_s, \alpha_d, K_D) \cdot \cos(\theta) \right) \dots \right. \\ \left. + \left[\left(\frac{\rho_s}{\rho_L} - 1 \right) \cdot \alpha_d + 1 \right] \cdot L \cdot \sin(\theta) \right]$$

Wasp friction factor and pressure loss

$$f_{Wasp_f}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \theta, f) := \left| \begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Phi_v \leftarrow \Phi_d(v, \rho_L, \mu_L, d_p, \rho_s, \phi_s, f) \\ \rho_v \leftarrow \rho_c(\rho_s, \rho_L, \Phi_v) \\ \mu_v \leftarrow \mu_c(\mu_L, \Phi_v) \\ f_W \leftarrow f_c(D_{pipe}, v, \rho_v, \mu_v, \epsilon) \cdot \frac{\rho_v}{\rho_L} \quad \text{vehicle friction contribution normalized to liquid} \\ \text{for } j \in 0..J \\ \left| \begin{array}{l} \phi_{vj} \leftarrow \phi_v(v, \rho_L, \rho_v, \mu_v, d_{p_j}, \rho_s, \phi_{s_j}, f) \quad \text{plus friction contribution from remaining heterogeneous particles using clear liquid properties} \\ \phi_{aj} \leftarrow \phi_{s_j} - \phi_{vj} \\ f_W \leftarrow f_W + f_{dDurand}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_{p_j}, \rho_s, \phi_{s_j}, 82) \cdot \cos(\theta) \end{array} \right. \\ f_W \end{array} \right.$$

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By: _____

$f_f := 0.025$ initial guess

$$f_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \theta) := \text{root}(f_f - f_{Wasp_f}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \theta, f_f), f_f)$$

$$\Delta P_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, L, \theta) := \left[\begin{array}{l} J \leftarrow \text{length}(d_p) - 1 \\ \Phi_s \leftarrow \sum_{j=0}^J \phi_{s_j} \\ \rho_L \cdot g \cdot \left[\frac{L}{D_{pipe}} \cdot \frac{v^2}{2 \cdot g} \cdot f_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \theta) \dots \right. \\ \left. + \left[\left(\frac{\rho_s}{\rho_L} - 1 \right) \cdot \Phi_s + 1 \right] \cdot L \cdot \sin(\theta) \right] \end{array} \right]$$

$$\Delta P_L(D_{pipe}, v, \rho_L, \mu_L, \epsilon, L, \theta) := \rho_L \cdot g \cdot \left(\frac{L}{D_{pipe}} \cdot \frac{v^2}{2 \cdot g} \cdot f_c(D_{pipe}, v, \rho_L, \mu_L, \epsilon) + L \cdot \sin(\theta) \right) \quad \text{pressure drop for liquid only}$$

EVALUATION ANALYSIS

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Wasp et al. 1977, illustrative example, page 95

$d_p := 0$ $\phi_s := 0$ $\phi_{cum} := 0$ $\theta := 0 \text{ deg}$

Given solid particle size distribution

d_p (cm)	volume frequency (%)
0.0021	19.55
0.0059	2.3
0.0111	1.15

$d_p := \text{data}^{(0)} \cdot \text{cm}$

$\phi_s := \text{data}^{(1)} \cdot \%$

$d_p = \begin{pmatrix} 21 \\ 59 \\ 111 \end{pmatrix} \mu\text{m}$

Given parameters

$\mu_L := \mu_{\text{water}}(T_C(68))$

$\rho_L := \rho_{\text{water}}(T_C(68))$

$\rho_s := 5 \frac{\text{kg}}{\text{liter}}$

$D_{\text{pipe}} := 12 \text{ in}$

$\epsilon := 0.002 \text{ in}$

$v := 4.5 \frac{\text{ft}}{\text{sec}}$

$L := 1 \text{ ft}$

Carrier liquid viscosity

$\mu_L = 1.002 \text{ cP}$

Carrier liquid density

$\rho_L = 0.998 \frac{\text{kg}}{\text{liter}}$

Average solids density

ID of pipe

pipe roughness

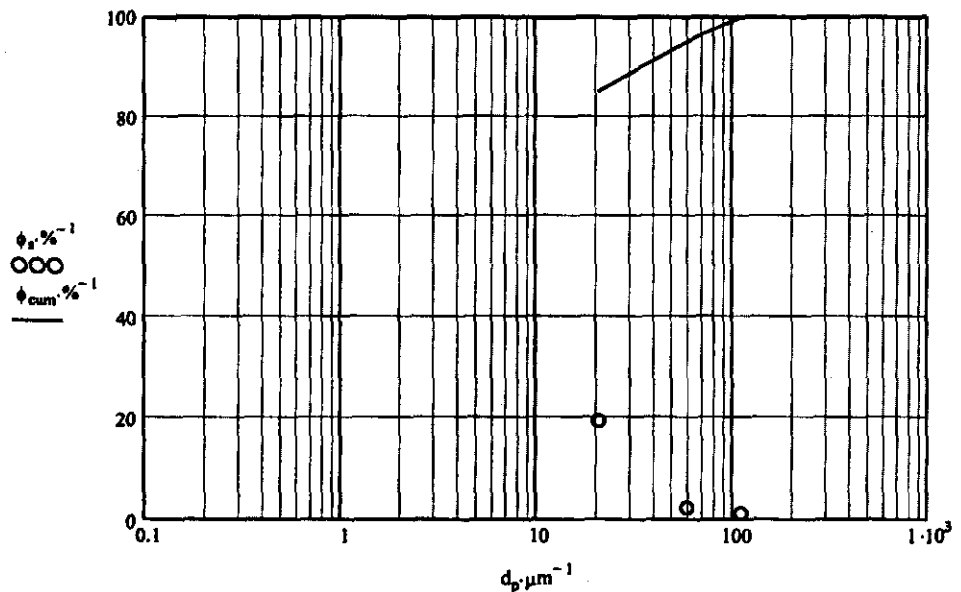
flow velocity

pipe unit length

$\Phi_s := \sum_{j=0}^{\text{length}(\phi_s)-1} \phi_{s_j}$ $\Phi_s = 0.23$ $j := 0..J$ $J := \text{length}(\phi_s) - 1$ $J = 2$

$\phi_{cum_j} := \frac{1}{\Phi_s} \cdot \sum_{i=0}^j \phi_{s_i}$

Cumulative undersize volume distribution



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TOL = 1×10^{-7}

$f_W := f_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, \theta)$ $f_W = 0.05282$

$\frac{\Delta p_{Wasp}(D_{pipe}, v, \rho_L, \mu_L, \epsilon, d_p, \rho_s, \phi_s, L, \theta)}{\rho_L \cdot g} = 0.01662 \text{ ft (ft of water/ft of pipe)}$ 0.01659 ft water / ft of pipe reported in Wasp et al. 1977 as approximate answer

$\frac{0.01662 - 0.01659}{0.01659} = 0.18\% \text{ error}$ Check OK

$\Phi_v := \Phi_c(v, \rho_L, \mu_L, d_p, \rho_s, \phi_s, f_W)$ $\Phi_v = 22.152\%$

$\rho_v := \rho_c(\rho_s, \rho_L, \Phi_v)$ $\mu_v := \mu_c(\mu_L, \Phi_v)$

$\rho_v = 1.885 \frac{\text{kg}}{\text{liter}}$ $\mu_v = 7.154 \text{ cP}$

$v_{t_j} := v_{\infty}(d_{p_j}, \rho_s, \rho_v, \mu_v)$

$\phi_{vv_j} := \phi_v(v, \rho_L, \rho_v, \mu_v, d_{p_j}, \rho_s, \phi_s, f_W)$

$\phi_{va_j} := \phi_s - \phi_{vv_j}$

j =	$\frac{d_{p_j}}{\mu\text{m}} =$	$\frac{v_{t_j}}{\text{ft/sec}} =$	$Re(d_{p_j}, v_{t_j}, \rho_v, \mu_v) = C_D(d_{p_j}, v_{t_j}, \rho_v, \mu_v) =$	$\phi_{vv_j} =$	%	$\phi_{va_j} =$	%	j =
0	21	0.00034	0.001	41469.3	19.29	0.26		0
1	59	0.00271	0.013	1869.9	2.07	0.23		1
2	111	0.00959	0.085	280.8	0.79	0.36		2

$v_{t_j} := v_{\infty}(d_{p_j}, \rho_s, \rho_L, \mu_L)$

$\sum_{j=0}^J \phi_{vv_j} = 22.15\%$ $\sum_{j=0}^J \phi_{va_j} = 0.85\%$

j =	$\frac{d_{p_j}}{\mu\text{m}} =$	$\frac{v_{t_j}}{\text{ft/sec}} =$	$Re(d_{p_j}, v_{t_j}, \rho_L, \mu_L) = C_D(d_{p_j}, v_{t_j}, \rho_L, \mu_L) =$
0	21	0.00315	0.020
1	59	0.02485	0.445
2	111	0.08796	2.965

$\frac{\epsilon}{D_{pipe}} = 0.00017$

$Re(D_{pipe}, v, \rho_v, \mu_v) = 1.101 \times 10^5$

$Re(D_{pipe}, v, \rho_L, \mu_L) = 4.165 \times 10^5$

Fanning friction factor

$\frac{f_c(D_{pipe}, v, \rho_v, \mu_v, \epsilon)}{4} = 0.00463$

$\frac{f_c(D_{pipe}, v, \rho_L, \mu_L, \epsilon)}{4} = 0.00387$

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